

A MULTI-COMPONENT HYDROGEOLOGIC
EVALUATION OF A SHALLOW GROUNDWATER FLOW
SYSTEM IN GLACIAL DRIFT

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ABSTRACT

A multi-component analysis of a shallow groundwater flow system in glacial drift at the Whiteshell Nuclear Research Establishment, Manitoba is described. The investigation included geologic test drilling, definition of the hydraulic potential distribution, hydrochemical analysis, studies of the temperature distribution in the groundwater zone, radioactive tracer studies of groundwater flow, observations of secondary permeability characteristics and mathematical modelling of the geohydrologic regime.

The stratigraphic section consists of seven glacial deposits which are: a lacustrine sand and gravel unit, a lacustrine silt unit, a lacustrine clay unit, a clay-loam till, a basal sandy drift, an undifferentiated sandy till, and a lacustrine sand unit. The glacial deposits rest on Precambrian metamorphic and intrusive rock.

The groundwater flow system is bounded on the east by a topographic high formed by the lacustrine sand, on the west by the Winnipeg River and in the subsurface by relatively impermeable Precambrian bedrock. Groundwater potential distribution exhibits four major geohydrologic zones. From east to west these are: an upland recharge area consisting of the topographically high lacustrine sand unit, a strong central discharge area, a strong central recharge area and a lowland discharge area confined to the banks of the Winnipeg River. Predominantly lateral flow occurs in the more permeable basal sandy drift with lateral potential gradients existing only locally in the overlying units. Maximum horizontal tracer velocities detected at

three sites are less than 10 feet per year. Groundwater flow through the less permeable upper clay-loam till and glacio-lacustrine silt and clay units occurs via joints and sand laminae.

Variations in electrolytic conductivity and distribution of Ca^{++} , Mg^{++} , Na^+ , and $\text{SO}_4^{=}$ exhibit distinct increases in the directions of groundwater movement. The increase in major-ion concentrations with distance along the flow path in the lacustrine sand unit and the basal sandy drift is less pronounced than in the clay-loam till and the glacio-lacustrine silt and clay units due to a more active flow system and limited availability of soluble minerals in the sandy deposits.

Results of the modelling suggest that the location of the boundary between the central recharge area and the central discharge area is a result of a pronounced thinning of the basal, highly permeable, sandy drift. Strong recharge gradients in the central recharge area could not be simulated in the model.

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INTRODUCTION

The objective of this study is to define the groundwater regime in the Environmental Control Area, Whiteshell Nuclear Research Establishment (WNRE), Pinawa, Manitoba. Since May, 1968, field and laboratory investigations have been conducted to provide hydrogeologic information as a basis for design and location of radioactive liquid and solid waste disposal facilities in the glacial drift which underlies the area. Drift thickness ranges from 30 to 75 feet. The drift is underlain by relatively impermeable Precambrian crystalline rock.

The field study included investigations of the geologic framework, subsurface permeability distribution, hydraulic potential distribution, temperature distribution in the groundwater zone, groundwater-flow velocities and major-ion distribution in the groundwater zone. A two-dimensional, steady-state model of the groundwater flow system was used to refine the field interpretation of subsurface permeability distribution.

The WNRE is located 65 miles east-northeast of Winnipeg on the Winnipeg River (Fig. 1). The Environmental Control Area (Fig. 2) occupies 4 square miles of the WNRE site. Radioactive waste is presently being disposed of in only a small portion of the area (Fig. 2).

This thesis reports on the hydrogeologic environment of the area. Specific application of the results to radioactive waste disposal will be the subject of a future report.

Four previous geotechnical studies of the plantsite have been conducted. Shawinigan Engineering Limited (1960) summarized the results of an initial geological and geophysical investigation of the WNRE site. Charron (1964) conducted a reconnaissance survey of the radioactive waste disposal capabilities of the groundwater regime. Zwarich (1968) is continuing studies on ion exchange characteristics of the glacial drift in the Environmental Control Area. Lund (1967) investigated groundwater flow velocities at one location using a tritiated heavy-water tracer. Apparent anomalies in the results of her study were partially responsible for prompting the present investigation.

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Chemical analyses were conducted by the Soil Science Chemistry Laboratory, University of Manitoba, under the supervision of Dr. M. Zwarich and the Analytical Chemistry Branch, WNRE. The use of tritiated heavy water was supervised by the Radiation and Industrial Safety Section, WNRE. Drilling was completed under contract by Great Plains Augering Limited, Saskatoon in 1968 and 1969 and by Mobil Augers and Research Limited, Edmonton, in 1970.

The investigation was conducted in 1968 with direct financial support of the Environmental Control Section, WNRE, at which time I was a summer research assistant on the project. The study was continued through 1969-70 by the University of Manitoba under research contract with Atomic Energy of Canada, Limited. During this period I was employed on the project as a research assistant.

GЕOMORPHIC AND HYDROLOGIC SETTING

The Environmental Control Area is located on the western edge of the Canadian Shield. Sedimentary terrain in the area originated from Wisconsinan glaciations of the Pleistocene Epoch and associated episodes of Glacial Lake Agassiz (Elson, 1966, McPherson, 1968; 1970). Deglaciation of the area occurred between 11,700 and 13,000 years BP with subsequent inundations by Glacial Lake Agassiz prior to 7300 years BP (Elson, 1966).

Relief in the Environmental Control Area varies between 905 feet and 840 feet above mean sea level (Fig. 3). The regional topography (Fig. 2) is a plain with isolated highs. Aerial photographs of the WNRE site and surrounding area (Fig. 4 and Appendix A) illustrate the low relief and marshy character of the terrain. Oblique modified infra-red photographs (Fig. 5 and Appendix A) show vegetation patterns delineating areas of both bog and sand outcrop.

In relation to worldwide climatic conditions, WNRE is within the region Dfb. (Koppen and Geiger, 1936), characterized by areas near the center of a continent with temperature extremes greater than the world average for the latitude. Recorded temperature extremes at WNRE are -53.5° F. and 108° F. (A. Reimer, personal communication). Mean daily temperature drops below 32° F. during the first part of November and rises above 32° F. near the end of March with an annual mean daily temperature of approximately 35° F.

Precipitation varies greatly with a mean of 20 inches per year, the greatest portion occurring during the summer months (Smith and Ehrlich, 1967). Recorded extremes are a low of 12.5 inches in 1961 and a high of 26.6 inches in 1968 (A. Reimer, personal communication).

METHODS OF INVESTIGATION

During the initial stages of the study, a base map prepared by Shawinigan Engineering Limited (1960) was used in conjunction with standard 1:16,000 aerial photographs to coordinate field studies. The field studies were conducted during the April-September periods of 1968, 1969 and 1970.

Physical Character of Glacial Drift

The geologic framework in the Environmental Control Area was defined by 76 test holes drilled by a truck-mounted hydraulic drill equipped with a six-inch diameter solid-stem auger flights. Disturbed samples were collected from auger flights, with sample depths estimated to within ± 1 foot. A small portable power drill (Mobil Augers Limited, Minuteman Model) equipped with three-inch auger flight was used for two preliminary test holes but its use was discontinued due to the bouldery character of the glacial till and poor quality of samples. Geologic logs of test holes with interpretations are listed in Appendix B.

Shelby tube cores 0.25 feet inside diameter and two feet in length were taken to define in detail the texture of the glacial drift and to investigate the frequency and character of joints and fractures. Immediately after sampling the cores were sealed using plastic sheeting in the tubes. The samples were then stored at room temperature until inspection could be made. A fluorescent dye, sodium fluorescein, was injected into the glacial drift prior to coring in an attempt to study the physical character of the joints.

Inspection of cores in which sodium fluorescein was thought to be present was conducted under ultraviolet light.

Piezometer Design and Installation

To define the hydraulic potential distribution in the glacial drift, 60 observation sites consisting of 210 piezometers and water table observation wells were installed in the glacial drift. Water levels were measured periodically.

Piezometers of the standpipe design (Fig. 6) were constructed of polyvinylchloride (PVC) pipe. Piezometers installed in 1968 and 1969 had an inside diameter of 0.94 inches and those installed in 1970 had an inside diameter of 0.80 inches. The bottom two feet of the standpipes were perforated and screened with fine-mesh stainless steel screen in 1968 and 1969 and with fine-mesh fibreglass mat in 1970. The intake area was packed with coarse silica sand. Bentonite or very fine silica sand was then placed above the coarse sand pack as a seal against grout. The hole was sealed by a grout mixture of sulphate-resistant cement.

Three piezometers using mild steel standpipes were installed in 1969. These piezometers were not entirely satisfactory because of difficulty preventing silt and sand from entering the standpipe as well as corrosiveness of the groundwater to mild steel. The PVC piezometers generally attained equilibrium within a week after installation (Fig. 7 and Appendix C) and were subsequently flushed and tested for response and return to equilibrium. The use of PVC standpipes made the installations ideal for hydrochemical purposes. Representative piezometer hydrographs are

shown in Figures 8 - 10. Hydrographs of all other piezometers installed from 1968 to 1970 are included in Appendix C. Piezometer locations and elevations were determined by transit and level with horizontal control of approximately 10 feet and vertical control of 0.1 feet.

Hydraulic conductivities of the glacial drift were obtained from piezometer response tests using the methods of Hvorslev (1951; 1954).

Hydrochemistry and Geothermometry

Groundwater samples were collected from the screened zone of piezometers using a narrow-diameter downhole sampler and by vacuum pump. Samples were isolated from the atmosphere immediately after sampling and stored in polypropylene bottles until analyses were made.

The results of analyses for Ca^{++} , Mg^{++} , Na^+ , Cl^- , and $\text{SO}_4^{=}$ are tabulated in Appendix D. Concentrations of HCO_3^- and $\text{CO}_3^{=}$ were calculated by the procedure discussed by Barnes (1964) from titration data and are listed with total dissolved solids in Appendix D. Field pH measurements were obtained using a null-balance pH meter (Radiometer Model pHM-4) following the procedure of Barnes (1964). Electrolytic conductivities were measured in the water at the intake area of piezometers using an extended-cable down-hole conductivity cell (Solubridge Model B3-338). Field pH and conductivity results are also listed in Appendix D. Groundwater temperatures were measured at intervals in piezometers with an extended-cable thermistor (Whitney Model TC-5A) and are summarized graphically in Appendix E along with meteorological summaries for 1968, 1969 and 1970 obtained from the Environmental Control Section, WNRE.

Heat transfer from the thermistor and connecting cable into the narrow-diameter standpipes resulted in difficulties in obtaining reproducible values. Conduction of heat from surface down the standpipe and convection within the standpipe are also believed to greatly reduce the reliability of the data.

Tracer Injection Experiments

Groundwater flow velocities were studied by injecting tritiated heavy water into the glacial drift at three locations in the study area (Fig. 3). The injection experiments were initiated in July, 1969 at a site immediately north of the present Active area. Injections at the other two sites were made in August, 1969. Each injection site comprises one 3.5 inch-diameter injection well and up to 40 sampling tubes concentrated predominantly in the anticipated direction of a flow. Sampling tubes were constructed and installed in the same manner as piezometers except that the perforated area was increased to 3 or 5 feet, depending on the sampling zone desired. The locations of injection wells and sample tubes are shown in Figures 11 - 13.

Samples were obtained from sample tubes by vacuum lines set in the perforated area of the sampling tube. Tritium analyses were conducted using a liquid scintillation counter supplied by the Environmental Control Branch, WNRÉ. Beta activity fluctuations for selected sample tubes are presented in Appendix F.

Prior to injection of tritium, hydraulic connection of all sample tubes to the injection well was confirmed by pressure injection of pure water. At injection site No. 1, a trial injection of sodium fluorescein was conducted to test injection and sampling procedures.

An injection well was also installed approximately 5 feet east of piezometer nest No. 20. Only sodium fluorescein was injected at this point, the primary purpose of the experiment being to take Shelby tubes within the injection zone as a means of investigating secondary permeability characteristics of the fine-grained deposits.

Steady-State Mathematical Modelling of the Groundwater Regime

A computer program initially prepared by Morris (1968; 1969) for IBM Systems 360-65 using the finite-element techniques outlined by Zienkiewicz and Cheung (1966) was used in the analysis of the groundwater flow pattern and subsurface permeability distribution. The program solves the two-dimensional steady-state flow equations for groundwater flow in saturated, anisotropic, heterogeneous porous media with an irregular water table and impermeable flow boundaries or their equivalent. The program was adapted for specific application to the flow regime in the WNRE area with solutions for the hydraulic potential distribution being plotted by a Calcomp plotter. Program listings and comments on the program are compiled in Appendix G.

STRATIGRAPHY

The stratigraphic section in the Environmental Control Area, WNRE, consists of seven units overlying Precambrian bedrock. These are: a lacustrine sand and gravel unit, an interbedded lacustrine silt, a lacustrine clay, a carbonate-rich, clay-loam till, a basal sandy drift rich in Precambrian-derived pebbles and boulders, a sandy till, and a unit of predominantly fine to medium-grained sand. The total thickness of these units overlying the Precambrian bedrock varies between 30 and 75 feet. The surficial geology is shown in Figure 14 and geologic cross sections are shown in Figures 15 - 17.

The stratigraphic section in the upland area is much more variable than in the western plain. As shown in Figure 15, the northwestern portion of the upland is underlain by only one stratigraphic unit, a thick deposit of relatively uniform fine to medium-grained lacustrine sand. The unit includes a few thin interbeds of silt and clayey silt which are probably discontinuous. This deposit extends westward beneath the clay-loam till and is terminated in the northeastern portion of the upland by the unit which appears to be a sandy and in places silty till. Figure 16 shows that the southern portion of the upland is underlain by a much different stratigraphic sequence. The thick section of sand shown in Figure 15 is not present, and a lacustrine sand and gravel deposit overlies the lacustrine clay.

West of test hole 48 (Fig. 16) the surficial sand and gravel deposits pinch out and the section consists of, in descending order, lacustrine clay, clay-loam till, and a basal unit of fine to coarse-grained sand.

The stratigraphic sequence east of the upland has not been defined by test drilling. However, based on the topographic setting, the poorly drained character of the terrain, and the continuity of both the clay-loam till unit and the lacustrine clay under the upland (Fig. 16) it appears likely that the area is underlain by both of these units.

A north-south geologic cross section (Fig. 17) illustrates the relatively uniform character of the stratigraphy in the more central portions of the Environmental Control Area.

Lacustrine Sand and Gravel Unit

The lacustrine sand and gravel unit consists of up to 15 feet of fine to coarse-grained sand with minor silt beds and gravel. The deposit is restricted to an isolated topographic high in the southeastern portion of the area (Fig. 15). Grain-size characteristics suggest relatively high hydraulic conductivities, probably in the range of 1×10^{-5} to 1×10^{-2} fps.

Lacustrine Silt Unit

The lacustrine silt unit observed in the Environmental Control Area represents a late phase shallow-water deposit of Glacial Lake Agassiz (McPherson 1968). This unit consists of massive to laminated very fine sands, sandy silts, silts,

clayey silts and silty clays with a maximum thickness of 15 feet occurring near the Winnipeg River, and wedges out in the central portion of the area. Where the unit is less than two feet thick, it is somewhat obscured by the effects of soil formation.

Easily eroded masses resembling primary bedding structures were recognized in cores of this unit (Fig. 18). The structures are generally one to three inches in length and less than 1/2 inch in thickness. They may represent scours subsequently filled with coarser silt. They were not observable unless the cores were washed with a high pressure stream of water. The lower contact of the silts to the deep-water clays appears gradational over several feet.

In the poorly drained western part of the Environmental Control Area underlain by the lacustrine silt unit, perched water table conditions frequently exist. Water-table observation wells located adjacent to areas frequently covered by water indicate that the water table lies three to five feet below ground level. This ponding is thought to be due to local interbeds of silty clay near the soil zone which retard downward percolation of surface water.

Lacustrine Clay Unit

The lacustrine clay unit was deposited during a high-water phase of Glacial Lake Agassiz (McPherson, 1970). The thickness has a maximum of 25 feet near the Winnipeg River. Carbonate pebbles are present throughout this unit but become more common in the lower portions. Fragments of till-like material up to 10 inches in diameter (Fig. 19) were noted in isolated cases. Angular buff-coloured calcareous silt clasts are very common

near the bottom of the unit (Fig. 20). The pebbles, till and silt clasts are probably due to ice rafting although Weller (1960), Hills (1963) and Elson (1966) have discussed possible alternate origins for such features.

Laminae of fine to very fine sand are present throughout the unit at angles up to 90° to the bedding (Fig. 21). These laminae may cut horizontal or shallow dipping laminae of clay and silty clay or may be conformable to them. A probable origin of the sand laminae is infilling of slump or desiccation cracks formed by fluctuations in the lake level.

The lower contact of the clay is marked by a stratified zone up to three feet thick of pebbly, sandy silt, and silty sandy clay (Fig. 22).

Grain-size analyses of the glacio-lacustrine clays conducted by Zwarich (1968) on five samples collected from drill holes near the present radioactive disposal area (Fig. 2) indicate a 60:30:10 ratio of clay, silt and sand, respectively. It is expected that the largest portion of the silt and sand occur as laminae within the clay unit rather than a homogeneous unit of clay, silt and sand.

Inspection of Shelby tube cores taken near piezometer nest No. 20 (Fig. 2) indicate well-defined jointing throughout this unit. The joints may appear as thin dark lines cutting laminae (Fig. 23) or as planes coated with carbonate and gypsum precipitates (Fig. 24). Rootlets follow the joint surfaces as deep as 14 feet (Fig. 20).

The results of Hvorslev response tests indicate that the lacustrine clay unit is characterized by hydraulic conductivities in the range of 1×10^{-10} to 1×10^{-7} fps at depths below ground surface greater than 14.5 feet and by conductivities in

the range of 1×10^{-10} to 1×10^{-5} fps at depths less than 14.5 feet below ground surface. Laboratory tests conducted by Templeton Engineering Company Limited (1960) on small samples obtained from the lacustrine clay near the plant (south half, section 21) yielded values in the range 1×10^{-10} to 3×10^{-9} fps. This suggests that joints, fractures or other secondary in situ characteristics of the lacustrine clay unit significantly increase the hydraulic conductivity. A more detailed analysis of the hydraulic conductivities associated with the lacustrine clay is discussed in a later section.

Clay-loam Till Unit

The lacustrine clay unit grades through a stratified zone of pebbly, sandy silt, and silty sandy clay, a few feet thick into a pebbly, carbonate-rich, clay-loam till. The deposit ranges from 0 to 30 feet in thickness. Grain-size analyses (Zwarich, 1968) of composite samples show approximately equal proportions of clay, silt and sand near the upper contact with the sand content increasing to over 50 per cent near the lower contact. Pebbles and cobbles are predominantly dolomitic with the remainder commonly being highly weathered Precambrian material. The lower contact is marked by a pavement of Precambrian cobbles and boulders. Considerable difficulty penetrating this layer was encountered at many locations. This unit correlates with the Libau Drift mapped and described by McPherson (1968) in the area west of the Winnipeg River.

Joint planes coated with carbonate precipitates (Fig. 25) were commonly observed in Shelby tube cores taken near the upper contact of this unit.

The results of Hvorslev response tests indicate that the clay-loam till is characterized by hydraulic conductivities in the range of 1×10^{-7} to 1×10^{-4} feet per second (fps). Joints appear to account for a significant secondary permeability network although only a few irregular joint-fracture features were observed in the Shelby tube samples from this unit. A more detailed analysis of the hydraulic conductivities associated with this unit is presented in a later section.

Basal Sandy Drift Unit

The basal sandy drift unit underlies the clay-loam till unit in the western third of the study area (Figs. 15 - 17). The deposit varies in thickness from 0 to 15 feet. Texturally, it varies from a silty till to unsorted gravel and sand to a well sorted fine to coarse-grained sand with few, if any, till-like features. Compositionally the unit is characterized by angular rock fragments of Precambrian Shield lithologies. Sedimentary rock fragments were observed only in isolated cases. The upper contact of the basal sandy drift with the clay-loam till is defined by a boulder pavement.

McPherson (1970) states:

The main basis for distinguishing the Belair Drift is its occurrence on surface as a sandy end moraine or outwash, its characteristic mineralogy indicating it was primarily derived from acid igneous rocks, and its stratigraphic position in the subsurface being stratigraphically lower than Lake Agassiz sediments and Libau Drift.

By this definition, the basal sandy drift unit can be stratigraphically correlated with the Belair Drift.

The results of Hvorslev response tests indicate that the basal sandy drift is characterized by hydraulic conductivities in the range of 1×10^{-7} to 1×10^{-4} fps. A detailed analysis of the hydraulic conductivities associated with this unit is presented in a later section.

Undifferentiated Sandy Till

In the eastern portion of the area the lacustrine sands are overlain by a deposit which is apparently a sandy and in places silty till. Only limited test hole data is presently available in this area, making correlation of this unit to the western portion of the area impossible.

Lacustrine Sand Unit

The lacustrine sand unit consists of up to 60 feet of fine to medium-grained sand in the northeastern portion of the area (Fig. 15). In the lower portions, several interbeds of silt and clay, often containing angular carbonate rock fragments. The lacustrine sand unit is overlain by the clay-loam till, although its stratigraphic relationships to the basal sandy drift are not clear.

The results of Hvorslev response test indicate hydraulic conductivities in the range of 1×10^{-7} to 1×10^{-5} fps, hydraulically similar to the basal sandy drift.

Precambrian Bedrock

Underlying the glacial drift in the WNRE area are granite and granodiorite of Precambrian age (Templeton, 1960). The bedrock contact defined in Figs. 15 - 17 has been assumed to occur within a foot or two of the maximum depth penetrated at

each of the test holes drilled to refusal. This assumption is supported by the results of a geophysical investigation of the Environmental Control Area by Pakiriah (1970), by the relative uniformity of penetration depths within most portions of the area. The similarity of the steady-state mathematical model and the field data also support this location for the impermeable lower bounding surface.

Templeton Engineering Company Limited (1960) concluded on the basis of four diamond-drill holes in the southern half of section 21 that a deposit of "broken and weathered granite bedrock rather than glacial till" rests on the solid bedrock surface. Results of the present investigation indicate that this is a misinterpretation of the deposit here referred to as the basal sandy drift.

Although the overall bedrock surface in the Environmental Control Area has been defined to be gently rolling, local relief of over 20 feet over a horizontal distance of 10 feet was observed at tritium injection site No. 2. McPherson (1970) noted local relief on the bedrock surface in drift covered areas near Pinawa, Manitoba in excess of 100 feet.

The bedrock is assumed to have negligible permeability. Examination of outcrops near WNRE and the steady-state mathematical model support this assumption.

FIELD STUDIES OF GROUNDWATER FLOW

The objective of this portion of the investigation was to define the geohydrologic regime in the Environmental Control Area by interrelating studies of fluid potential distribution, hydrochemistry, subsurface temperature variations, groundwater flow velocities and fracture flow characteristics.

Groundwater Flow Pattern

A piezometer network of 210 piezometers and water-table observation wells was utilized to establish the hydraulic potential distribution (Figs. 26 - 28) and the magnitude of transient head variations generated within the system by water-table fluctuations. Representative piezometer hydrographs (Figs. 8 - 10) illustrate the response to seasonal precipitation and indicate short term reversals in hydraulic potential gradients in the central discharge area due to water-table fluctuations. A corresponding increase in recharge head is observed in the central recharge area. These transient effects would not be expected to affect the flow pattern as the condition is short-term and rapidly returns to the original gradient direction.

Data used for preparation of flow diagrams (Figs. 26 - 28) were taken from piezometer hydrographs (Figs. 8 - 10 and Appendix C) as an average value for 1969 and 1970. Values for 1968 were discarded due to abnormally high rainfalls which resulted in highly fluctuating values (Appendix C).

The flow pattern interpretations (Figs. 26 - 28) were made according to the methods of van Everdingen (1963) to account for a 10:1 vertical exaggeration and Freeze (1969) to approximate refractions of equipotential lines at permeability field boundaries defined by test drilling. The potential distribution indicates two areas of recharge referred to as the upland and the central recharge areas and two areas of discharge referred to as the central and lowland discharge areas (Fig. 29). The upland recharge area occurs within the outcrop area of the lacustrine sand and gravel. The lowland discharge area is confined to the banks of the Winnipeg River. Vertical discharge gradients in the central discharge area have a maximum of 0.3 ft/ft with a head above the average water-table level of up to 12 feet and vertical recharge gradients in the central recharge area have a maximum of 0.2 ft/ft. Horizontal potential gradients are restricted to the basal sandy drift and the lacustrine sand with a local horizontal gradient existing in a narrow zone along the boundary between the central discharge and central recharge areas.

A flow pattern interpretation (Fig. 28) along a north-south cross section along the western boundary of Section 28 indicates recharge to the basal sandy drift except near the northern edge of the study area. The sandy drift unit pinches out and small discharge gradients are present.

The hydraulic potential distribution suggests two scales of flow regimes in the Environmental Control Area. The smallest scale 'local' flow systems consists of recharge in the upland recharge area and discharge in the central discharge area, and recharge in the central discharge area discharging into the Winnipeg River (Fig. 29).

The larger scale 'regional' flow system consists of recharge in the upland recharge area and discharge into the Winnipeg River with lateral ground-water flow continuing under the central discharge area. This co-existence of flow systems has been described by Meyboom (1962) and Tóth (1966), as well as others.

The seasonal stability of recharge-discharge boundaries was studied in detail by installing a line of piezometers at closely spaced intervals across the central discharge-central recharge area boundary in section 28 (Fig. 29). During the period of study from summer, 1968 to fall, 1970, this boundary was not observed to vary significantly. On the basis of these data, it has been assumed that the recharge-discharge boundaries are stable with respect to seasonal fluctuations of precipitation, evapotranspiration, and freezing.

Hydrochemistry

The major-ion chemistry of groundwater in a shallow flow system results from numerous geochemical processes. The results of studies by such investigators as Meyboom (1962), Tóth (1966), Parsons (1967), Rozkowski (1967), Charron (1969), van Everdingen (1970), and Cherry, et al (1971), suggest that the most important geochemical influences are: (a) generation of gases in the soil zones of recharge areas, (b) solution of porous media minerals, (c) precipitation of mineral phases within the pore network, (d) ion exchange between the pore solution and clay minerals in the porous medium, and (e) oxidation-reduction reactions. These processes are interdependent and are affected by mineral associations in the porous media, the relative order of geologic units along the groundwater flow path

rate of groundwater flow and the length of the groundwater flow path. Using these guidelines, the major-ion distributions and electrolytic conductivity in the groundwater regime in the Environmental Control Area were investigated as a means of refining the flow system interpretation based on the hydraulic potential distribution.

Hydrochemical cross sections (Figs. 30 and 31) were prepared for Ca^{++} , Mg^{++} , Na^+ , Cl^- , HCO_3^- , CO_2^- and SO_4^- and electrical conductivity and exhibit hydrochemical patterns which correlate very closely with the flow pattern interpretations obtained from hydraulic head data (Figs. 26 - 28). In recharge areas the conductivity increases downwards to a maximum of 600 millimhos/cm. In the central discharge area conductivity increases from a minimum of 10 millimhos/cm near the Precambrian bedrock to greater than 680 millimhos/cm near the water table. A complex hydrochemical pattern is developed in a zone along the boundary of the central recharge and central discharge boundary (Figs. 30 - 32). This is probably due to a sluggish flow regime with long groundwater residence times resulting from low hydraulic potential gradients coupled with low permeability material, as well as short-term reversals of groundwater potential distribution west of this zone.

Groundwater in the lacustrine sand deposit and the basal sandy drift up to the central recharge-central discharge area boundary has very low concentrations of dissolved solids. Samples taken from shallow piezometers in the upland recharge area indicate that essentially all the dissolved solids are picked up in or near the soil zone. The subsequent lack of major variations in the dissolved mineral load in these deposits is due to the low remaining H_2CO_3 aggressivity as well as the lack of

relatively soluble salts in the predominantly quartzo-feldspathic sand. The more permeable deposits would also tend to allow a more active flow regime with resulting shorter residence time and therefore fresher water.

West of the central discharge-central recharge area boundary in the basal sandy drift, conductivity increases over several hundred yards to a maximum of 450 millimhos/cm. This rapid increase results from downward influx of highly mineralized water from the upper units, possibly enhanced by increased availability of fine-grained soluble materials in the drift unit.

In the lacustrine clay and clay-loam till units in the central recharge area, downward seepage is accompanied by increases in Na^+ , Ca^{++} , Mg^{++} , Cl^- , and $\text{SO}_4^{=}$. In the central discharge area, upward seepage is accompanied by similar increases. This is attributed to the availability of soluble sulphate and chloride salts in the fine grained units.

The hydrochemical patterns (Figs. 30 and 32) indicate that precipitation infiltrates in the lacustrine sand and moves downward and laterally with dissolved solids remaining below 50 mg/l. A portion of the water in the central discharge area is forced upwards into the overlying upper till and lacustrine deposits with total dissolved solids reaching 200 mg/l near the water table.

Water infiltrating in the central recharge area rapidly picks up a maximum of 6000 mg/l dissolved solids as it descends to the basal sandy drift unit where it is somewhat diluted by the fresher westward lateral flow from the upland recharge area. The combined flows travel over the Precambrian surface to discharge in and near the Winnipeg River.

Field pH data have been obtained (Appendix D) and will be incorporated in a future report dealing more intensively with the hydrogeochemical regime in the WNRE area.

Groundwater Temperature Distribution

Groundwater temperature variations with depth and time were studied during the spring and summer of 1969. Typical variations (Fig. 32 and Appendix E) indicate that seasonal temperature variations affect the groundwater temperature through the full section of glacial drift at WNRE.

There is an apparent correlation of the range of temperatures and discharge-recharge zones. From graphical results (Appendix E) of 13 piezometer nests in discharge areas and 8 nests in recharge zones it was found that the mean of groundwater temperatures were 1.1 C° and 1.6 C° respectively for the first week of July, 1969. At this time the approximate mean groundwater temperature was 5.2° C . Temperature data from water-table observation wells was not included in this test.

Tracer Velocity Studies

To obtain real values of horizontal groundwater flow velocity, tritiated heavy water was injected into the flow system at three selected locations (Figs. 11 - 13) in the Environmental Control Area. Injection site 1 (Fig. 11) was installed in the basal sandy drift in the central discharge area. During the injection process the tracer was observed to arrive at sample points 13 and 15 located approximately 8 feet from the injection point. Arrivals of the tracer at other sample points located

8 to 48 feet from the point of injection were not observed during the period July, 1969 to August, 1970 although fluctuations in tritium concentration were noted in the active sample points. The movement of the tritium is confirmed by the gradual decrease in tritium concentration (Appendix F) at the injection point. Hydraulic connection of all sample tubes to the injection point was confirmed prior to injection of the tracer by pressure testing.

The tracer is known to have been successfully injected into the groundwater flow regime in the basal sandy drift. The data show that the lateral rates of groundwater movement have not been sufficient to move the tracer completely away from the area of injection. Hydraulic potential data require that the tracer front move westward and it is very unlikely that the tracer front could move undetected between the sample tubes to the west. Since sample tubes are present from 8 to 15 feet west of the injection site, it can be safely assumed that west-lateral groundwater velocities are considerably less than 10 feet per year at this location.

Injection site 2 (Fig. 12) was installed in the basal sandy drift in the central recharge area. Arrivals of the tracer during injection was observed in sample tubes 26 - 43, 1 and 4. Arrivals of the tracer at other sample tubes located 8 to 58 feet from the point of injection were not observed during the period August 1969 to August, 1970, although decreases in tritium concentrations were observed in the active sample tubes and the injection well. The procedures and results were very similar to those observed at injection site 1. This indicates a maximum horizontal flow velocity of less than 10 feet/year.

Injection site 3 (Fig. 13) was installed adjacent to injection site 2 in the central recharge area and bracketed the contact of the lacustrine clay and the clay-loam till units. Arrival of the tracer was observed in sample tube 6 during injection. Arrivals of tritium at sample tubes 8 and 48 feet from the injection point were not observed during the period August, 1969 to August, 1970. However, very low tritium levels did occur sporadically in various sample tubes, probably due to contamination during sampling. Similarly to injection sites 1 and 2, this indicates maximum westward horizontal flow velocities of less than 10 feet/year.

The experiment was designed to detect horizontal flow velocities and was not capable of detecting vertical flow. The sampling and injection methods resulted in a large mass of tritiated water (Figs. 11 - 13) which, if moving horizontally, would necessarily pass through at least one sample point.

The possibility of hydraulic fracturing due to the pressure injection method would increase the flow velocity because of increased permeability and would not have caused an anomalously low value for the flow rate. Movement of the tracer was confirmed by fluctuating tritium levels in active holes (Appendix F).

A previous tracer study in 1967 by B. Lund (1967) gave a horizontal flow velocity of 430 feet per year. The study was conducted from surface to a depth of 20 feet in the centre of section 28 (Fig. 3) in an area of extremely low lateral hydraulic potential gradients. The velocity determined by here is now thought to be due to flow in the unsaturated zone as a result of a groundwater 'mound' built by the injection of tritiated heavy water rather than flow through the saturated zone.

Tracer Investigations of Secondary Permeability Characteristics

Joints in the lacustrine clay and clay-loam till units were observed in Shelby tube cores and disturbed samples. In an attempt to define the effects of the joints on permeability in the fine-grained deposits, a fluorescent dye, sodium fluorescein was injected into these units and then Shelby tube cores were taken of the injected material.

Examination of cores under ultraviolet radiation showed no visible dye on joint surfaces or in the bulk of the material. The most permeable joints are coated with carbonate accumulations and the coating may mask the presence of the dye. Intergranular flow of groundwater in the clay unit and upper carbonate till unit is probably negligible with respect to flow through joints. Photographs of fractures (Figs. 19 - 25 and Appendix A) support this view.

Further study on the problem of quantitative determination of fracture flow is being considered. This aspect is very important to radioactive waste disposal as ion exchange capacities of a geologic medium will be drastically reduced if waste solutions do not have access to intergranular pore spaces.

Hvorslev Response Tests

Of the 210 piezometers installed in the area of investigation, 170 were subjected to response tests based on the principles and methods described by Hvorslev (1951; 1954). The ratios of vertical versus horizontal permeabilities were assumed unity for all the hydrostratigraphic units. Data summarized by Davis and DeWiest (1966) and Bakhtiari (1970) suggest that this assumption is normally acceptable

within 50 per cent for sandy unconsolidated deposits not containing fine-grained interbeds or oriented platy mineral orientations. The K_v/K_h ratios for the fine grained deposits were assumed to be unity due to the relatively orthogonal orientation of joints and laminae prominent in these deposits, although the actual value of K_v/K_h was not determined.

Hydraulic conductivities obtained from water-level response tests can be affected by: (1) water-intake geometry, (2) textural variations within the intake zone, (3) disturbances of the formation during piezometer installation (4) nature and efficiency of the grout seal above the intake zone, (5) gas accumulations in the intake zone, (6) any artificial restriction or enhancement of the hydraulic conductivity of the formation or intake zone resulting from piezometer installation. Because of these and other factors involved in water-level response tests, the resulting hydraulic conductivities are likely not reliable to more than one order of magnitude.

Figures 33 and 34 summarize the permeabilities calculated from the water-level response tests. Data which appear to have been significantly affected by one or more of the above factors have been omitted. The exclusion of results was usually based on obvious evidence of sedimentation or clogging or poor graphical correlations of water-level recovery with time.

All of the 7 piezometers in the lacustrine sand unit from which reliable water-level response data were obtained yielded hydraulic conductivities in the range 1×10^{-7} to 1×10^{-4} fps. Of the 34 piezometers in the basal sandy drift from

which reliable data were obtained, 22 yielded values in the range 1×10^{-7} to 1×10^{-5} . The remainder of the conductivities are in the range 1×10^{-7} to 5×10^{-10} . Auger samples from these units have been compared visually to glacial sands from southwestern Manitoba. The visual textural correlations support the conclusion that conductivities in the range 1×10^{-7} to 1×10^{-5} are characteristic of most zones within the two units.

Of the 36 piezometers in the clay-loam till from which reliable water-level response data were obtained, 20 yielded hydraulic conductivities in the range of 1×10^{-9} to 1×10^{-7} fps. Of the remainder, 9 are in the range 1×10^{-7} to 9×10^{-6} and 9 in the range 1×10^{-9} to 1×10^{-10} fps. There is little doubt that joints in the clay-loam till account for most, if not all, of the hydraulic conductivities larger than about 1×10^{-8} fps and that the wide range of the data reflects heterogeneous characteristics in the permeability system. Variability in the character and frequency of joints in till at numerous locations in western Canada has been observed by A. Vonhof (personal communication, 1970).

Hydraulic conductivities in the lacustrine clay unit vary within the relatively wide range of 1×10^{-10} to 1×10^{-5} fps. Fig. 35 indicates that 10 of the 31 piezometers located in this unit at depths between 5 and 14.5 feet below ground surface yield hydraulic conductivities greater than 1×10^{-7} fps. Of the 14 piezometers in this unit at depths below 14.5 feet, only one yielded a hydraulic conductivity greater than 1×10^{-7} fps. The hydraulic conductivity data suggest that the lower portion of the lacustrine clay and the clay-loam till have rather similar hydraulic properties.

The significantly higher frequency of larger hydraulic conductivities nearer the ground surface can be accounted for by the increased occurrence of joints, desiccation cracks, and channels caused by plant roots and burrowing animals. The seasonal water-table fluctuations in the area underlain by the lacustrine clay is between 2 and 7 feet below ground surface. Decline of the water table below 7 feet probably occurs during years of exceptionally low precipitation. The clay fraction of sediments in the Lake Agassiz basin generally contain abundant montmorillonite (Kushnir, 1970) which characteristically undergoes major volume changes on wetting and drying. The zone of water-table fluctuation is subject to numerous cycles of wetting and drying and is therefore commonly a zone in which desiccation cracks and joints develop.

STEADY-STATE MATHEMATICAL MODEL OF THE FLOW REGIME

The steady-state mathematical model (Morris, 1969) was used to: (a) refine the interpretation of the groundwater flow pattern, (b) test and refine the field interpretation of the subsurface permeability distribution, (c) test the sensitivity of the flow regime to water-table fluctuations, (d) test the assumption of an impermeable bedrock contact and (e) determine if this mathematical model was useful in predicting flow patterns as a method of initial stage planning of hydrogeologic models.

The boundary conditions defining the limits of the model (Fig. 35) are: (a) the bedrock surface is a lower bounding impermeable surface, (b) the lateral bounding surfaces are vertical and coincide with the topographic high of the lacustrine sand to the east and the Winnipeg River to the west, (c) the water table is constant and acts as a free-surface and (d) steady-state conditions exist.

The models tested include isotropic and anisotropic homogeneous, isotropic and anisotropic 2-layer and isotropic and anisotropic 2-layer with variations in permeability within the basal sandy drift and the lacustrine sand and gravel. Most combinations of permeability ratios from one to 10,000 were modelled. In all cases the lacustrine silt unit, lacustrine clay and clay-loam till were considered to have unit vertical permeability.

By using a water-table configuration determined from the observation well network (Fig. 26) a model was developed which very closely approximated the

conditions observed in the field. This model (Fig. 35) was developed using unit permeability for the clay-loam till, clay and silt deposits and a permeability of 1,000 for the sandy drift and lacustrine sand deposits. This model does not, however, simulate the magnitude of recharge gradients existing in the central recharge area, although all other features are very similar.

Studies currently in progress are attempting to determine whether improved correlations between the model and the flow system can be obtained. The modelling technique is also being used in an analysis of cross section B-B' (Fig. 27). The results described above indicate that (a) although from a small-scale viewpoint each hydrostratigraphic unit is heterogeneous in texture and permeability, it is only the large scale permeability contrasts between the units which are the dominant internal factor in the development of the flow pattern, (b) the initial assumption that the bedrock is relatively impermeable is acceptable and (c) the location of the bedrock contact as indicated by the drilling data is reasonable. Although many different conductivities have been assigned to the basal sandy drift and the lacustrine sand, the results obtained from the model using a uniform conductivity continue to be the closest representation of the head distribution.

SUMMARY OF CONCLUSIONS

The stratigraphic section found in the WNRE Environmental Control Area conforms generally to the Pleistocene stratigraphic succession defined by McPherson (1968, 1970).

The glacial drift sequence includes a lacustrine sand and gravel deposit, a late-phase deposit of Glacial Lake Agassiz silts, a deep-water deposit of clay and silty clay, a clay-loam till, a basal sandy drift rich in Precambrian-derived pebbles and boulders, an undifferentiated sandy till, and a uniform lacustrine sand. The glacial drift is underlain by relatively impermeable Precambrian granite and granodiorite.

The groundwater flow pattern is characterized by a westward flow to the Winnipeg River with four zones of recharge and discharge. From east to west these are: an upland recharge area consisting of an outcrop of the lacustrine sand and gravel which act, as a groundwater divide, a central discharge area, a central recharge area and a lowland discharge area confined to the banks of the Winnipeg River which is the groundwater divide to the west. Tracer studies at three locations suggest maximum groundwater flow velocities to be less than 10 feet per year.

Ca^{++} , Mg^{++} , Na^+ , SO_4^{--} and electrolytic conductivity increased in the directions of apparent groundwater movement and correlated with flow directions interpreted from the hydraulic potential distribution. Values increased laterally in the more permeable basal units from the upland recharge area in the east to the Winnipeg River. Values in the till and lacustrine units increase upwards in the

central discharge area and downward in the recharge areas. A more complex hydrochemical pattern has developed along a zone between the central discharge and central recharge area boundary due to stagnant flow conditions. The excellent correlations of hydrochemistry and groundwater flow pattern are due to the relatively active flow regime in the Environmental Control Area, lack of solubility constraint for most major ions, and highly contrasting hydrochemical facies in each geologic unit. Hydrochemistry confirmed flow pattern interpretations which would otherwise have required considerably more detailed instrumentation.

Laboratory studies of cores taken from the lacustrine and upper till units indicate that groundwater flow is occurring mainly along joint surfaces and sand laminae. This is supported by the accumulation of carbonate precipitates on joint surfaces and oxidized zones. Attempts to quantitatively describe joint-fracture flow by means of fluorescent dye tracing was not successful.

A steady-state mathematical model of the groundwater flow regime using boundary conditions defined by test drilling and the hydraulic potential distribution suggests a permeability ratio between the lacustrine clays and clay-loam till deposits and the basal sandy drift and lacustrine sand and gravel units of 1:1000. The model supports the conclusion that the water-table conditions and the discharge-recharge boundaries are relatively stable. The use of this model with preliminary hydrostratigraphic data may allow a more economical approach to simple hydrogeologic investigations.

The complementary use of several hydrogeologic techniques has allowed a relatively detailed interpretation of the shallow groundwater flow system in glacial drift at WNRE. Some of the more general conclusions resulting from the investigation are summarized as follows:

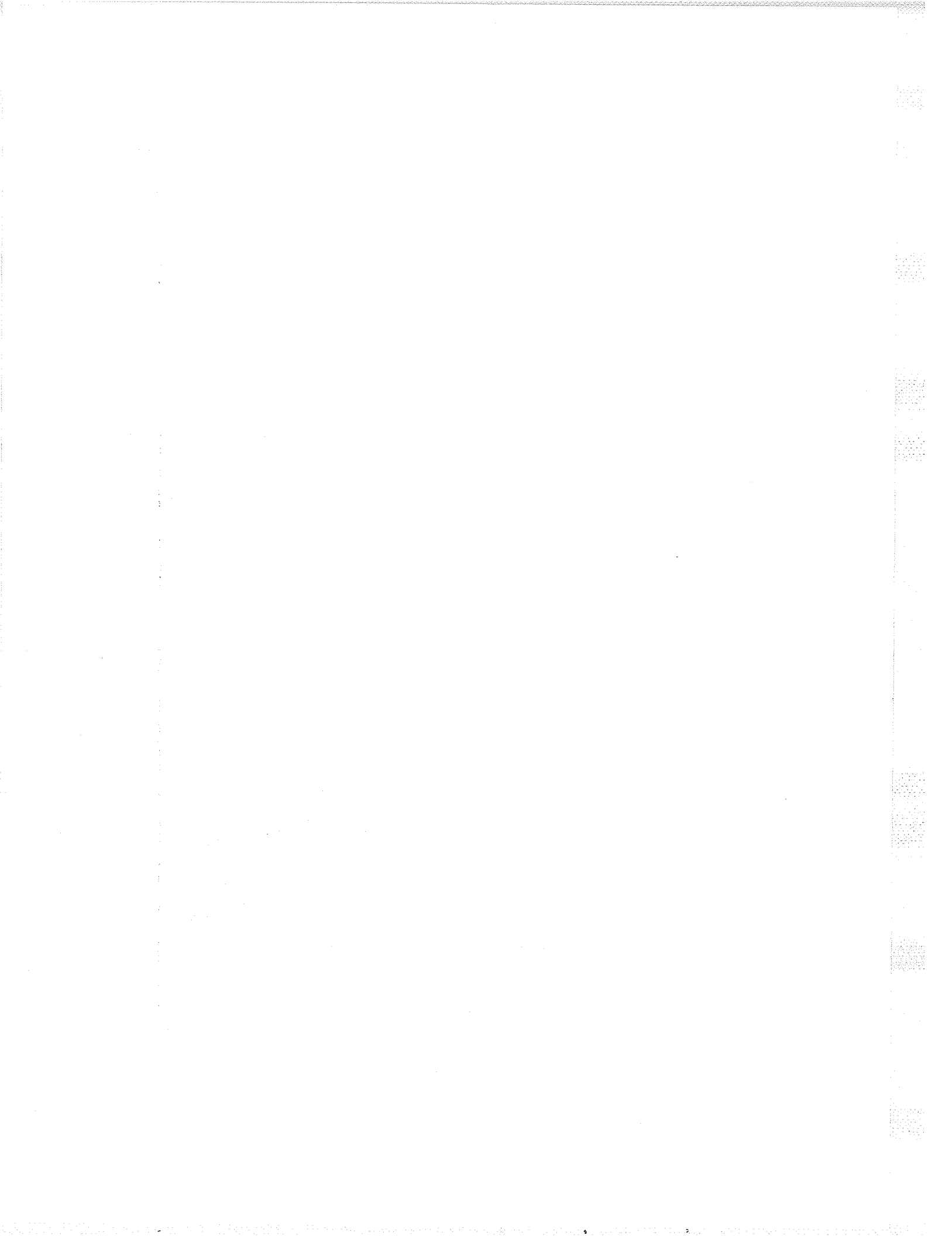
- 1) A comprehensive understanding of the stratigraphic framework is a prerequisite to the design of a piezometer network and the interpretation of the resulting potential distribution.
- 2) Hydrochemical patterns are a result of flow patterns and stratigraphy and may be used in some cases to aid in groundwater flow interpretations.
- 3) Influence of joints and small sand laminae increase hydraulic conductivities of otherwise low-permeable materials.
- 4) A finite-element steady-state model of groundwater flow may refine interpretations of field data and reduce the cost of detailed field study.

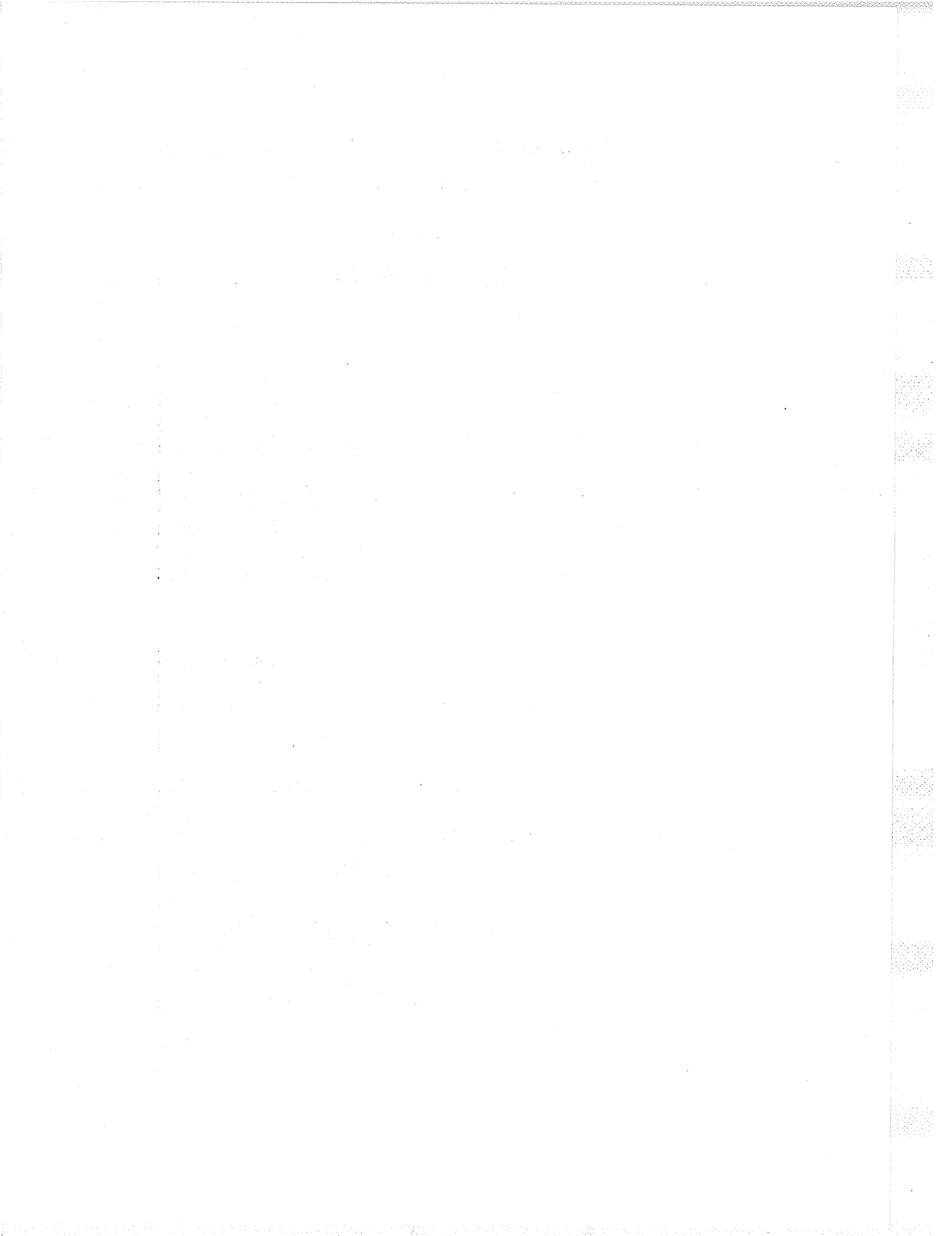
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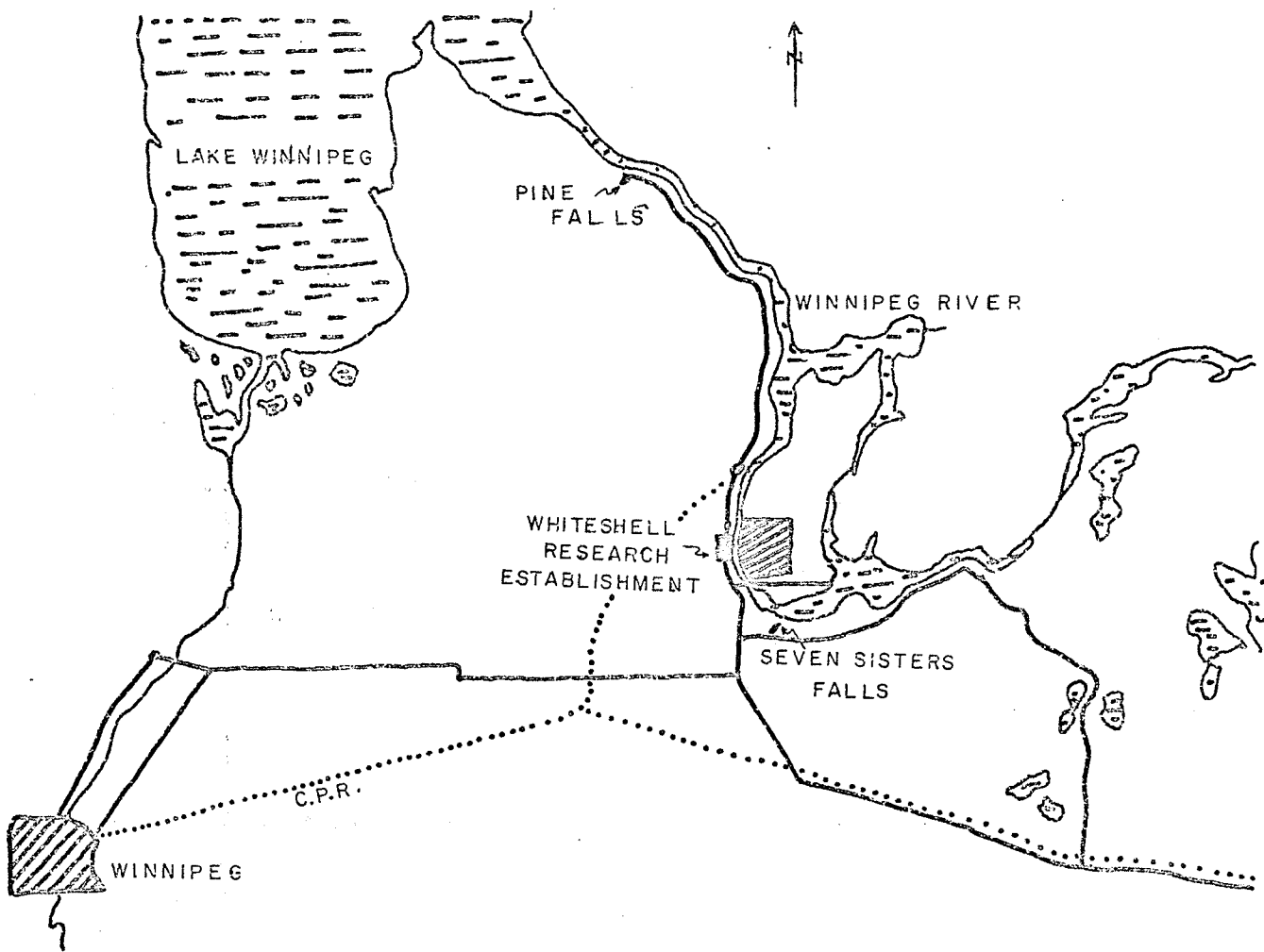


FIG. 1. Vicinity Map-Whiteshell Nuclear Research Establishment

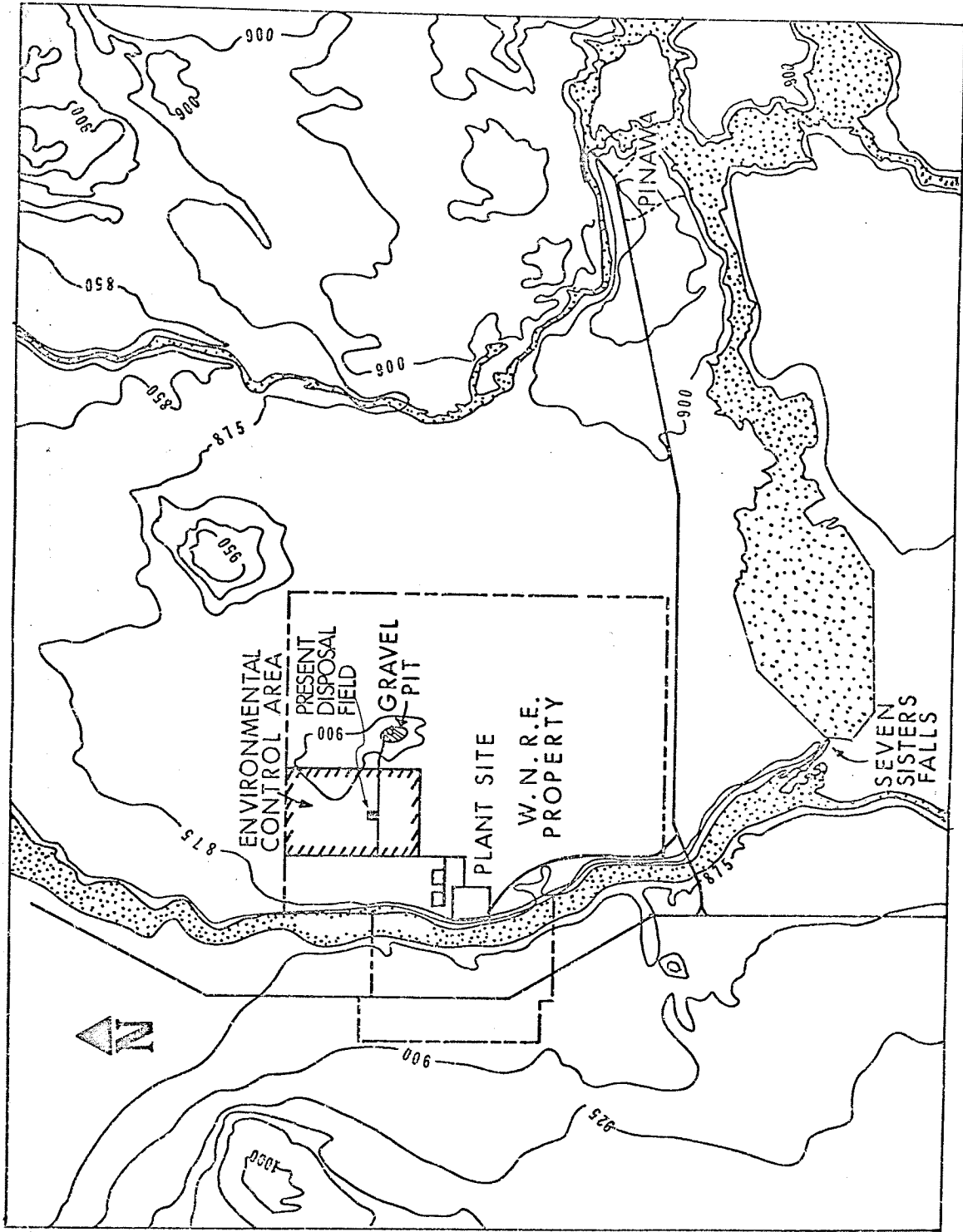


Fig. 2 Environmental Control Area and present radioactive disposal field, WNRE.

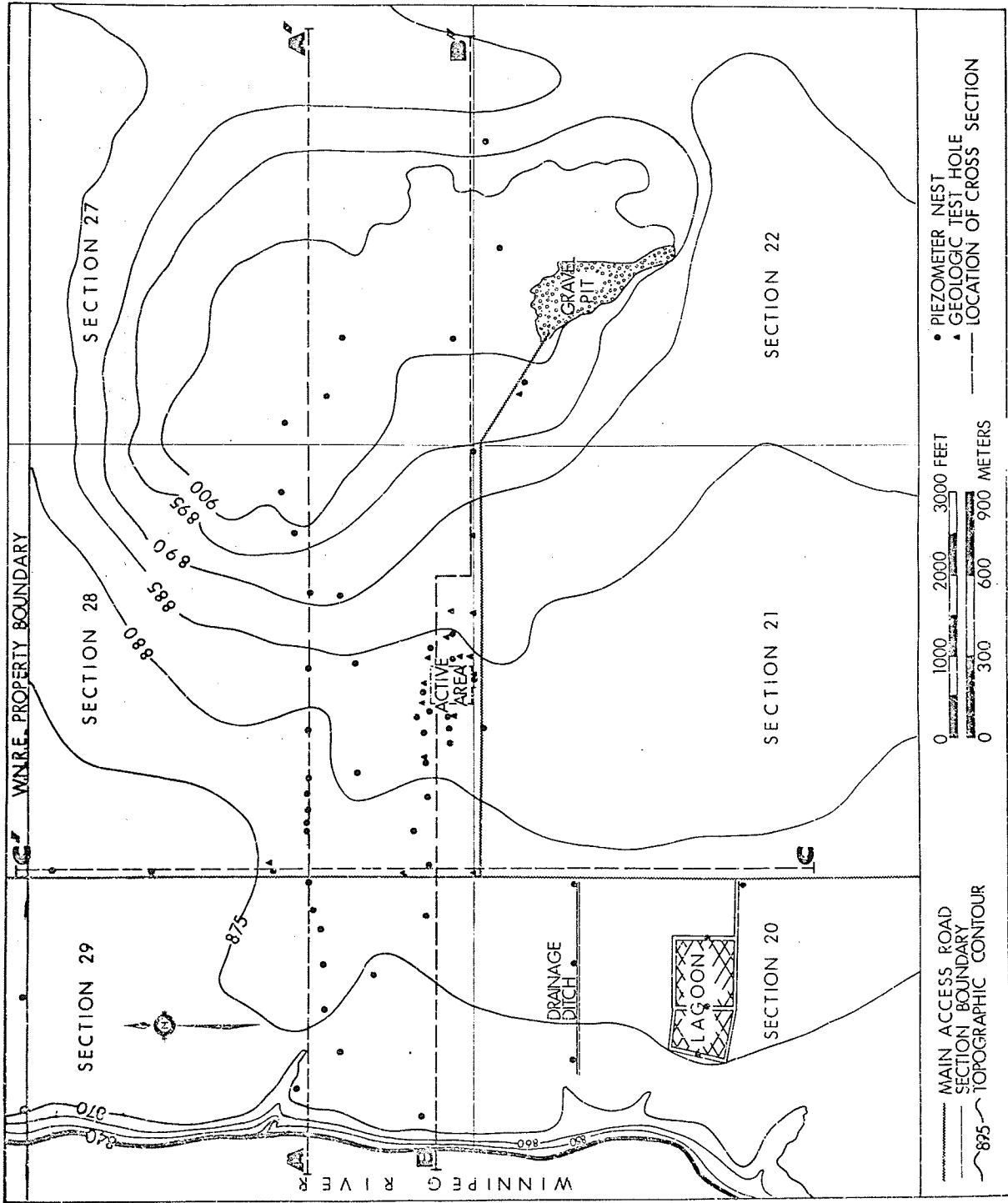


Fig. 3 Topography and location of instrumentation.



Fig. 4 Looking west towards Winnipeg River over portion of Environmental Control Area.

Fig. 5 Modified infra-red photograph of the upland area of recharge.

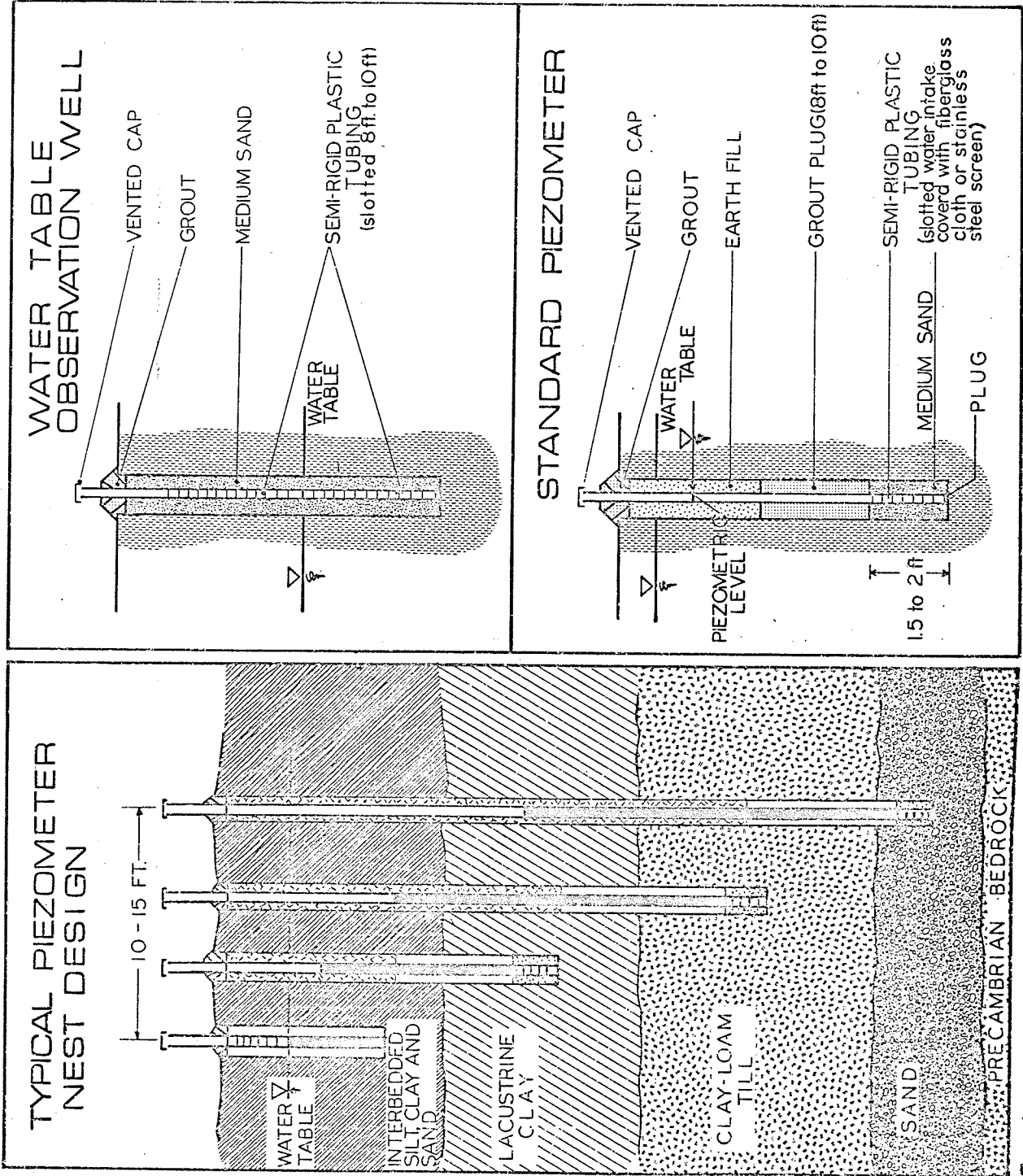
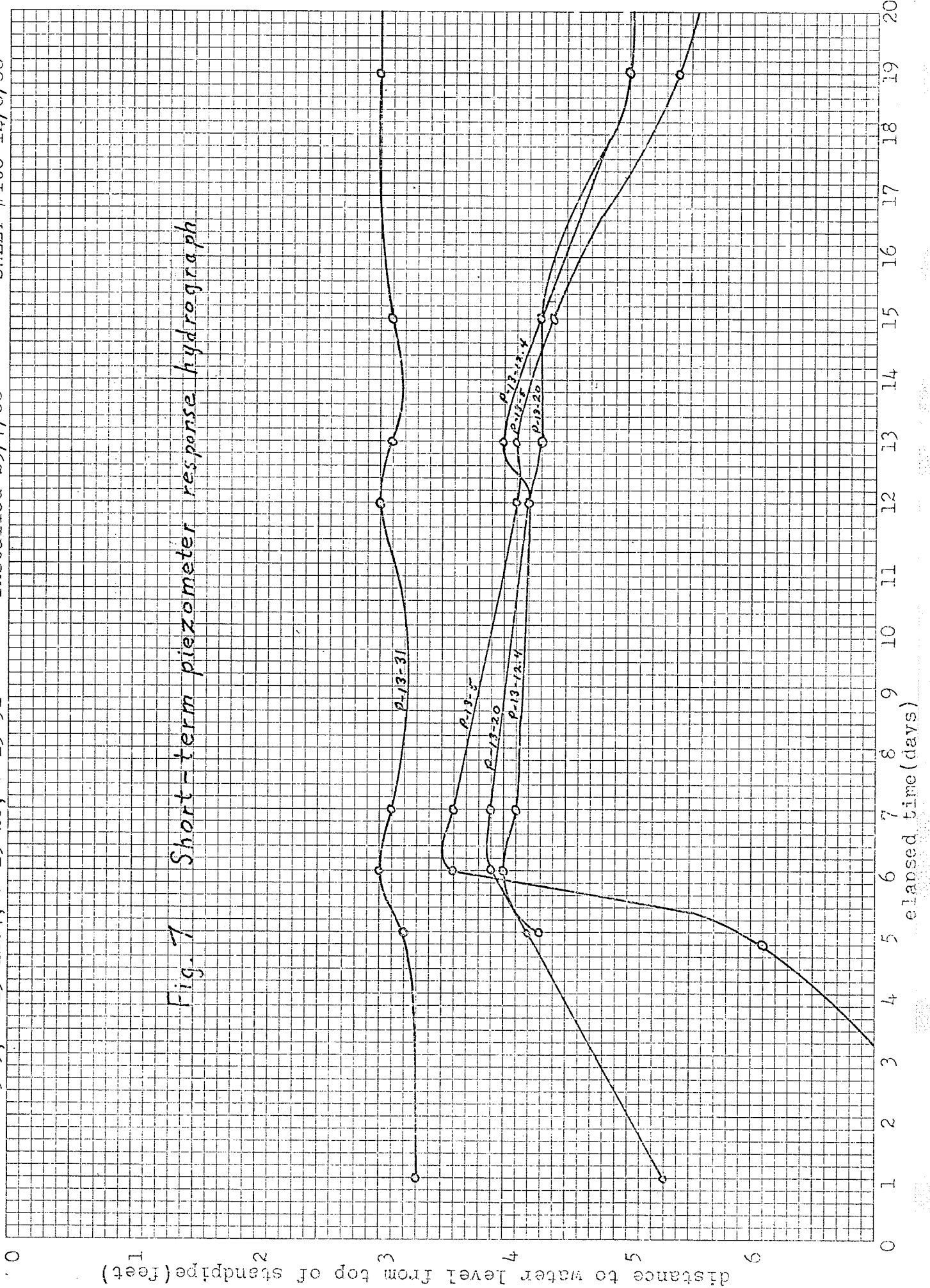


Fig. 6

Fig. 7 Short-term piezometer response hydrograph



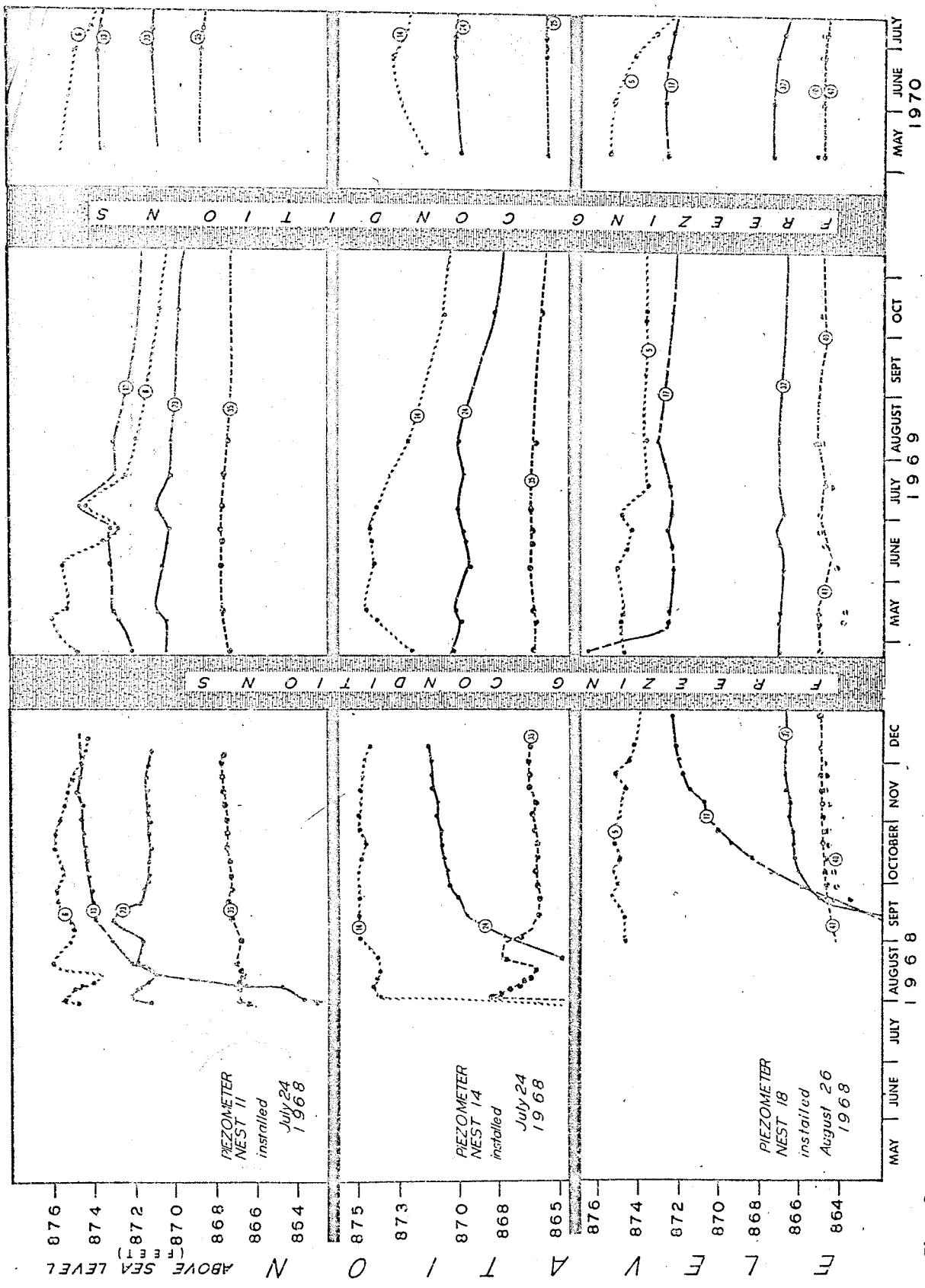


Fig. 8 REPRESENTATIVE PIEZOMETERS IN CENTRAL RECHARGE AREA

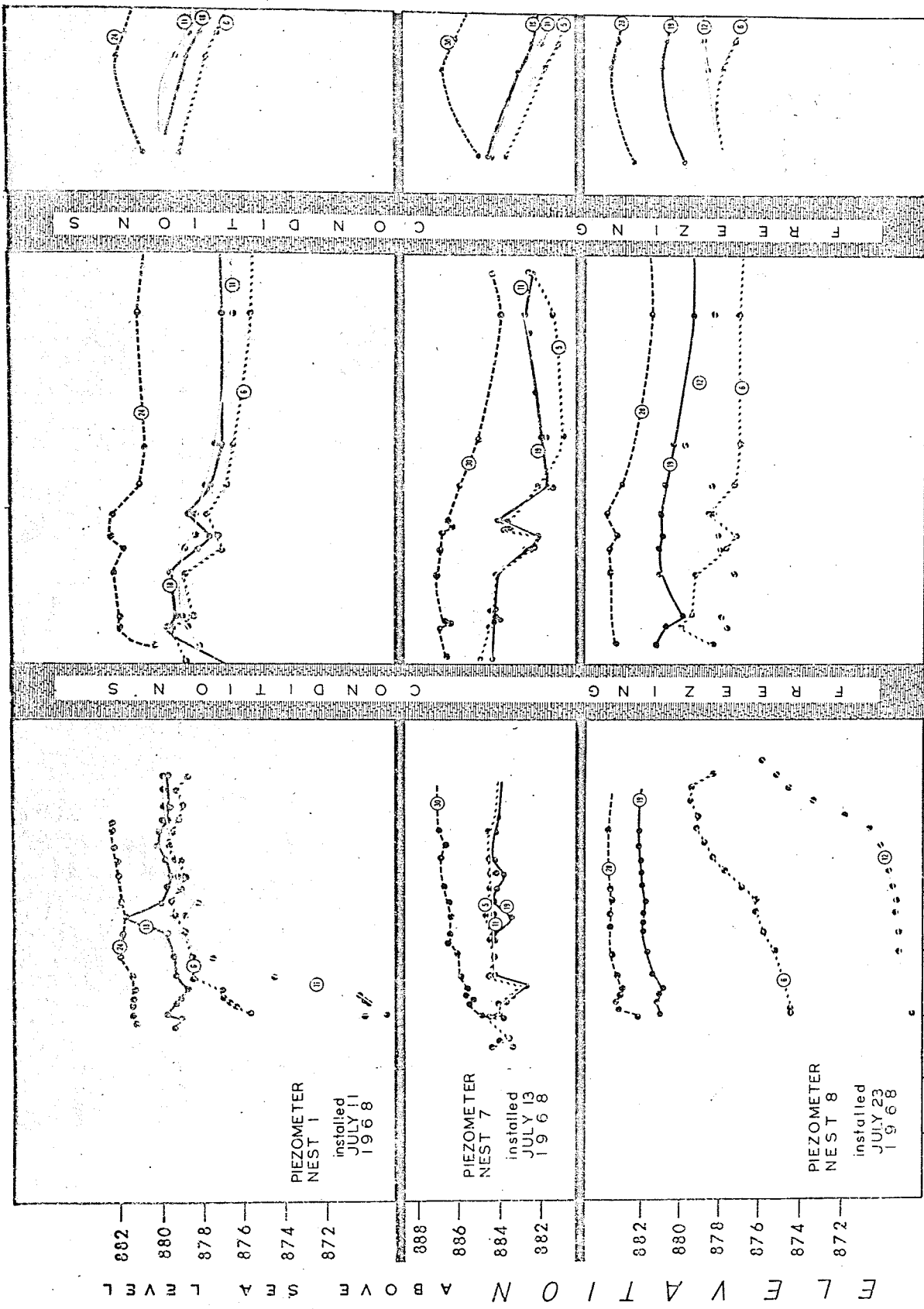


Fig. 9 REPRESENTATIVE PIEZOMETERS IN CENTRAL DISCHARGE AREA

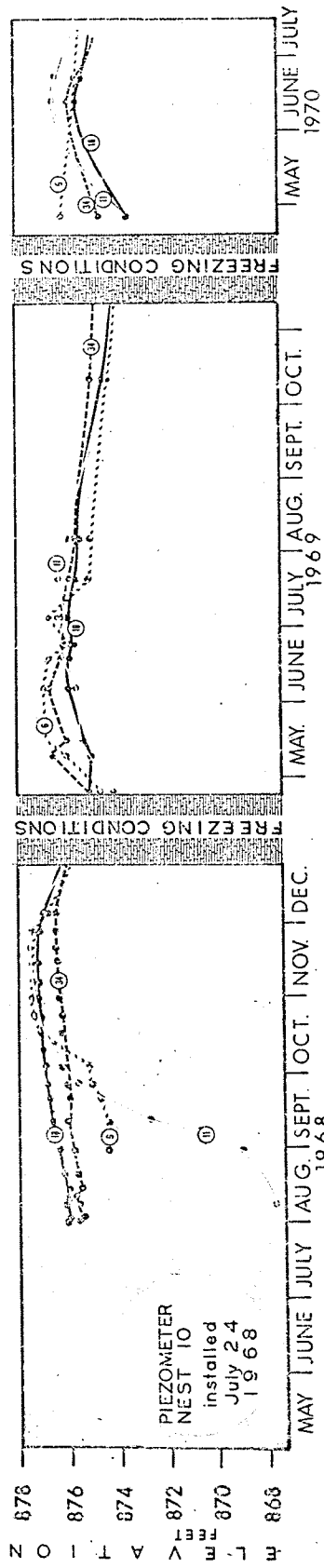
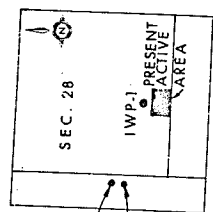
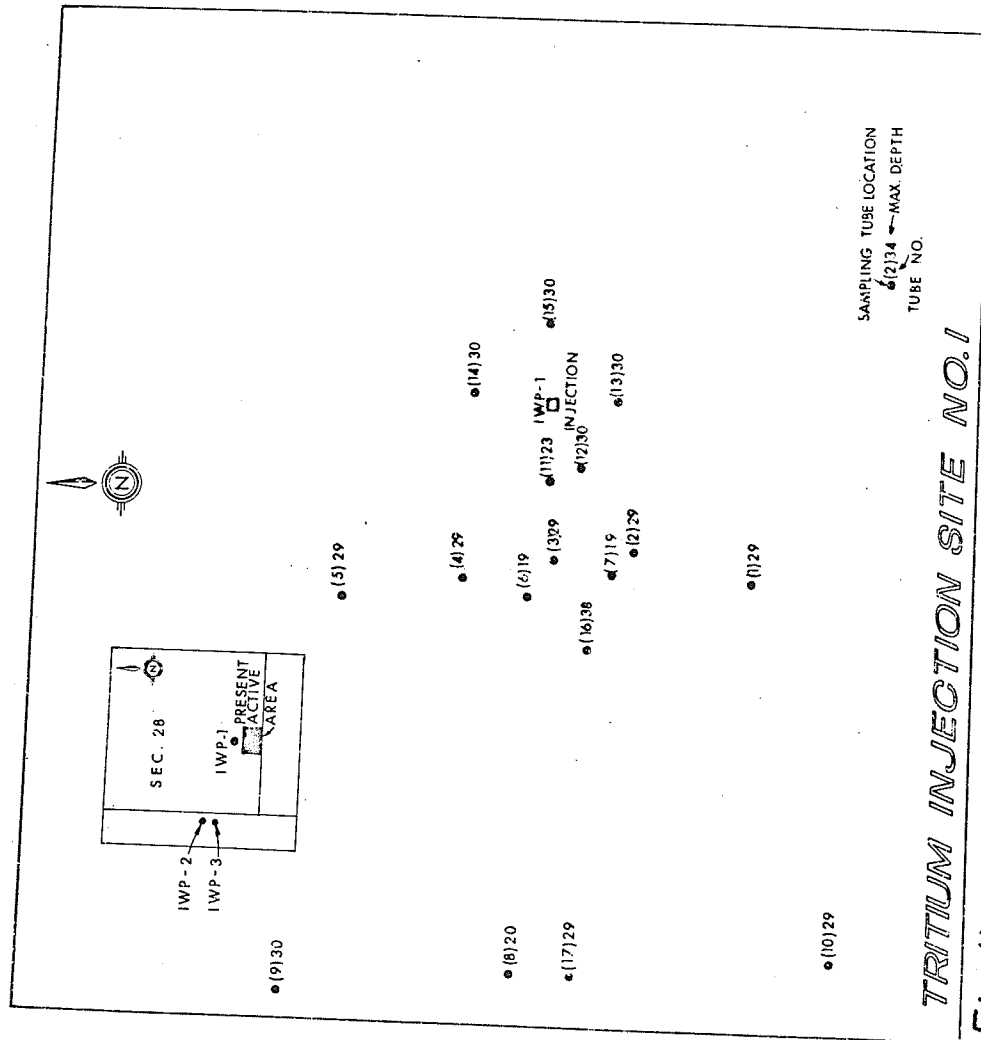


Fig. 10 Piezometer nest in transition zone between central recharge area and central discharge area



• (9)30

• (5)29

• (4)28

• (14)30

• (8)20

• (6)19

• (11)23

• (15)30

• (17)29

• (3)9

• (12)30

• (16)38

• (7)19

• (13)30

• (12)29

• (1)29

• (10)29



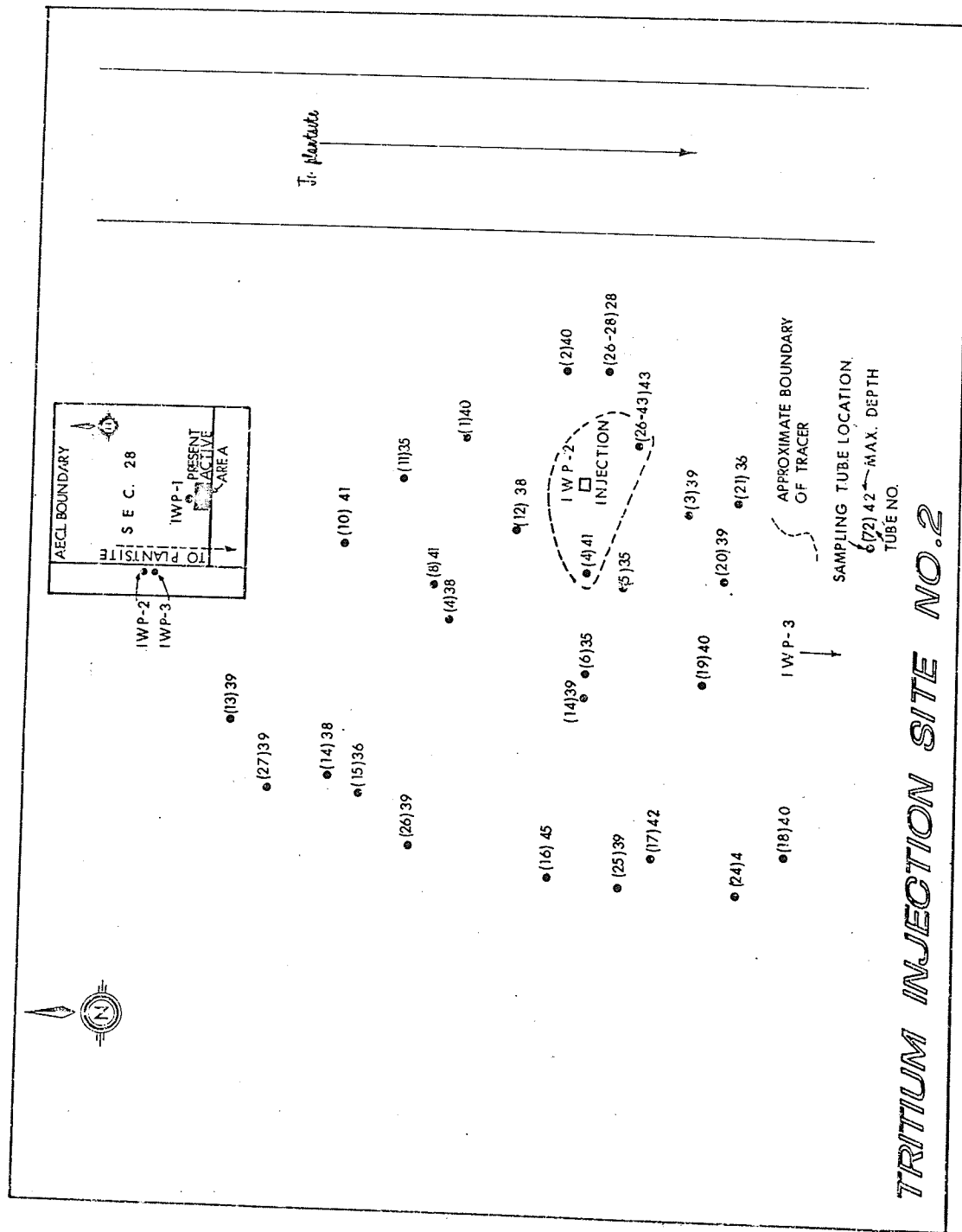


Fig. 12 Layout of tritium injection site No. 2.

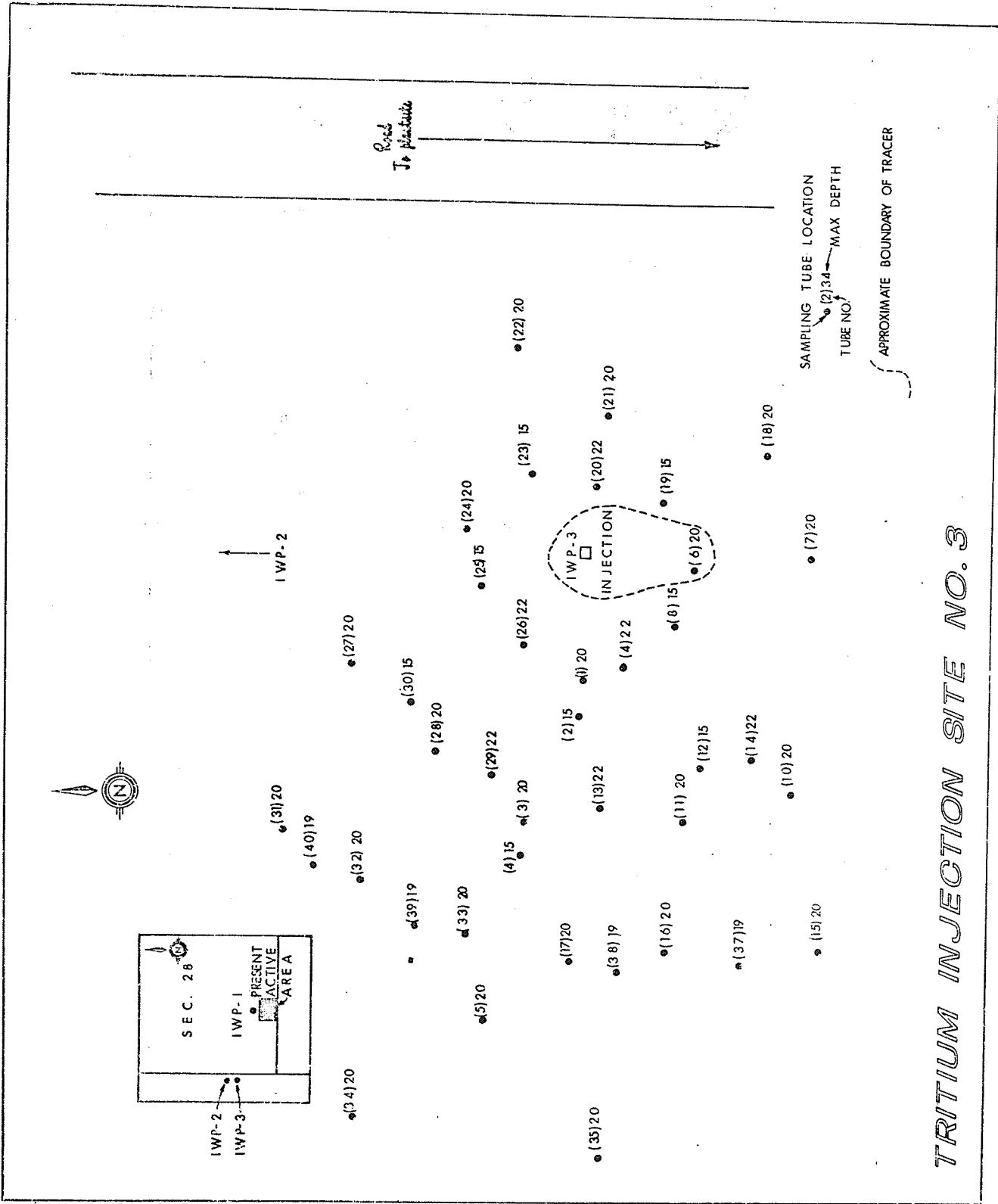


Fig. 13 Layout of tritium injection site No. 3.

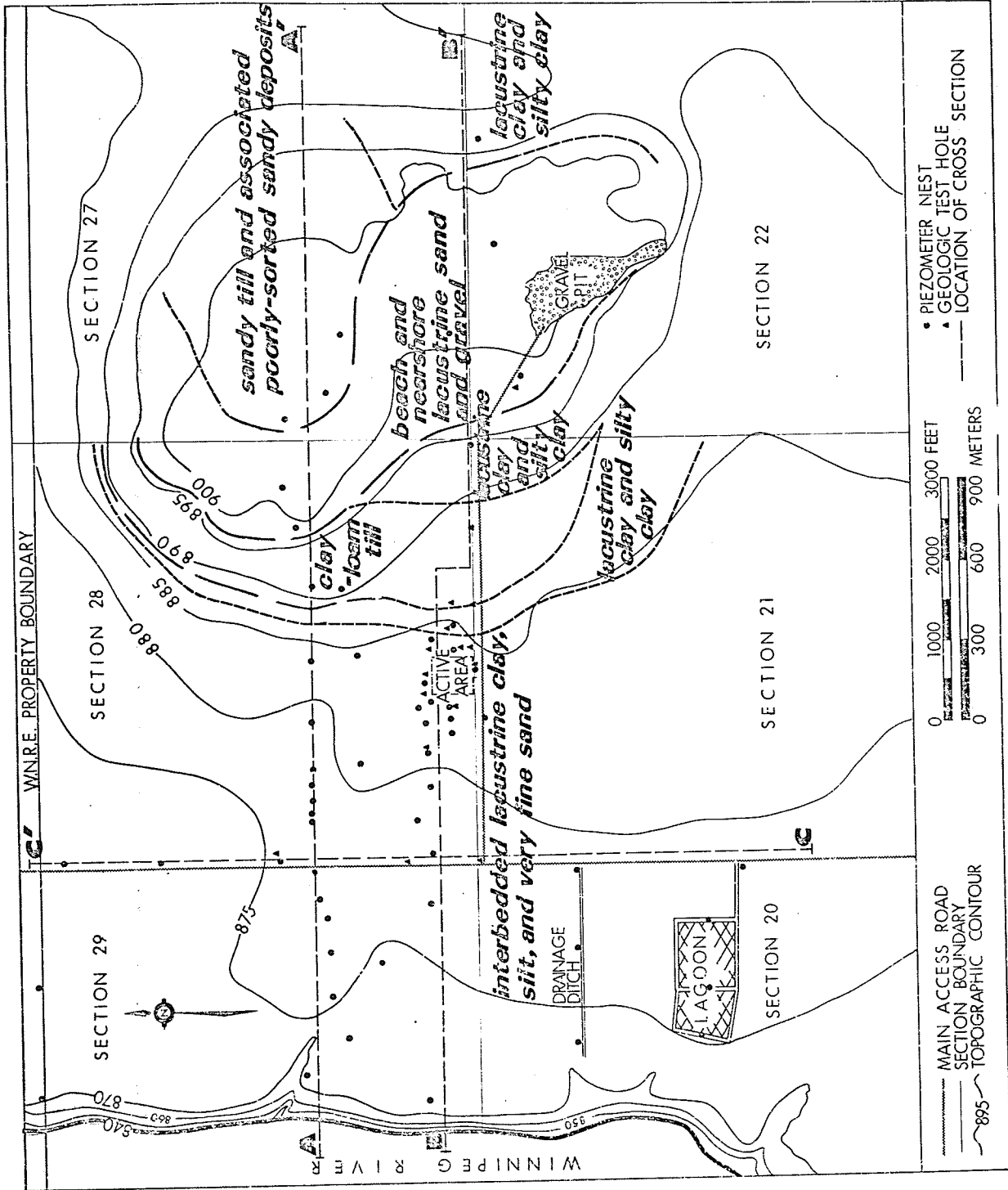


Fig. 14 Surficial geology - Environmental Control Area.

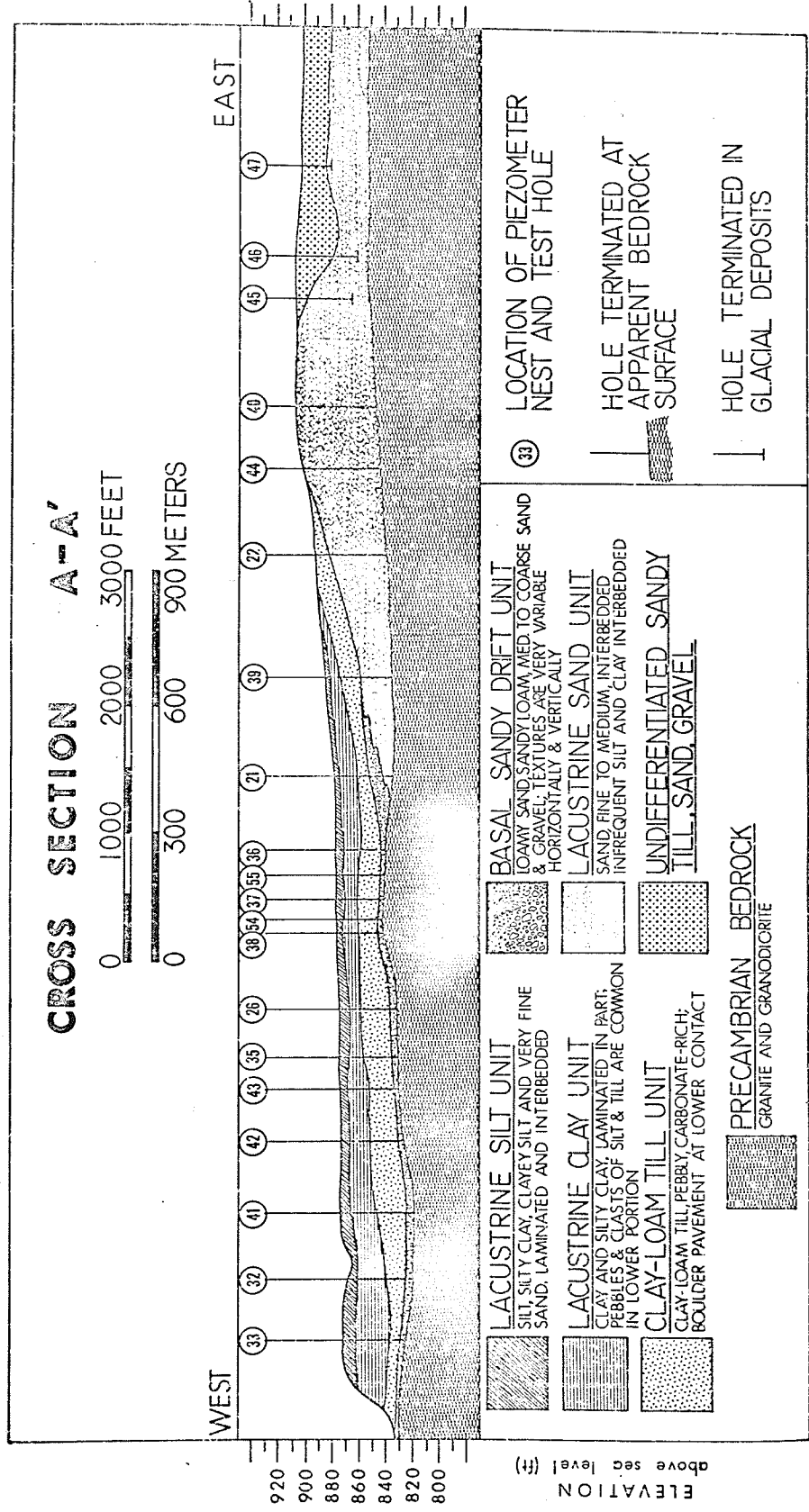


Fig. 15 Geologic cross section A-A'

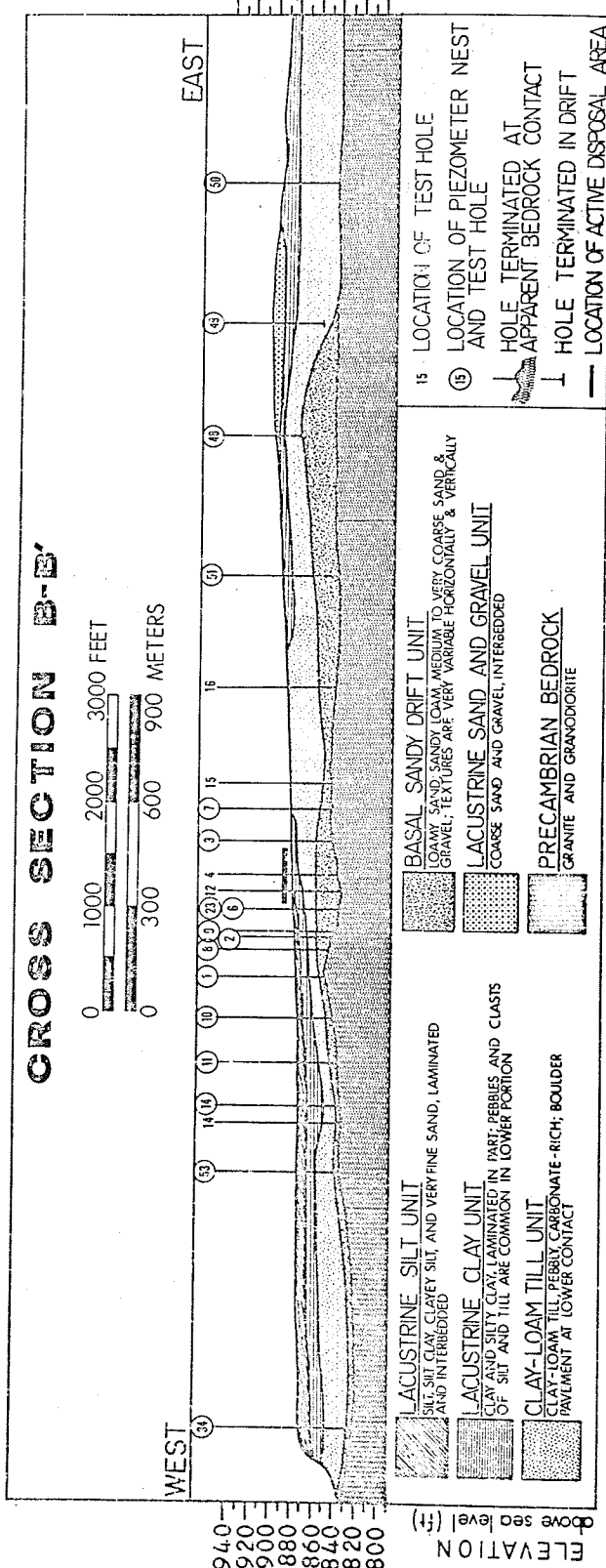


Fig. 16 Geologic cross section B-B'

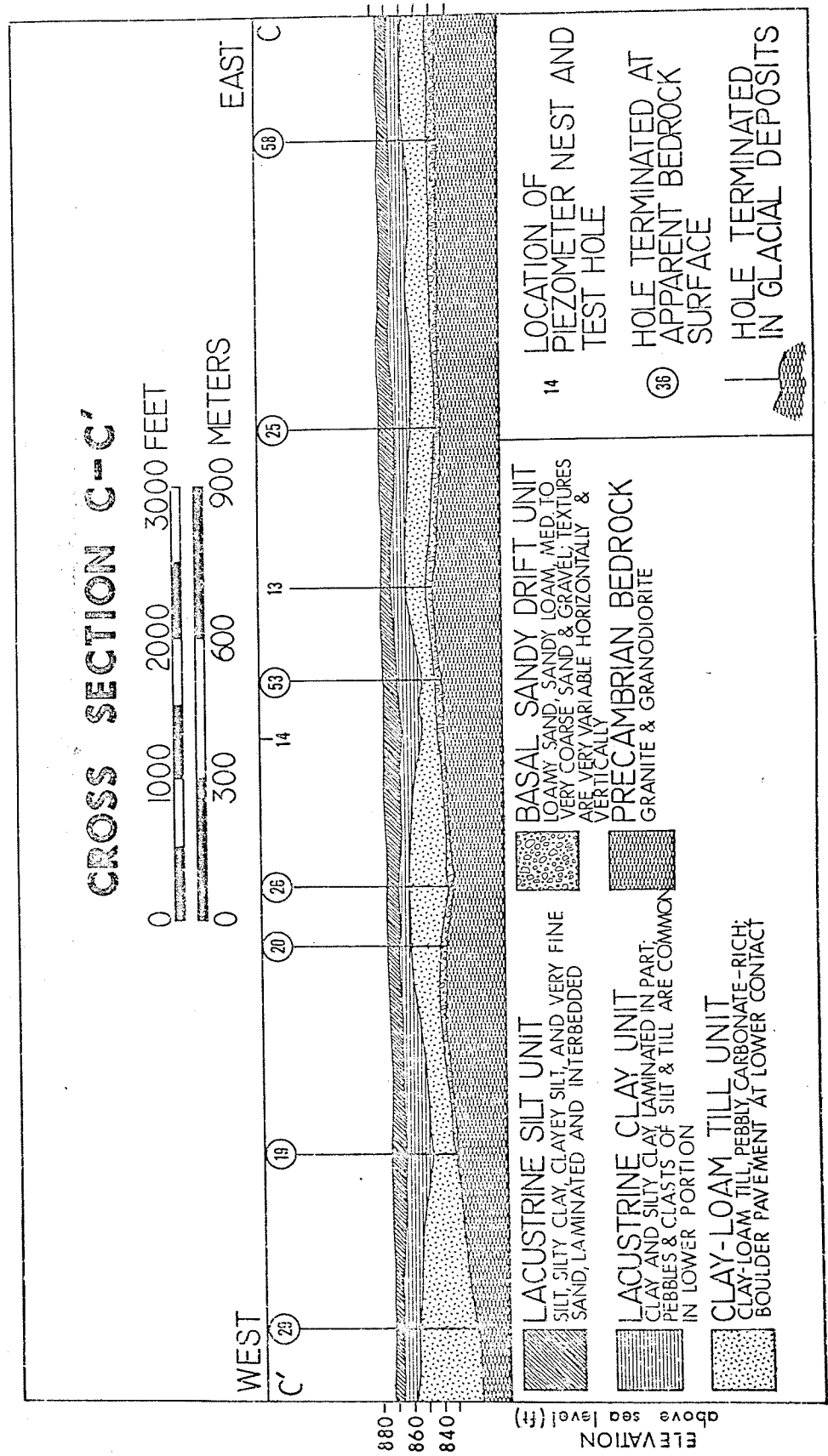


Fig. 17 Geologic cross section C-C'

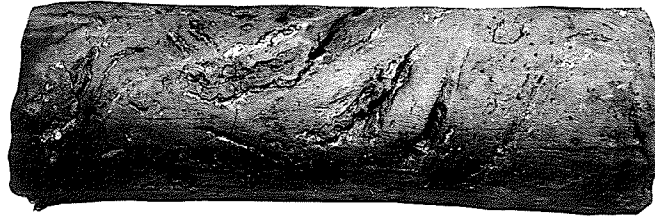


Fig. 10 Soft pockets of easily eroded silt in the upper unit - possibly primary bedding structure. Sample taken from a depth of 5 feet.



Fig. 19 Ice rafted till fragment in lacustrine clay unit. This photo taken at bottom of a 9 foot cut.

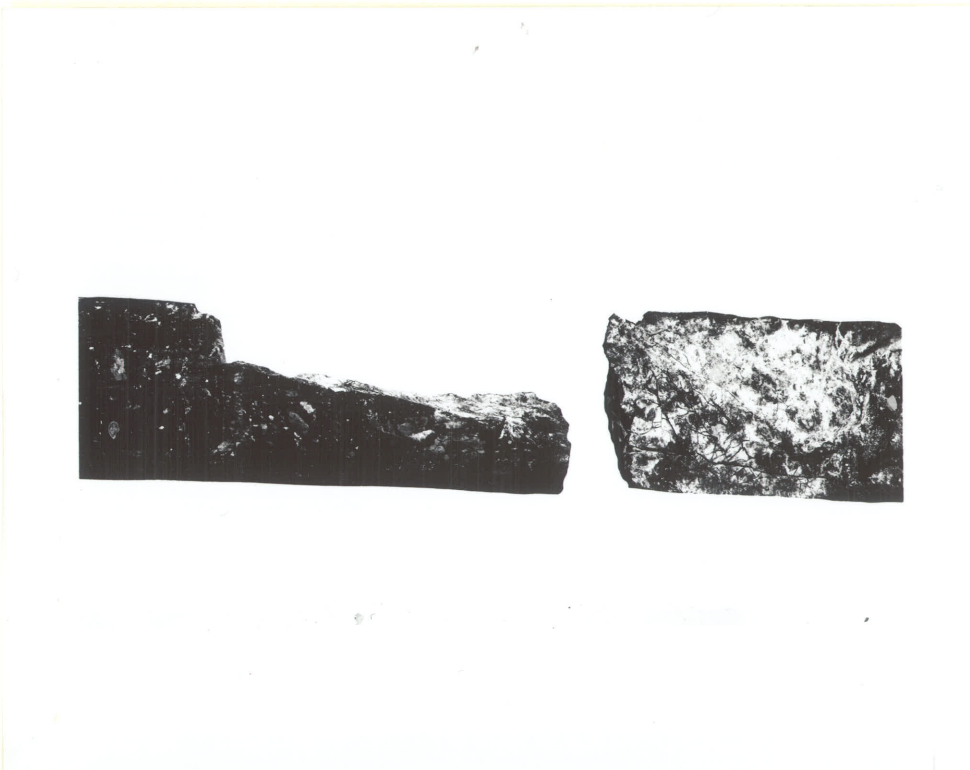


Fig. 20 Angular calcareous silt fragments embedded in lower part of the clay unit - also note precipitates and rootlets on joint surfaces. This sample taken from a depth of 14 feet.



Fig. 21 Steeply dipping laminae of very fine sand in clay unit - carbonate precipitates give a whitish tone to the laminae. Bedding is essentially horizontal.



Fig. 22 Stratified transition zone between lacustrine clay unit and the clay-loam till.

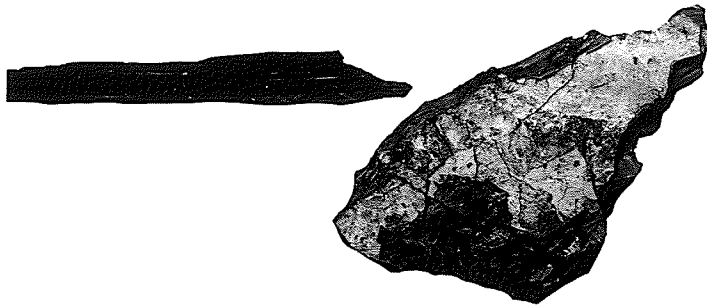


Fig. 23 Joints cutting laminae in clay unit. Joints have been accentuated artificially for illustrative purposes.



Fig. 24 Carbonate and gypsum precipitates on vertical fracture in lacustrine clay unit.



Fig. 25 Carbonate precipitates on joint surface in clay-loam till unit. Sample taken from a depth of 20 feet.

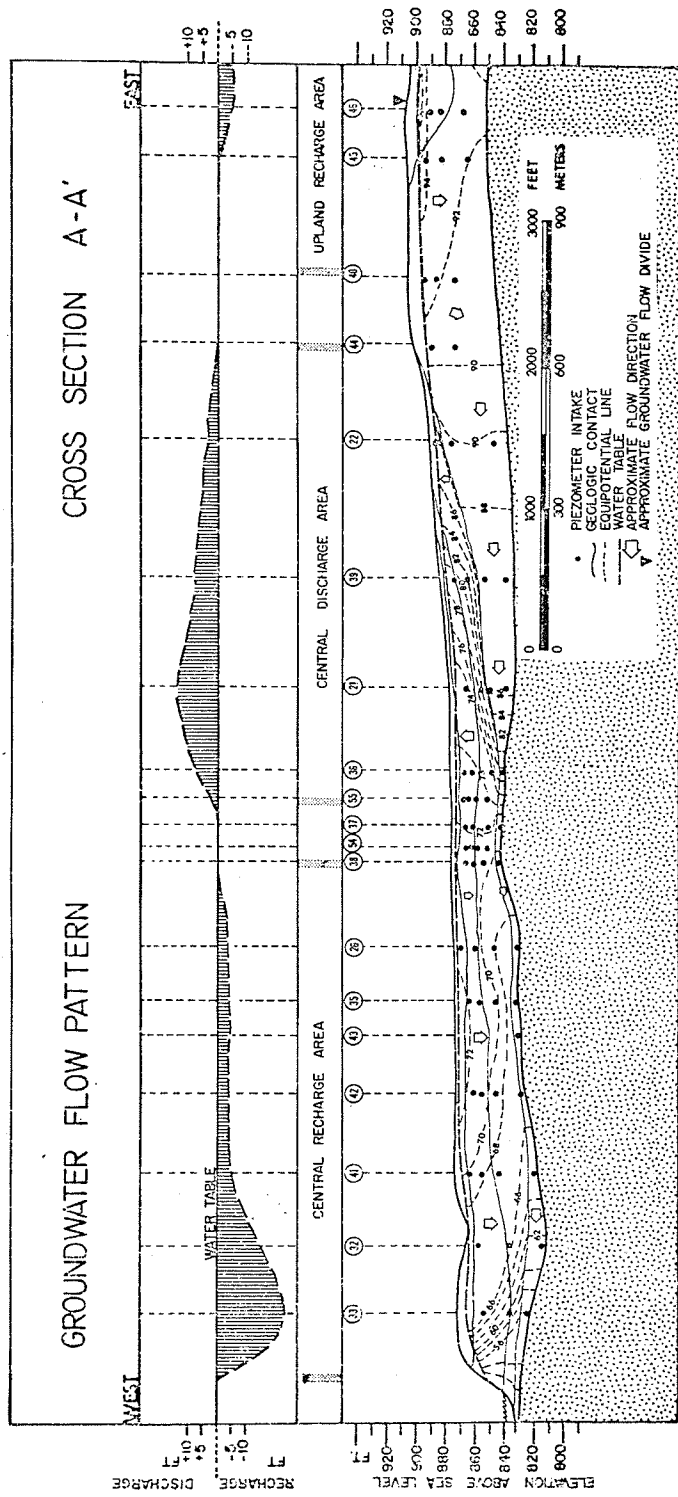


Fig. 26 Flow pattern interpretation based on the hydraulic potential distribution
Cross section A-A'

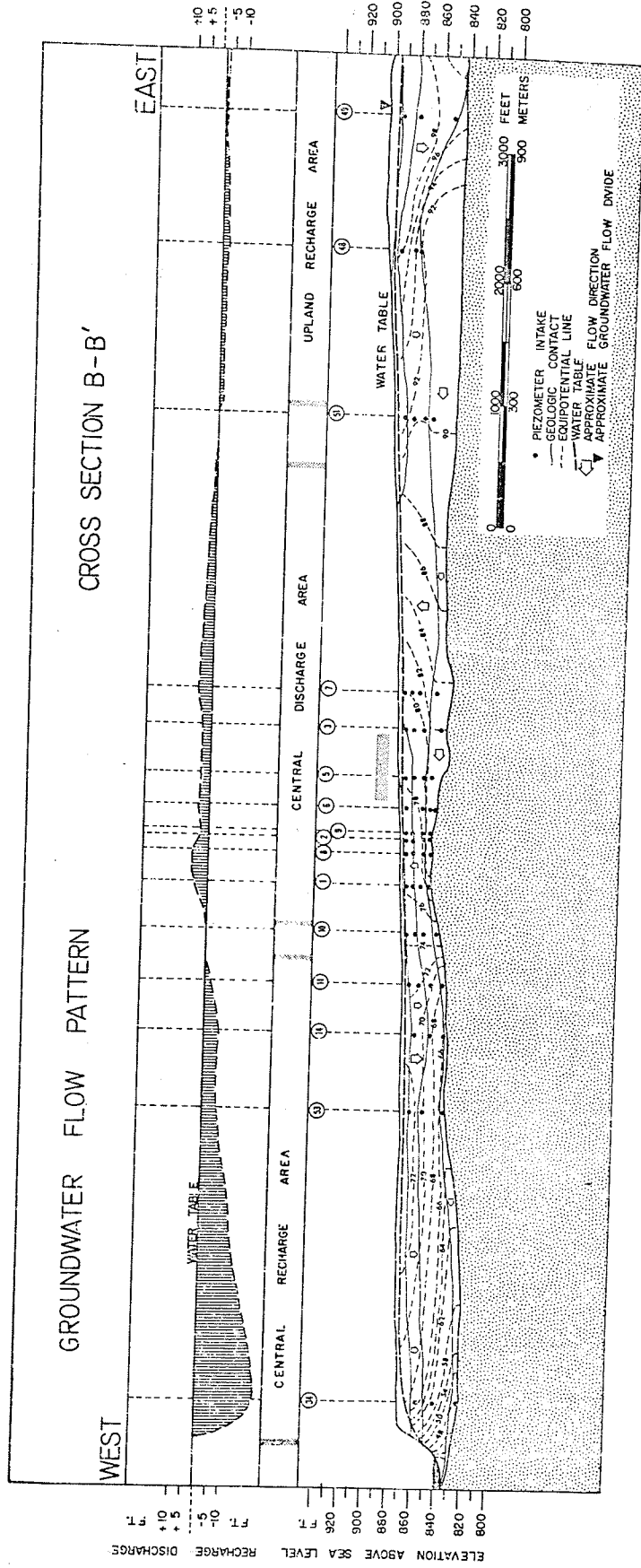


Fig. 27 Flow pattern interpretation based on the hydraulic potential distribution
Cross section B-B'

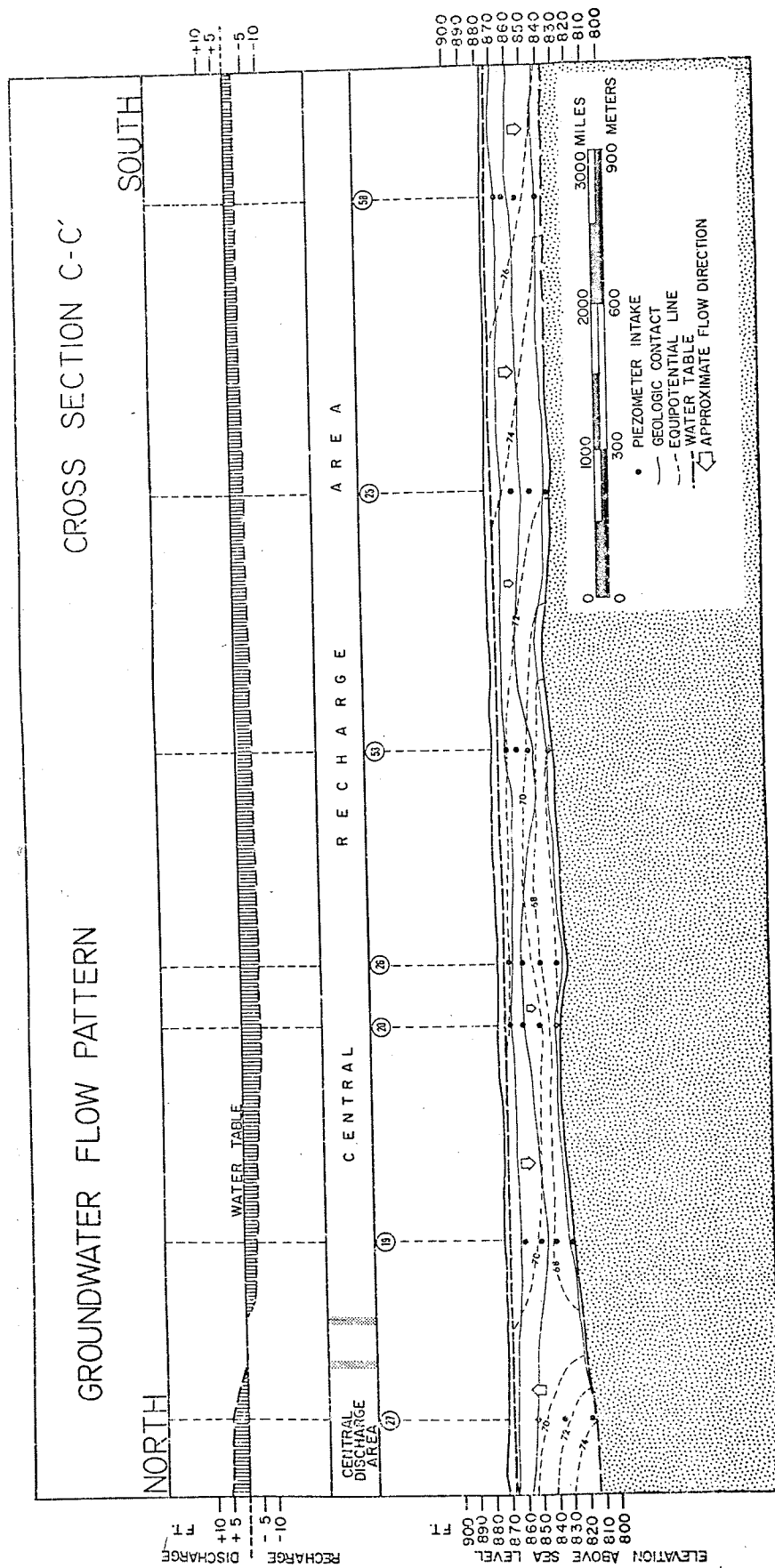


Fig. 28 Flow pattern interpretation based on the hydraulic potential distribution
Cross section C-C'

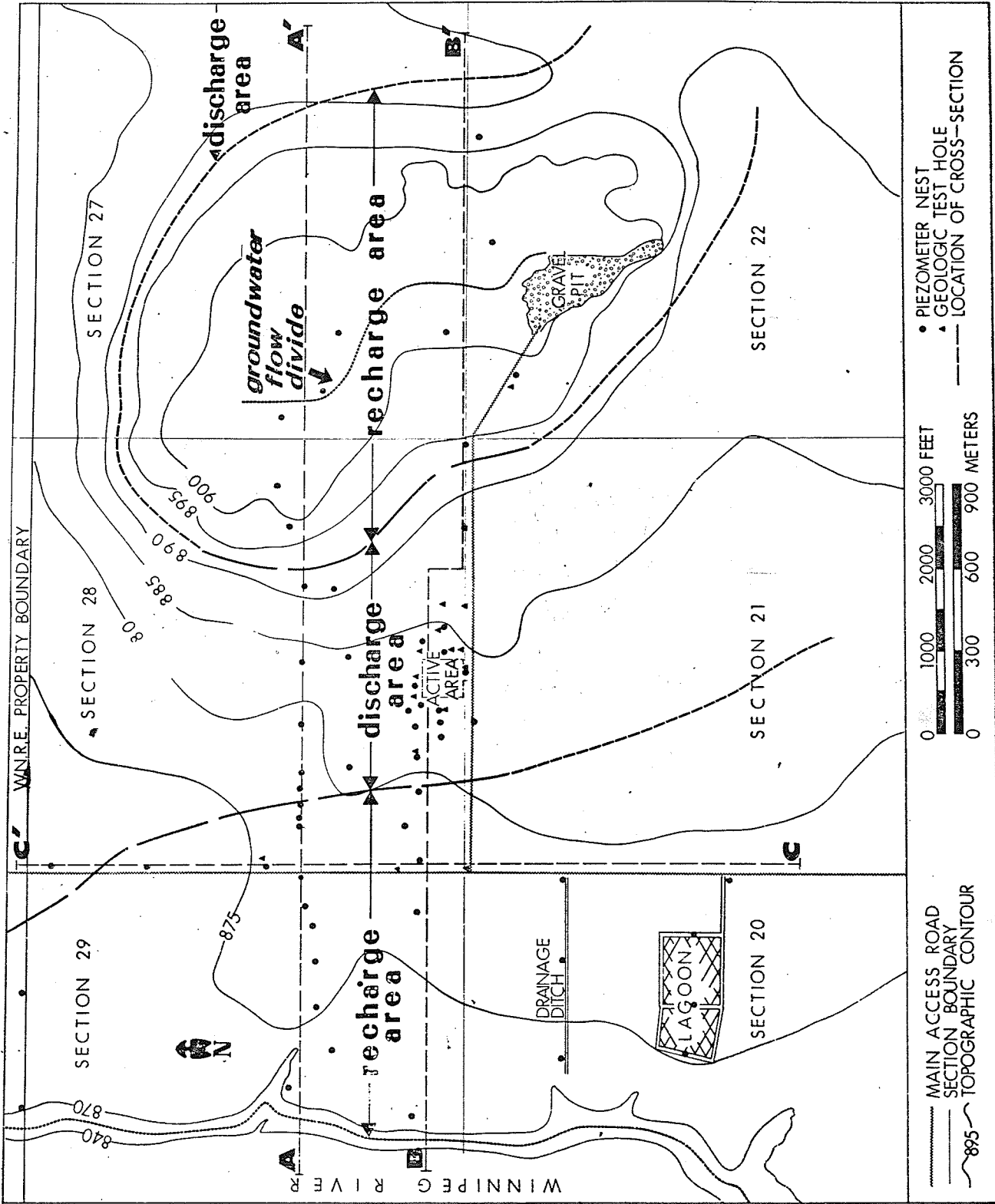


Fig. 29. Areal distribution of recharge-discharge areas.

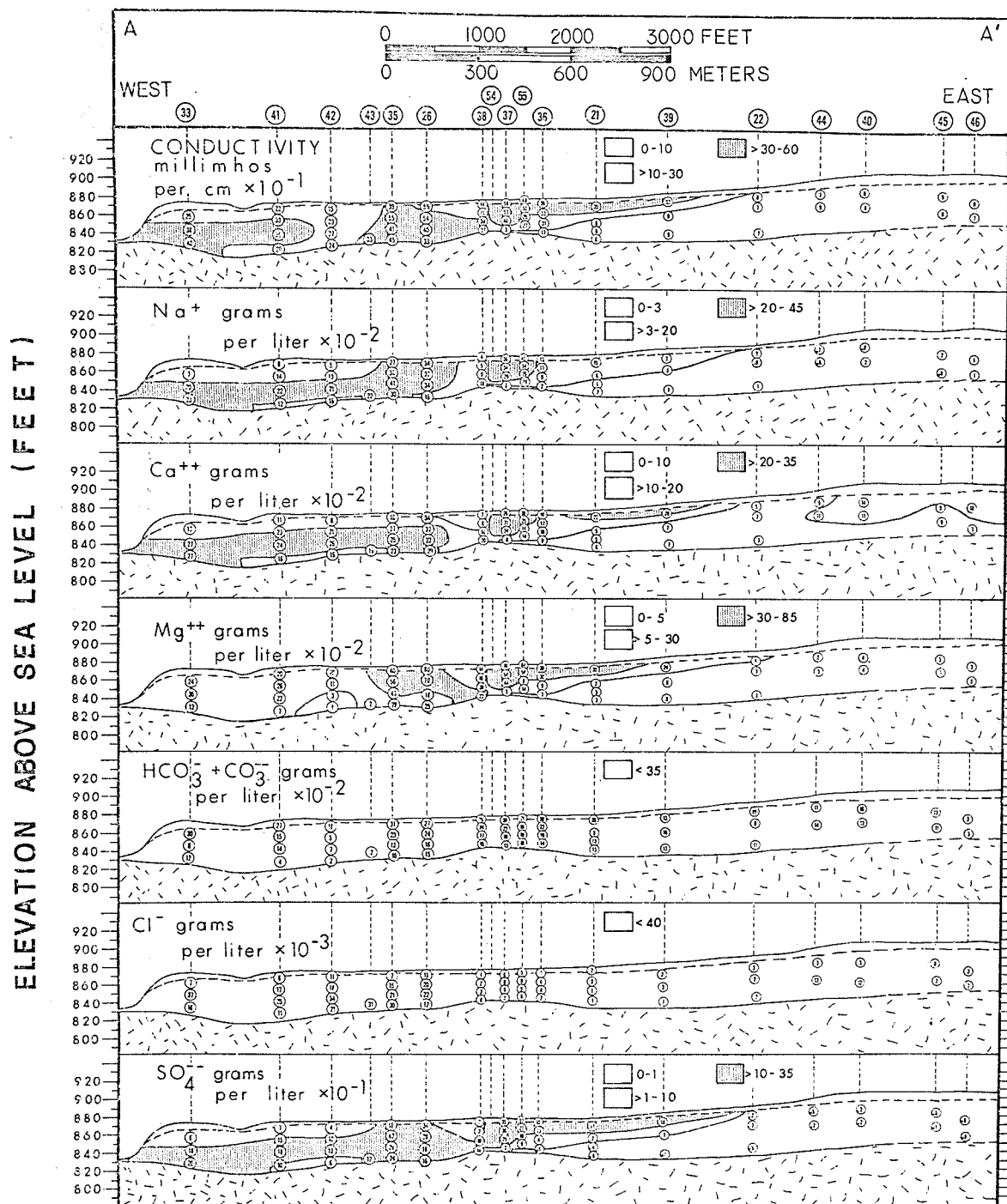


Fig. 30

Hydrochemical patterns - Cross Section A-A'

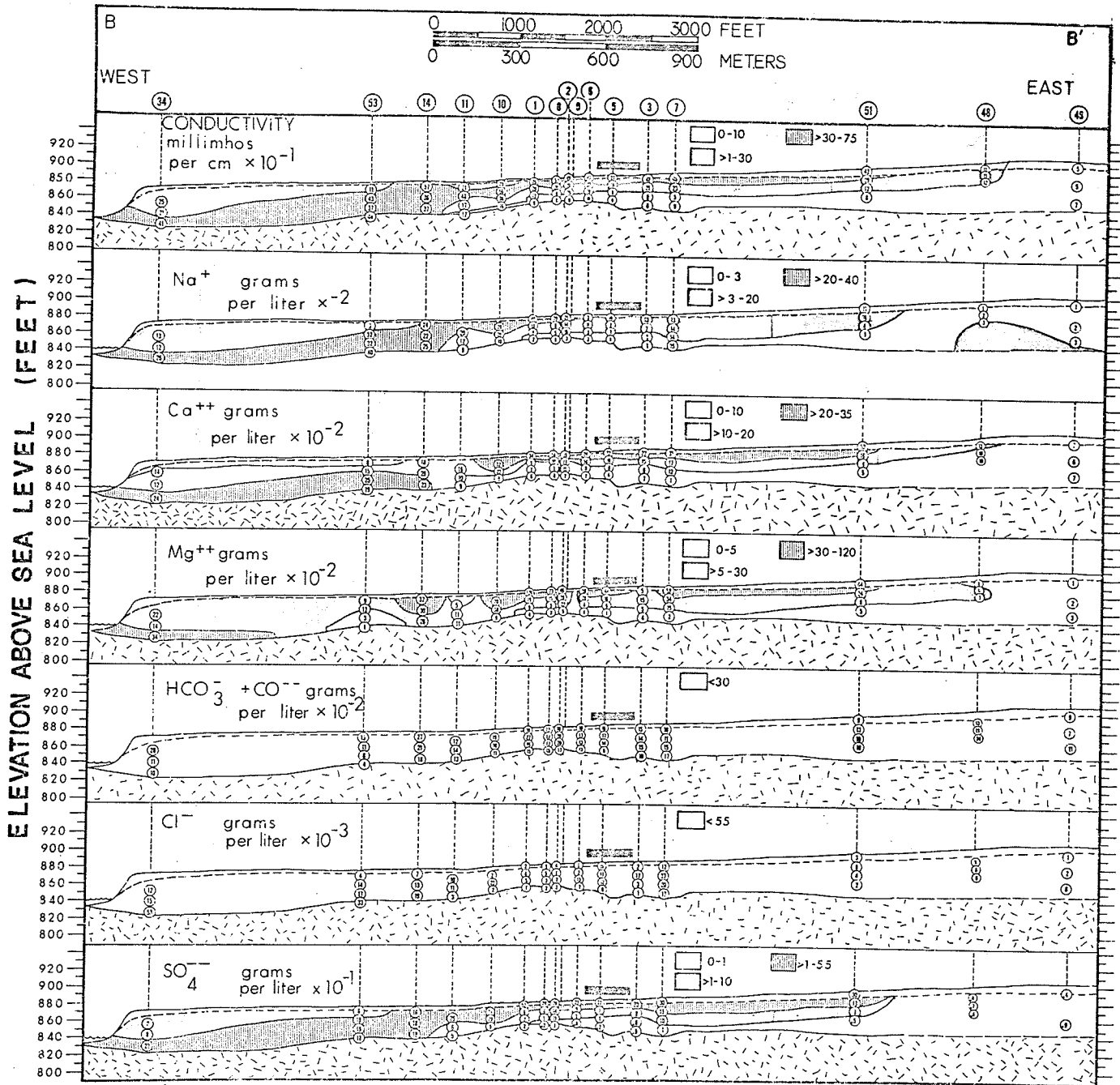


Fig. 31 Hydrochemical patterns - cross section B-B'

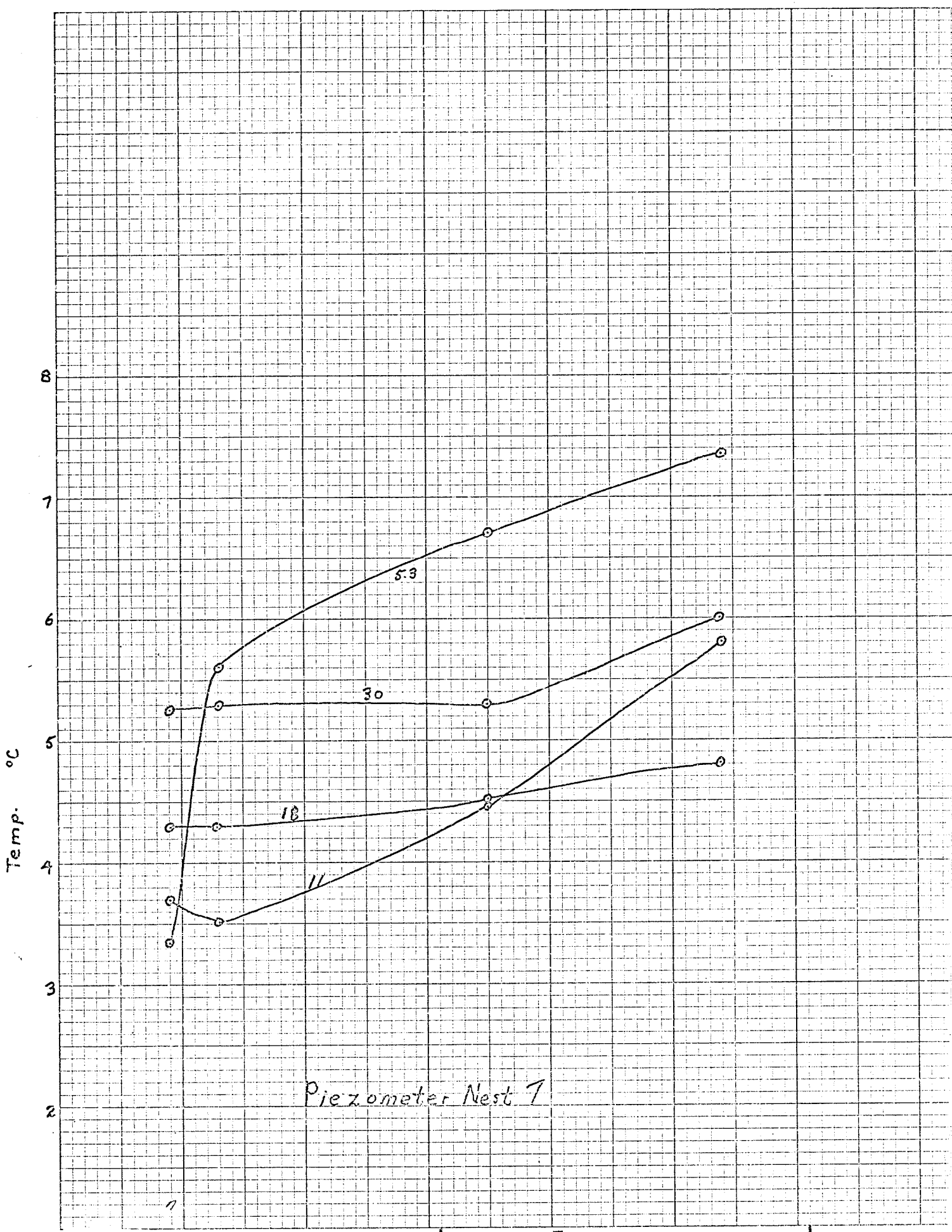


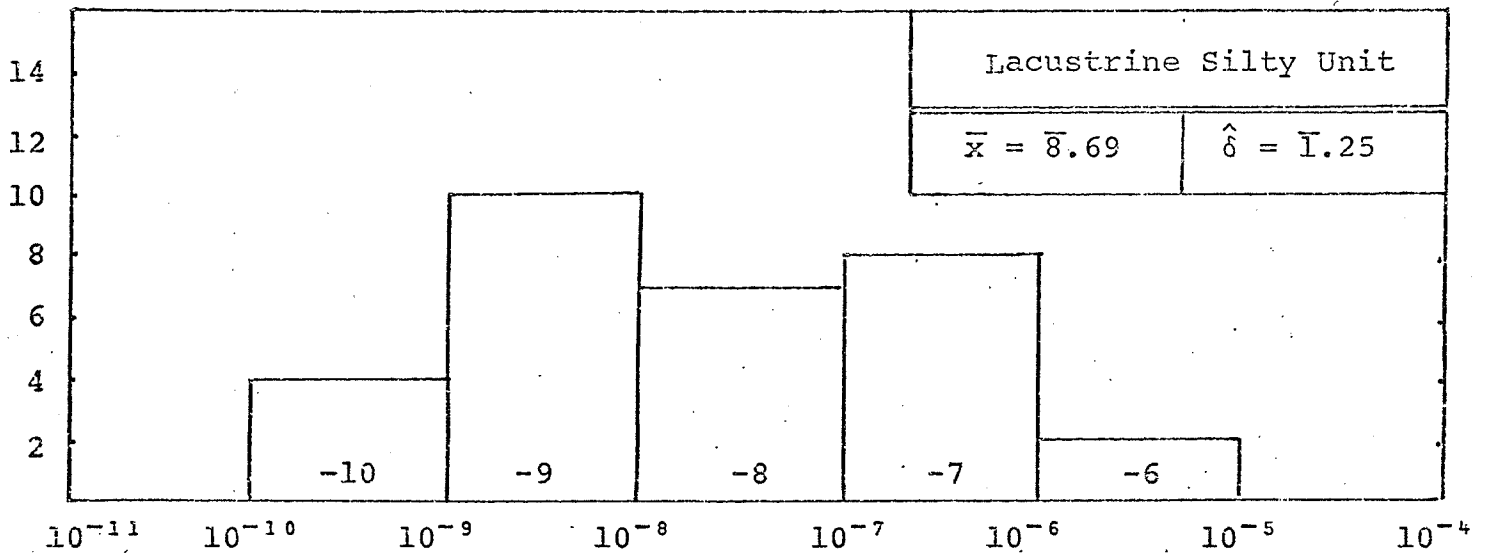
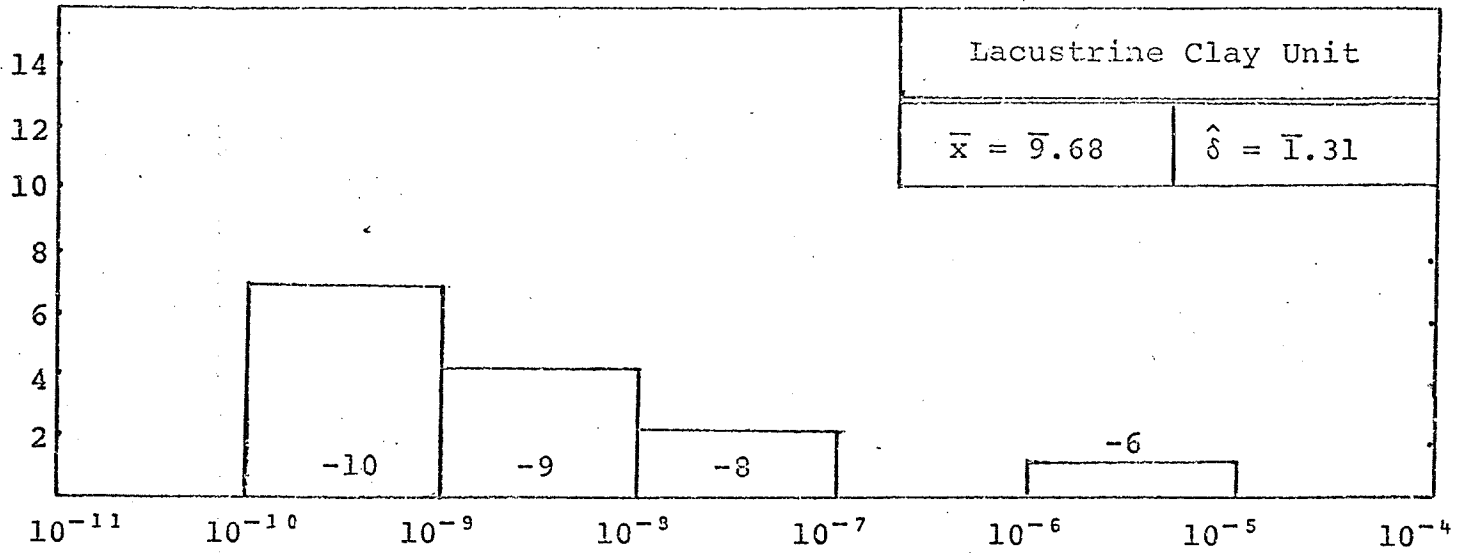
Fig. 32

May

June

HYDRAULIC CONDUCTIVITY (ft/sec)

NUMBER OF RESPONSE TESTS



\bar{x} = Unbiased estimate of logarithmic mean.

$\hat{\delta}$ = Population logarithmic standard deviation.

Fig. 33 Statistical summary of the Hvorslev hydraulic conductivity data from the lacustrine clay unit grouped according to depth intervals.

HYDRAULIC CONDUCTIVITY (ft/sec)

NUMBER OF RESPONSE TESTS

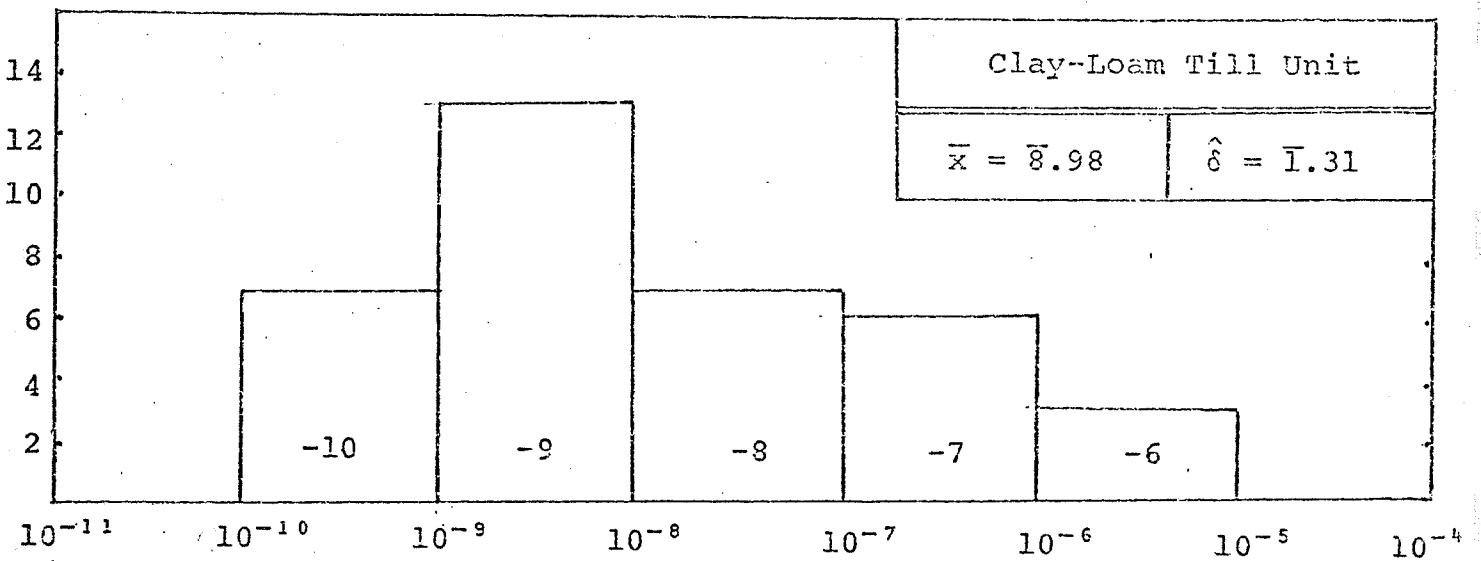
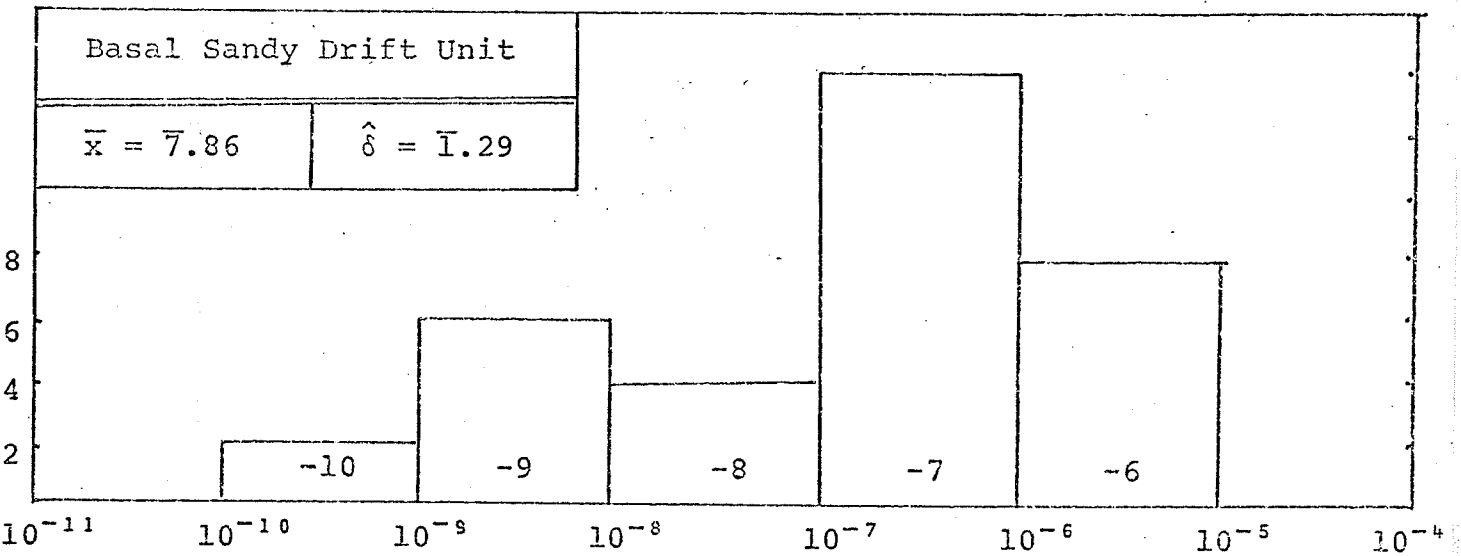
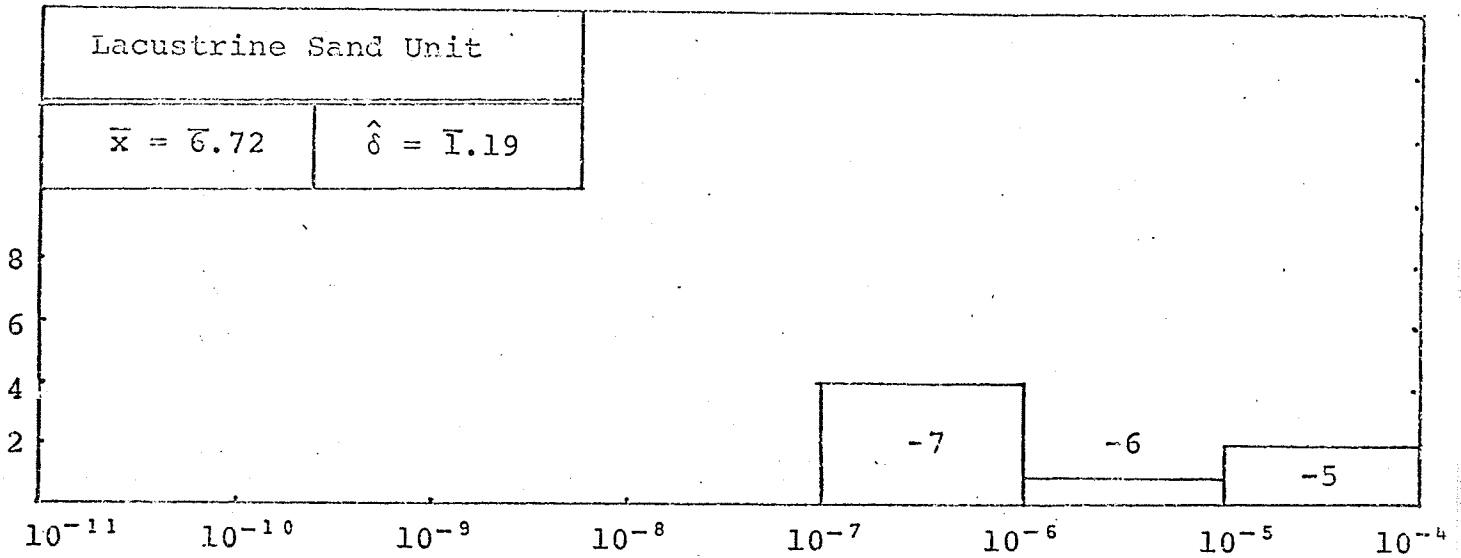


Fig. 37 Statistical summary of Hvorslev hydraulic conductivity data grouped according to hydrostratigraphic units.

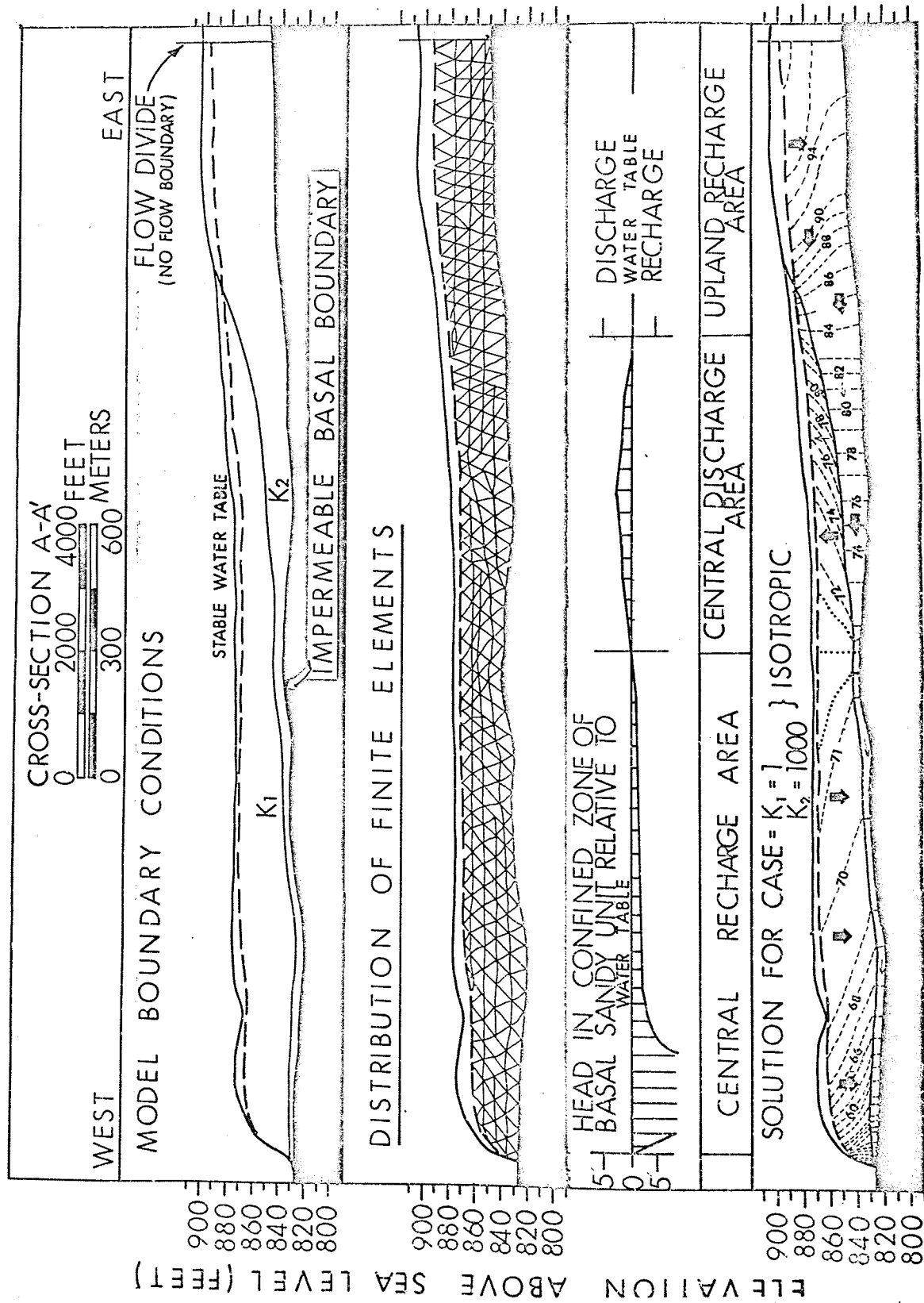


Fig. 35 'Best fit' model of the hydraulic potential distribution.

APPENDIX A

COMPILATION OF PHOTOGRAPHS

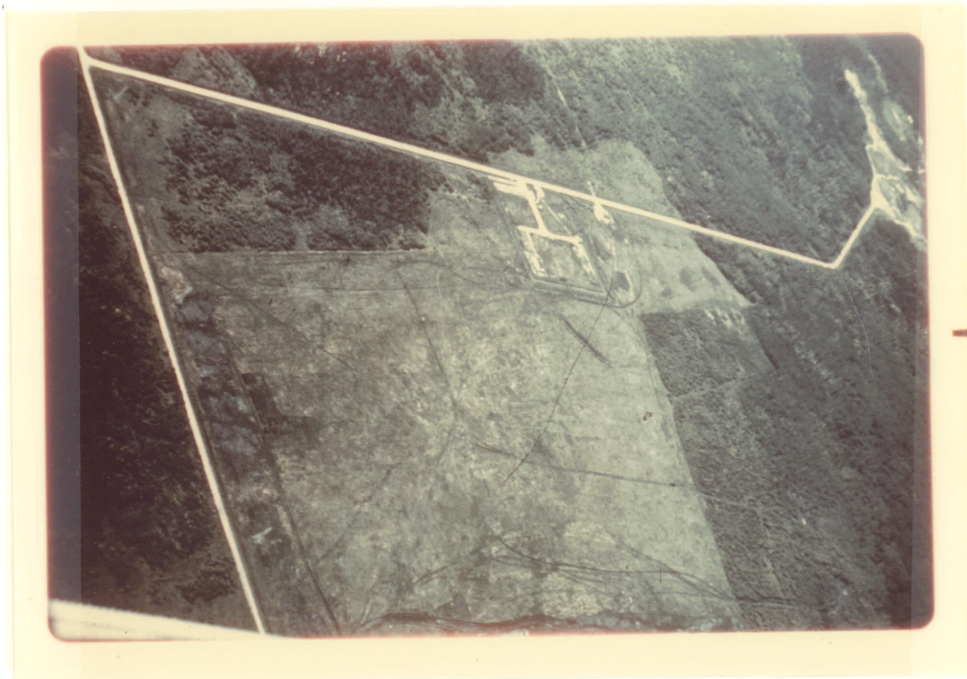


Fig. 1A Modified infra-red photograph looking east over Environmental Control Area. Outcrop of lacustrine sand and gravel unit is on the left. Present waste disposal area is in top centre.



Fig. 2A Caissons for subsurface disposal of radioactive wastes.



Fig. 3A Winter conditions in Environmental Control Area



Fig. 4A Parting along bedding in lacustrine silty unit.



Fig. 5A Jointing in lacustrine clay unit with precipitates on joint surfaces.



Fig. 6A Jointing in lacustrine clay unit without visible precipitates.



Fig. 7A Ice-rafted clayey-silt fragment embedded in lacustrine clay unit.



Fig. 8A Clay-loam till unit.



Fig. 9A Installation of piezometers in central discharge area.



Fig. 10A Installation of piezometers in upland recharge area.



Fig. 11A PVC piezometer with intake zone wrapped with fine mesh screen.



Fig. 12A Reading water levels in piezometers in late fall after water-table freezing begins.



Fig. 13A Injection of tritium tracer at injection site No. 1.



Fig. 14A Injection vessel with level gauge.



Fig. 15A Sampling for tritium at injection site No. 1. Colouring of the water sample due to sodium fluorescein remaining in the groundwater from a trial injection.



Fig. 16A Injection site No. 2 in late fall.

APPENDIX B

GEOLOGIC LOGS OF TEST HOLES

TEST HOLE #1

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-15	Finely laminated clay and silty clay; gypsum flakes and iron concretions present throughout; colour 2.5Y browns and olive browns.	lacustrine clay unit
15-16	Interlayered sandy, pebbly clay, sandy silts.	
16-36	Sandy, silty, clayey till; carbonate pebbles; colour 2.5Y light olive brown and yellowish brown near 16 feet becoming 2.5Y4/2 below.	clay-loam till
36-39	Till and interbedded medium to coarse sand; igneous pebbles and quartzo-feldspathic sand.	basal sandy drift
39	End of hole on rock.	

TEST HOLE #2

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-12	Complexly laminated clay with carbonate pebbles and sand grains common in upper few feet; gypsum flakes common in upper five feet; colour 2.5Y4/2 dark grey brown with lighter grey laminae, mottled 2.5Y shades in upper 5 feet.	lacustrine clay unit
12-13.5	Layered silty sands; gradational from clay to till-like material.	
13.5-28	Silty sandy clayey till with carbonate rock fragments; silt content increases slightly downward; colour 2.5Y4/2 becoming 2.5Y4/4 near 28 feet.	clay-loam till

TEST HOLE #2 Cont'd.

28-40	Very poorly sorted gravelly sands with till-like interbeds; rock fragments and sand of igneous origin; colour 5Y4/2 olive grey.	basal sandy drift
40	End of hole due to auger breakage.	

TEST HOLE #3

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-12	Laminated clay with isolated pebbles and sand grains in upper portion; occasional laminae of very fine sand between 5 and 10 feet; silt content increased slightly near 12 feet; colour 2.5Y/3/2.	lacustrine clay unit
12-21	Sandy silty clayey till with carbonate pebbles; clay-loam till colour 2.5Y4/4.	
21-21.5	Coarse gravel and cobbles.)
21.5-24	Medium to coarse pebbly sand, colour 5YN3.) basal sandy drift
24-30	Very sandy slightly silty till; colour 5YN3.)
30	End of hole on rock.	

TEST HOLE #4

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-7	Laminated silty clay; pebble content increases with depth; till-like zones; gypsum flakes common; colour 2.5Y greys.	lacustrine clay unit
7-10	Highly variable sand-silt-clay layered complex.	

TEST HOLE #4 Cont'd.

10-23	Sandy silty clayey till with carbonate pebbles; colour 2.5Y3/2.	clay-loam till
23-42	Quartzo-feldspathic sand, medium to fine, with clay interbeds a few inches thick in lower 15 feet; colour 5YN/3.	basal sandy drift
42	End of hole on rock.	

TEST HOLE #5

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-3	Clay; laminated; pebbles common; colour 2.5Y5/4.	lacustrine clay unit
3-21	Till; carbonate pebbles predominate; colour 2.5Y5/4 grading downward to 5Y5/4.	clay-loam till
21-46	Sand; medium to fine poorly sorted gravelly; igneous origin; sandy till-like layer near the upper contact and clay interbed believed present near bottom; colour 5YN3.	basal sandy drift
46	End of hole on rock.	

TEST HOLE #6

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-6	Clay; laminated; silty, pebbly; colour 2.5Y4/2.	lacustrine clay unit
6-28	Till; carbonate pebbles predominate; becomes progressively more sandy downwards; pebble content decreases downwards; colour 2.5Y4/4 becoming 5YN/3 around 15 feet.	clay-loam till

TEST HOLE #6 Cont'd.

28-47	Very poorly sorted medium, gravelly sand, igneous origin; below 40 feet driller reported till-like drilling condition but samples were not obtained due to boulders.	basal sandy drift
47	End of hole on rock.	

TEST HOLE #7

Depth (ft)	Characteristics	Interpretation
0-1	Top soil.	
1-31	Sandy silty clayey till with carbonate pebbles; sand and silt content increasing downward; colour 2.5Y4/2.	clay-loam till
31-37	Silt, sand, pebbles, boulders; colour 5YN/3.	basal sandy drift
37	End of hole on rock.	

TEST HOLE #8

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-5	Laminated, mottled, clay; carbonate pebbles present; colour 5Y4/3.	lacustrine clay unit
5-23	Normal; till with carbonate pebbles; colour 5Y4/3 grading to olive grey.	clay-loam till
23-36	Sand; medium very poorly sorted; many cobbles; lower few feet become very fine silty sand; colour 5YN/4.	basal sandy drift
36	End of hole.	

TEST HOLE #9

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-5	Laminated, mottled, clay; gypsum flakes common; colour 5Y4/2.	lacustrine clay unit
5-26	Normal till with carbonate pebbles; becomes more silty in lower portions; colour 5Y5/4.	clay-loam till
26	End of hole on rock. NOTE: second hole drilled five feet from TEST HOLE #9 also ended at 26 feet on rock.	

TEST HOLE #10

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-5	Laminated, mottled, clay; carbonate pebbles present; colour 2.5Y light browns.	lacustrine clay unit
5-23	Till; normal; colour 2.5Y4/4.	clay-loam till
23	End of hole on rock. NOTE: second hole drilled five feet away from TEST HOLE #10 also ended at 23 feet on rock.	

TEST HOLE #11

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-11	Clay; massive, mottled; pebbles absent; colour 2.5Y3/2.	lacustrine clay unit
11-17	Till; normal; colour 2.5Y4/4.	clay-loam till
17	End of hole on rock.	

TEST HOLE #11 Cont'd.

New hole started 4 feet away.

13-22	Till; normal; carbonate pebbles predominate.	clay-loam till
22-30	Sand; fine to very fine well sorted, no pebbles; colour 5YN/4.	basal sandy drift
30	End of hole on rock.	

TEST HOLE #12

Depth (ft.)	Characteristics	Interpretation
0-2	Road fill.	
2-11	Clay; laminated, mottled, light grey silt balls (angular fragments) in dark clay.	lacustrine clay unit
11-22	Till; normal; colour 5Y4/3 grading to 5YN/4.	clay-loam till
22-40	Sand; upper three feet is coarse poorly sorted silty; 25 to 40 is fine well sorted dense clean sand; colour 5YN/4.	basal sandy drift
40	End of hole, no more auger available.	

TEST HOLE #13

Depth (ft.)	Characteristics	Interpretation
0-4	Road fill.	
4-7	Very fine sand and silt.	lacustrine silty unit
7-16	Clay; massive; no pebbles noticed; many gypsum flakes; colour 5Y4/3.	lacustrine clay unit
16-18	Transition zone.	
18-29	Till; normal; carbonate pebbles predominate; colour 5Y5/3.	clay-loam till

TEST HOLE #13 Cont'd.

29-35	Sand; very fine silty sand interlayered with black silty clay; becomes gravelly in last foot.	basal sandy drift
35	End of hole on rock.	

TEST HOLE #14

Depth (ft.)	Characteristics	Interpretation
0-3	Road fill.	
3-29	Clay; layered massive and laminated zones; pebbled content increasing with depth; angular silt fragments increase with depth; gypsum flakes concentrated around 20 feet; laminae of very fine sand; colour 5Y5/4 and 5Y4/3, sand laminae 5Y7/1.	lacustrine clay unit
29-30	Clay; massive; very compact; slightly sandy; colour 5Y4/3.	lacustrine clay unit
30-38	Till; hard, very silty; carbonate and granitic pebbles; very bouldery in the last foot.	clay-loam till
38	End of hole on rock.	

TEST HOLE #15

Depth (ft.)	Characteristics	Interpretation
0-4	Road fill.	
4-11	Clay and silty clay; fine sand laminae present, pebbly.	lacustrine clay unit
11-25	Till; upper portion has deficiency of sand but approaches equal proportions near lower boundary.	clay-loam till
25-30	Till; slightly less clay than above and very bouldery; some portions drill like sand.	basal sandy drift
30	End of hole on rock.	

TEST HOLE #16

No drilling log available.

TEST HOLE #17

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-8	Sand and gravel, interbedded.	lacustrine sand and gravel unit
8-16	Silty clay and clay; laminated, pebbly; colour 5Y greys and olives.	lacustrine clay unit
16-42	Till; carbonate pebbles predominate; colour 5Y4/1 dark grey.	clay-loam till
42-45	Till; slightly more silty than above; drills very bouldery.	basal sandy drift

TEST HOLE P-1

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-2	Silty sand.	lacustrine silt unit
2-15	Clay; laminated; many gypsum flakes and much iron staining; pebbles absent near surface but become common near 15 feet.	lacustrine clay unit
15-15.5	Gravelly layer.	
15.5-23	Till; mottled, with carbonate pebbles; upper two feet silty and gravelly, lower material normal till composition; dark grey fine sand layers present in upper 4 feet.	clay-loam till
23-30	Sand; clean fine grained with minor gravel; becomes silty and clayey near 27 and resembles a sandy till near 30 feet.	basal sandy drift
30	End of hole for piezometer installation.	

TEST HOLE P-2

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-8	Clay; laminated; many gypsum flakes and iron staining; carbonate pebbles predominant over highly weathered igneous pebbles; colour 2.5Y3/2.	lacustrine clay unit
8-21	Till; normal with dark grey fine sand layers less than 0.1 inches thick; near 8 feet till appears banded with sand and gravel; colour 2.5Y4/2.	clay-loam till
21-29	Sand; poorly sorted, gravelly; igneous fragments.	basal sandy drift
29-30	Sandy till.	

TEST HOLE P-3

Same as Test Hole #8.

TEST HOLE P-4

Same as Test Hole #10

TEST HOLE P-5

Same as Test Hole #4.

TEST HOLE P-6

Same as Test Hole #11.

TEST HOLE P-7

Same as Test Hole #6.

TEST HOLE P-8

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-14	Clay; laminated; gypsum flakes common; dark grey brown.	lacustrine clay unit
14-21	Till; silty; igneous and carbonate pebbles.	clay-loam till
21-27	Sand; very poorly sorted, gravelly, bouldery, silty; possibly sandy till.	basal sandy drift
27-29	Till; normal; mainly carbonate pebbles.	
29	End of hole on rock.	

TEST HOLE P-9

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-9	Clay; mottled, laminated; dark grey brown.	lacustrine clay unit
9-15	Clay-till transition; based on drilling feel and samples.	
15-23	Till; normal; olive brown.	clay-loam till
23-30	Sand; very coarse gravelly dirty; in bottom) three feet approaches a sandy till.)	basal sandy drift
30-41	Sand; gravelly, bouldery; less silt and clay) than above.)	
41	End of hole due to difficult drilling in boulders.	

TEST HOLE P-10

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-13	Clay; generally non-mottled massive; no silt fragments; slightly silty; minor gypsum flakes.	lacustrine clay unit
13-15	Transition to till.	
15-23	Till; normal; olive brown.	clay loam till
23-32	Sand; silty, clayey; gravelly; possibly a sandy till; upper three feet are very till-like.	basal sandy drift
32	End of hole on rock.	

TEST HOLE P-11

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-18	Clay; laminated; lacks usual mottling and silt fragments; patches of white and grey material; colour 5Y4/3.	lacustrine clay unit
18-27	Till; normal; carbonate pebbles predominate; colour 5Y4/3.	clay-loam till
27-30	Till; becomes very sandy near 30 feet; colour 5Y5/4.	basal sandy drift
30-36	Sand; very coarse to coarse, gravelly, slightly silty; colour 5Y5/2.	
36	End of hole.	

TEST HOLE P-12

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-6	Clay; laminated, mottled; many gypsum flakes and angular silt fragments; colour olive grey 5Y4/3 and 5Y4/4.	lacustrine clay unit
6-21	Till; normal; carbonate and igneous pebbles; colour 5Y4/3 grading downward to 5Y4/1.	clay-loam till
21-25	Sand and gravel; very poorly sorted; angular pebbles and sand grains; granitic.)	
25-37	Sand; very fine to fine well sorted; a dense silt or very fine sand layer encountered at 36 feet.)	basal sandy drift
37-42	Gravel; based on drilling feel; no samples obtained.)	
42	End of hole on rock.	

TEST HOLE P-13

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-11	Clay; massive, non-mottled; occasional pebbles; fissile; colour 5Y4/2.	lacustrine clay unit
11-13	Clay-till transition.	
13-25	Till; more silty than normal; carbonate and igneous pebbles; colour 5Y4/3.	clay-loam till
25-30	Till; slightly more sandy and silty than normal; colour 5YN4.	clay-loam till
30-33	Sand; silty gravelly, resembling sandy till.	basal sandy drift
33	End of hole due to auger breakage.	

TEST HOLE P-14

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-22	Clay; laminated in some portions; fine sand layers at angles to layering; some silt fragments in lower portion; pebble content increases with depth; colour 5Y4/2 to 5Y4/3.	lacustrine clay unit
2-35	Till; normal; seems abnormally soft in places but no visible variation in grain size ratios; colour 5YN4.	clay-loam till
35	End of hole on rock.	

TEST HOLE P-15

Same as Test Hole #12

TEST HOLE P-16

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-16	Till; igneous and carbonate pebbles; occasional slightly silty zone; colour 2.5Y4/4 grading to 5Y3/1 near 16 feet.	clay-loam till
16-49	Sand; very fine to fine well sorted; drills as if layered possibly with clayey interbeds; colour 5YN4.	lacustrine sand unit
49-52	Silt and clay; interbedded; colour 5YN4.	
52-55	Sand; granitic, very coarse, poorly sorted.	
55	End of hole on rock.	

TEST HOLE P-17

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-13	Clay; laminated; many gypsum flakes; silt fragments increase near 13 feet; colour 2.5Y3/2.	lacustrine clay unit
13-21	Till; normal; colour 2.5Y5/4.	clay-loam till
21-34	Sand; very fine; well sorted; silt and clay interbeds; colour 5YN4; sand becomes coarse and poorly sorted from 31-34 feet.	basal sandy drift
34	End of hole on rock.	

TEST HOLE P-18

Depth (ft.)	Characteristics	Interpretation
0-4	Top soil and fill.	
4-11	Clay; laminated to massive; colour 5Y6/2, 5Y5/4 and 5Y4/3.)))	lacustrine clay unit
11-27	Silty clay; very soft; appears bedded; colour dark grey 5Y4/1.))	
27-36	Till; more silty than normal; very plastic; colour 5Y4/1.	clay-loam till
36-48	Sand; from 36 to 41 feet very fine silty sand, well sorted from 41 to 48 feet medium poorly sorted silty pebbly sand with boulders; definitely not a till.	basal sandy drift
48	End of hole on rock.	

TEST HOLE P-19

Depth (ft.)	Characteristics	Interpretation
0-5	Top soil and road fill.	
5-27	Silty clay and clay; upper 10 feet somewhat till-like; colour 5Y5/3; below 10 feet generally massive; pebbles common; colour 5Y5/3 and 5Y4/1.	lacustrine clay unit
27-42	Till; less clay than normal; carbonate and igneous pebbles common; colour 5Y5/1.	clay loam till
42-44	Sand?; driller reported sand but no sand sample was obtained.	basal sandy drift
44	End of hole on rock.	

TEST HOLE P-20

Depth (ft.)	Characteristics	Interpretation
0-3	Top soil.	
3-17	Clay; massive to slightly laminated; silt fragments common.	lacustrine clay unit
17-36	Till; normal; igneous and carbonate pebbles; colour 5Y4/1.	clay-loam till
36-39	Till; very bouldery and sandy; sand interbeds suspected.	basal sandy drift
39	End of hole on rock.	

TEST HOLE P-21

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-17	Clay; many selenite crystal concentrations; laminated grading to more massive in lower portions; occasional pebble; colour 5Y5/4 and 5Y4/1.	lacustrine clay unit

TEST HOLE P-21 Cont'd .

17-25	Till; normal; igneous and carbonate pebbles; colour 5Y4/1 dark grey.	clay-loam till
25-45	Sand; silty coarse very poorly sorted; layered; colour 5YN5.	basal sandy drift
45	End of hole on rock.	

TEST HOLE P-22

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-9	Till; normal; igneous and carbonate pebbles; colour 5Y5/3.	clay-loam till
9-10	Boulders.	lacustrine sand unit
10-43	Sand; fine, well sorted; colour 5Y5/1.	
43-53	Sand; granitic; coarse pebbly; no silt or clay.)	
53	End of hole on rock.	

TEST HOLE P-23

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-9	Clay.	lacustrine clay unit
9-11	Clay-till transition.	
11-21	Till; normal; colour 5Y4/3.	clay-loam till
21-24	Bouldery.	
24-37	Sand; coarse very poorly sorted; gravelly, bouldery; possible slightly silty; igneous origin.	basal sandy drift

TEST HOLE P-23 Cont'd.

37-41	Till; very bouldery; more sandy and silty than normal; colour 5YN4.	basal sandy drift
41	End of hole on rock.	

TEST HOLE P-24

Depth (ft.)	Characteristics	Interpretation
0-3	Fill.	
3-10	Silt and clayey silt; laminated; occasional pebble; colour -- greys and olives 5Y.	lacustrine silt unit
10-24	Clay and silty clay; no pebbles apparent; laminated.	lacustrine clay unit
24-51	Till; silty; less pebbles than usual; colour - dark grey.	clay-loam till
51-56	Sand; isolated pebbles and cobbles; slightly silty; has appearance of till in places in terms of pebble distribution.	basal sandy drift
56	End of hole on rock.	

TEST HOLE P-25

Depth (ft.)	Characteristics	Interpretation
0-3	Fill.	
3-10	Silts and clayey silts; laminated; greys and olives 5Y.	lacustrine silt unit
10-23	Clays and silty clays; grades into above; laminated.	lacustrine clay unit
23-38	Till; slightly more silty and less clayey than usual; colour 5Y4/1.	clay-loam till
38-43	Till; very silty and sandy; igneous pebbles; bouldery; difficult drilling.	basal sandy drift
43	End of hole on rock.	

TEST HOLE P-26

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-10	Silty and clayey silt; laminated; occasional pebbles and silt fragments; colour - greys and olives 5Y.	lacustrine silt unit
10-15	Clay and silty clay; grades into above; calcite concentrations (?).	lacustrine clay unit
15-42	Till; normal; igneous and carbonate pebbles; colour 5Y4/1.	clay-loam till
42-45	Sand; medium, pebbly; medium to poorly sorted; clean to slightly silty; some boulders or coarse gravel.	basal sandy drift
45	End of hole on rock.	

TEST HOLE P-27

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-27	Clay and silty clay; laminated; colour 5Y4/2 and 4?4 becoming 5Y4/1 near base; upper few feet are very silty.	lacustrine clay unit
27-56	Till; clayey silty sandy; carbonate pebbles predominant; in places clay content drops; colour 5Y4/1.	clay-loam till
56-59	Till; very pebbly, sandy, bouldery; igneous material.	basal sandy drift
59	End of hole on rock.	

TEST HOLE P-28

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-8	Silt and silty clay; laminated; olive 5Y colouring.	lacustrine silt unit
8-24	Clay and silty clay; laminated in places; oxidized near 8 feet grading rapidly to 5Y4/1 unoxidized.	lacustrine clay unit
24-75	Till; generally normal with minor variations in silt-sand-clay ratios as well as pebble content; pebbles mainly carbonate; no sign of sand or gravel interbeds.	clay-loam till
75-77	Clay; laminated; occasional pebble; gypsum concentrations.	
77	End of hole on rock.	

TEST HOLE P-29

Depth (ft.)	Characteristics	Interpretation
0-4	Road Fill.	
4-19	Clay and silty clay; laminated to massive; occasional pebble; gypsum flakes; olive 5Y colouring.	lacustrine clay unit
19-56	Till; carbonate pebbles predominate; colouring 5Y4/1.	clay-loam till
56	End of hole on rock.	

TEST HOLE P-31

Depth (ft.)	Characteristics	Interpretation
0-3	Top soil and peat.	

TEST HOLE P-31 Cont'd.

3-15	Silt and clayey silt; olives and yellow browns; massive.	lacustrine silt unit
15-37	Clay; soft, slightly silty; grades into the above; dark grey 5Y.	lacustrine clay unit
37-42	Till; normal; soft; dark grey 5Y.	clay-loam till
42-45	Bouldery; no sample, auger broke in hole.	basal sandy drift
45	End of hole due to auger breakage.	

TEST HOLE P-32

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-10	Silt and sand; very fine sands and silts; laminated; iron oxide staining; olive, yellow, and yellow brown colour.	lacustrine silt unit
10-32	Clay; slightly silty; silt fragments; generally massive.	lacustrine clay unit
32-46	Till; normal; mainly carbonate pebbles; soft; dark grey 5Y.	clay-loam till
46-59	Till; sandy, silty, becoming very sandy near bottom; igneous pebbles and sand grains pre-dominant; dark grey 5Y; very bouldery.	basal sandy drift
59	End of hole on rock.	

TEST HOLE P-33

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-10	Silt and clayey silt; very fine sand laminations; yellow and yellow brown colouration.	lacustrine silt unit

TEST HOLE P-33 Cont'd.

10-35	Clay; slightly silty; dark grey 5Y.	lacustrine clay unit
35-42	Till; normal; mainly carbonate pebbles; soft; dark grey 5Y.	clay-loam till
42-48	Till; sandy, silty becoming very sandy near bottom; igneous pebbles, bouldery; dark grey 5Y.	basal sandy drift
48	End of hole on rock.	

TEST HOLE P-34

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-13	Silt and very fine sand; olives, yellows, and yellow brown.	lacustrine silt unit
13-24	Clay; slightly silty.	lacustrine clay unit
24-46	Till; silty, sandy slightly clayey; very soft; dark grey.	clay-loam till
46-49	Till; sandy, silty; igneous pebbles; contact noted due to sudden increase in boulder content.	basal sandy drift
49-51	Till; as above but very pebbly.	

TEST HOLE P-35

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-8	Silts and very fine sand; laminated; yellows, browns, olives.	lacustrine silt unit
8-15	Clay; slightly silty; many silt fragments and gypsum flakes.	lacustrine clay unit

TEST HOLE P-35 Cont'd.

15-17	Transition to till.	
17-38	Till; clayey, silty, sandy; dark grey unoxidized; soft.	clay-loam till
38-43	Till; sandy (medium to fine) slightly silty, pebbly, bouldery; igneous pebbles.	basal sandy drift
43	End of hole on rock.	

TEST HOLE P-36

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-6	Silts and very fine sand.	lacustrine silt unit
6-19	Clay; laminated; oxidized olives 5Y; many silt fragments; selenite concentrations; occasional pebble; becoming oxidized dark grey 5Y near bottom.	lacustrine clay unit
19-22	Clay-till transition.	
22-31	Till; clayey silty, sandy; carbonate pebbles predominate; dark grey 5Y.	clay-loam till
31-37	Till; sandy, silty, pebbly; silt content decreased downwards; igneous pebbles.	basal sandy drift
37	End of hole on rock.	

TEST HOLE P-37

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-8	Silt; laminations of very fine sand and clay.	lacustrine silt unit

TEST HOLE P-37 Cont'd.

8-16	Clay; slightly silty; occasional pebble; silt fragments; selenite concentrations; oxidized olives 5Y.	lacustrine clay unit
16-17	Transition to till.	
17-34	Till; clayey silty sandy; very soft; mainly carbonate pebbles.	clay-loam till
34-36	Till; silty, pebbly, bouldery; igneous pebbles.	basal sandy drift
36	End of hole on rock.	

TEST HOLE P-38

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-4	Silt and very fine sand; laminated.	lacustrine silt unit
4-14	Clay; slightly silty; occasional pebble; silt fragments.	lacustrine clay unit
14-16	Transition to till.	
16-31	Till; normal; mainly carbonate pebbles;	clay-loam till
31-33	Till?; very bouldery; no samples as auger broke in hole.	basal sandy drift
33	End of hole in boulders.	

TEST HOLE P-39

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-4	Silts and very fine sand.	lacustrine silt unit

TEST HOLE P-39 Cont'd.

4-8	Clay; silty; silt fragments common; oxidized olive 5Y.	lacustrine clay unit
8-24	Till; normal; carbonate pebbles predominate.	clay-loam till
24-42	Sand; layered very fine silty sand and medium sand; no pebbles; occasional thin clay interbed.	lacustrine sand unit
42-46	Medium to coarse well-sorted sand.	
46	End of hole in boulders.	

TEST HOLE P-40

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-56	Sand; clean, well sorted fine to medium sand; no pebbles; oxidized to 18 feet.	lacustrine sand unit
56-57	Clay; layered, very silty, black; angular carbonate pebbles.	
57	End of hole on rock.	

TEST HOLE P-41

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-7	Silty clay, clayey silt and very fine sand; interbedded, laminated; oxidized.	lacustrine silt unit
7-20	Clay, lacustrine massive.	lacustrine clay unit
20-50	Till, clayey, silty, sandy, with carbonate pebbles; 5Y5/1 grey.	clay-loam till
50-54.5	Till, very sandy silt; igneous pebbles.	basal sandy drift
54.5	End of hole on solid rock.	

TEST HOLE P-42

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-6	Silt, silty clay and very fine sand; interbedded, laminated; gradational lower contact.	lacustrine silt unit
6-18	Clay, lacustrine; olive grey 5Y becoming dark grey; occasional pebble and silt ball.	lacustrine clay unit
18-22	Transition to till.	
22-43	Till; olive grey 5Y4/2 grading to dark grey 5Y4/1; clayey, silty, sandy; very plastic.	clay-loam till
43-45.5	Till; silty sandy; igneous pebbles.	basal sandy drift
45.5	End of hole in boulders.	

TEST HOLE P-43

Depth (ft.)	Characteristics	Interpretation
0-1	Topsoil.	
1-6	Silt, clayey silt and very fine sand; interbedded, laminated; oxidized.	lacustrine silt unit
6-18	Clay, lacustrine.	lacustrine clay unit
18-23	Transition to till.	
23-40	Till; silty, sandy, slightly clayey; carbonate pebbles.	clay-loam till
40-44	Sand, medium, poorly sorted; upper foot bouldery; slightly pebbly.	basal sandy drift
44	End on solid rock.	

TEST HOLE P-44

Depth (ft.)	Characteristics	Interpretation
0-53	Sand, very fine to fine; massive, poorly sorted; no pebbles, few fines.	lacustrine sand unit

TEST HOLE P-45

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-6	Till; silty, sandy, slightly clayey; igneous pebbles.	
6-43	Sand; fine to very fine; poorly sorted, no fines; oxidized to 20 feet; no pebbles; no clay interbeds observed.	lacustrine sand unit

TEST HOLE P-46

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-4	Sand; coarse to medium with occasional pebble; oxidized.	<i>undifferentiated sandy till</i>
4-8	Sand; coarse; very poorly sorted; pebbly, slightly silty; oxidized.	
8-17	Silt; sandy, pebbly; and sand; fine, silty; oxidized olive 5Y; pebbly.	
17-30	Silt and sand; interlayered; unoxidized dark grey 5Y.	
30-40	Sand; medium to coarse; pebbly, no fines.	
40-47.5	Sand; fine, poorly sorted; slightly pebbly, no fines.	

TEST HOLE P-47

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-10	Till; silty, sandy, slightly clayey; oxidized olive 5Y; gypsum and carbonate concentrations.)) <i>Undifferentiated sandy till</i>))
10-18	Till; as above but more sandy and bouldery; very little clay.	
18-21	Sand; very coarse, pebbly, bouldery; oxidized.	
21	End of hole in boulders.	

TEST HOLE P-48

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-4	Sand; medium to coarse; moderately sorted; no fines.	
4-7	Clay; lacustrine; occasional pebble; massive to slightly laminated; silt balls.	lacustrine clay unit
7-24	Till; silty, sandy, slightly clayey;	clay-loam till
24-26	Sand; silty, pebbly, bouldery; dark grey 5Y	basal sandy drift

TEST HOLE P-49

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-10	Sand; upper 5 feet medium grained moderately sorted, 2.5YR3/6; lower 4 feet slightly silty with occasional pebble.	lacustrine sand and gravel unit
10-23	Clay, lacustrine; massive; dark grey 5Y3.5/1.	lacustrine clay unit
23-53	Till; clayey, silty, sandy; very plastic.	clay-loam till

TEST HOLE P-50

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-8	Clay; lacustrine; massive to laminated; abundant silt balls; occasional pebble.	lacustrine clay unit
8-15	Transition to till.	
15-47	Till; clayey, silty, sandy; carbonate and igneous pebbles; olive 5Y to 20 feet and dark grey 5Y below.	clay-loam till
47	End of hole on boulder or bedrock.	

TEST HOLE P-51

Depth (ft.)	Characteristics	Interpretation
0-3	Road fill.	
3-7	Clay; lacustrine; silt balls and occasional pebble; gypsum concentrations; massive to slightly laminated.	lacustrine clay unit
7-10	Transition to till.	
10-25	Till; clayey, silty, sandy; igneous and carbonate pebbles.	clay-loam till
25-27	Boulders.	
27-50	Sand; medium grading to coarse near bottom; pebbly; poorly sorted near bottom; no fines.	basal sandy drift
50	End of hole in boulders.	

TEST HOLE P-52

Depth (ft.)	Characteristics	Interpretation
0-2	Road fill.	

TEST HOLE P-52 Cont'd.

2-12	Silty clay and clay; laminae of very fine sand; pebbly with silt balls.	lacustrine silt unit
12-30	Clay; lacustrine; silt balls and occasional pebble; massive to slightly laminated.	lacustrine clay unit
30-40	Till; clayey, silty, sandy; plastic and soft.	clay-loam till
40	End of hole on boulder.	

TEST HOLE P-53

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-7	Clayey silt and silty clay; laminae of very fine sand.	lacustrine silt unit
7-12	Transition to well sorted clay.	
12-27	Clay; laminated to massive; silt balls and occasional pebble.	lacustrine clay unit
27-35	Till; clayey, silty, sandy; very soft.	clay-loam till
35-37	Till and boulders; very sandy.	basal sandy drift
37	End of hole in boulders.	

TEST HOLE P-54

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-5	Clayey silt with laminations of very fine sand.	lacustrine silt unit
5-14	Clay; lacustrine; silt balls and occasional pebble; massive to slightly laminated.	lacustrine clay unit
14-26	Till; clayey, silty, sandy.	clay-loam till
26	End of hole on boulder.	

TEST HOLE P-55

Depth (ft.)	Characteristics	Interpretation
0-2	Top soil.	
2-3	Clayey silt.	lacustrine silt unit
3-15	Clay; lacustrine; silt balls and occasional pebble; secondary gypsum concentrations.	lacustrine clay unit
15-27	Till; clayey, silty; soft, oxidized.	basal sandy drift

TEST HOLE P-56

Depth (ft.)	Characteristics	Interpretation
0-7	Silty clay and clayey silt; laminae of very fine sand; lower portion has less silt, occasional pebble and silt balls.	lacustrine clay unit
7-23	Till; clayey, silty, sandy.	clay-loam till
23-26	Boulders and sand.	basal sandy drift
26	End of hole on rock.	

TEST HOLE P-57

Depth (ft.)	Characteristics	Interpretation
0-4	Silty clay and clayey silt; laminated with very fine sand.	lacustrine silt unit
4-10	Clay; massive with occasional pebble.	lacustrine clay unit
10-19	Till; clayey, silty, sandy.) clay-loam till
19-30	Till; very sandy silty; mixed pebble composition; moist; soft.	
30-41	Sand; possibly silty, very sandy till.	

TEST HOLE P-58

Depth (ft.)	Characteristics	Interpretation
0-1	Top soil.	
1-8	Silty clay.	lacustrine silt unit
8-17	Clay; lacustrine; silt balls and gypsum concentrations; laminated.	lacustrine clay unit
17-35	Till; silty, sandy.	clay-loam till

TEST HOLE P-59

Depth (ft.)	Characteristics	Interpretation
0-2	Road fill.	
2-10	Clay; slightly silty.)) lacustrine clay unit
10-25	Clay; lacustrine; silt balls; high gypsum content.	
25-59	Till; very clayey, silty, sandy.	clay-loam till

TEST HOLE P-60

Depth (ft.)	Characteristics	Interpretation
0-2.5	Road fill.	
2.5-13	Clay; silty; laminated with silt balls.)) lacustrine clay unit
13-23	Clay; lacustrine; silt balls and gypsum concentrations.	
23-40	Till; very clayey, slightly silty, sandy.	clay-loam till

APPENDIX C
PIEZOMETER HYDROGRAPHS

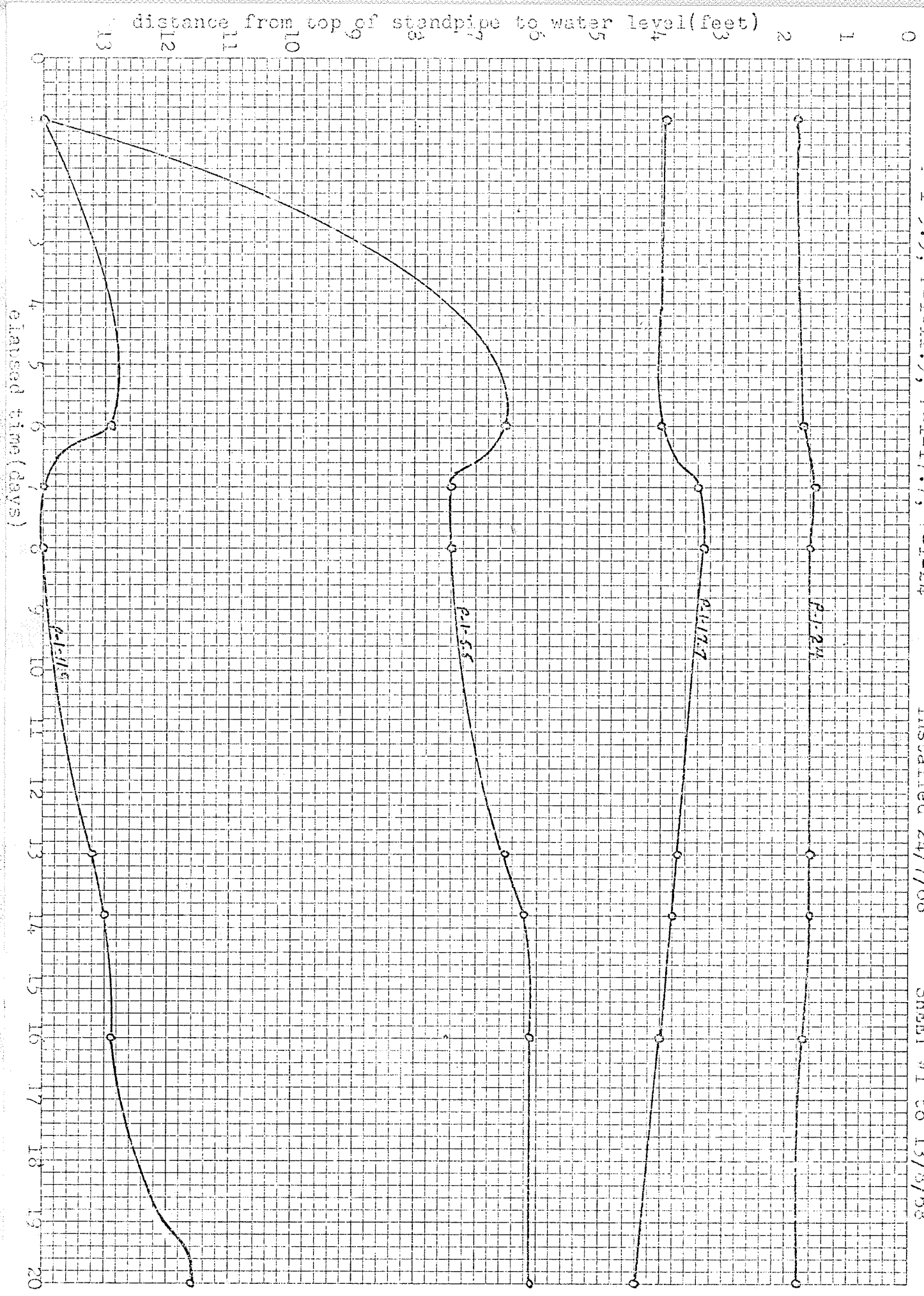
The first group of hydrographs is arranged so that, for each nest, a 20-day hydrograph is followed by a 100-day hydrograph. This allows a more convenient comparison of the initial stabilization period to a seasonal fluctuation. These hydrographs were prepared only for piezometer nests 1 through 14 installed during the summer of 1968. The 20-day hydrographs were prepared before survey data was available and list water levels from the top of the standpipe rather than an elevation. However, they are useful in illustrating short-term response characteristics.

Two-year hydrographs of piezometer nests 1 through 40 were prepared to illustrate seasonal and annual fluctuations. Precipitation bar graphs indicate the relative sensitivity of each hydrograph to short-term and long-term variations in precipitation.

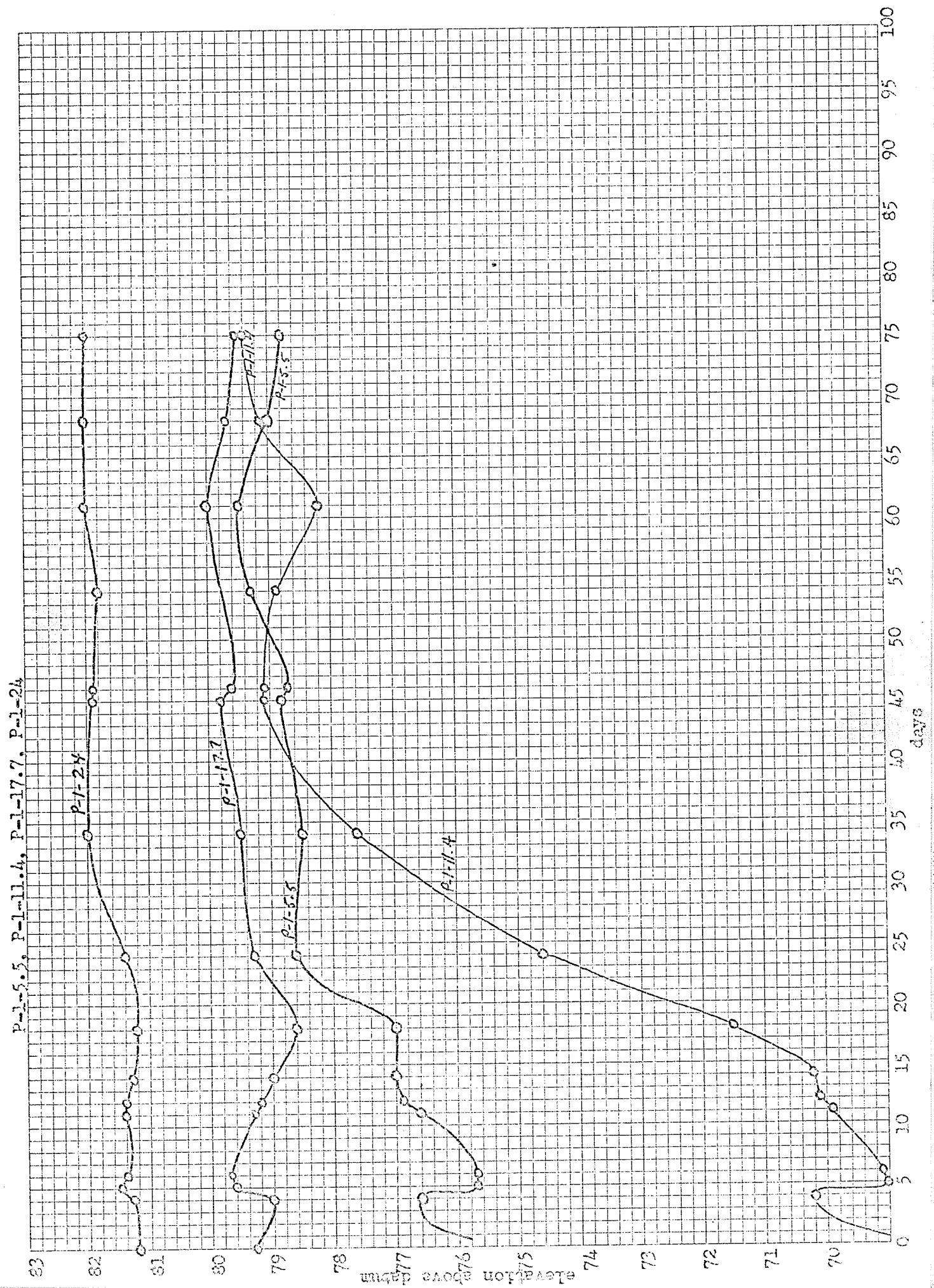
P-1-5.5, P-1-11.9, P-1-17.7, P-1-24

installed 24/7/68

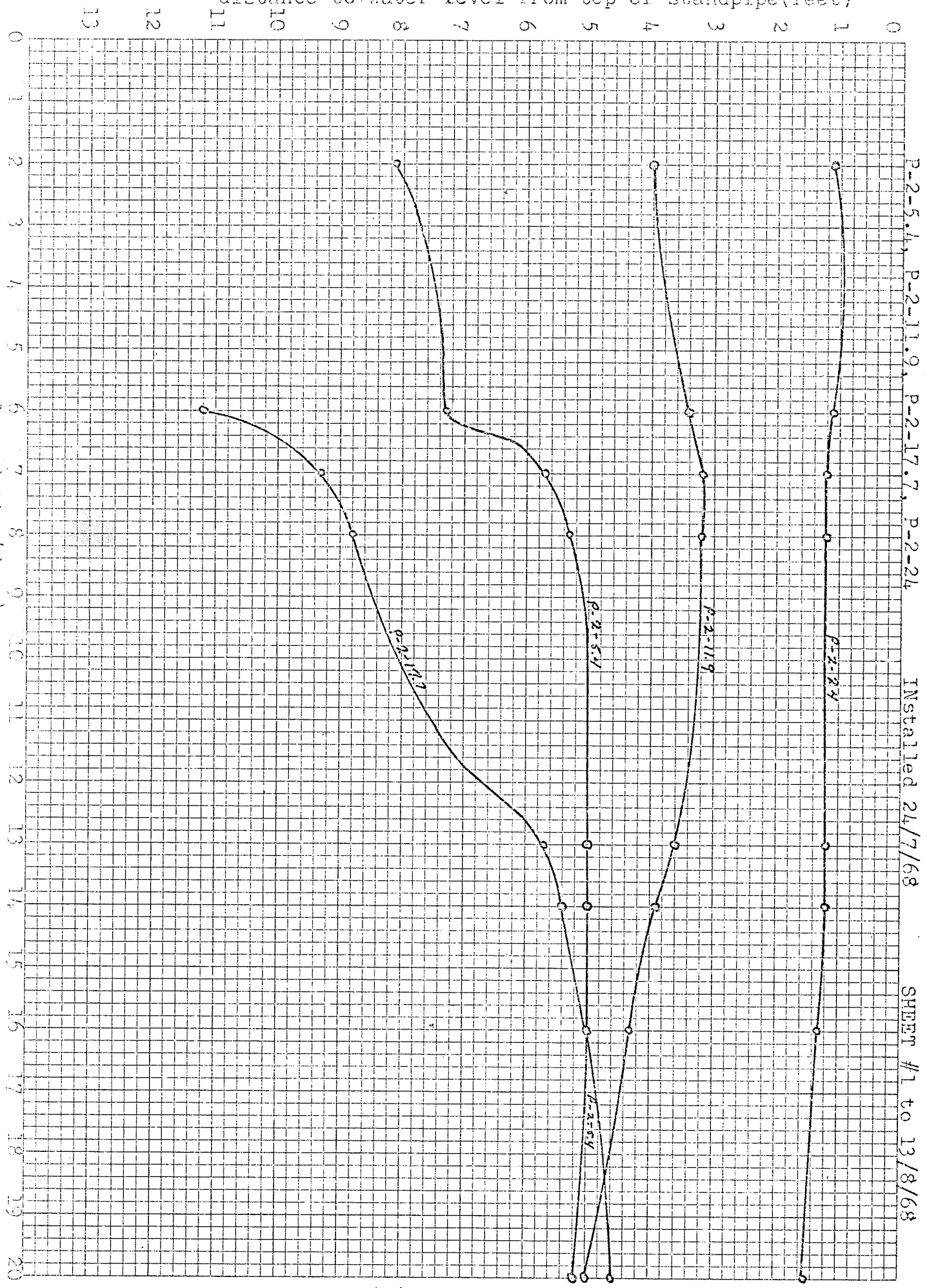
SHEET #1 to 13/8/68



last point 13/8/68



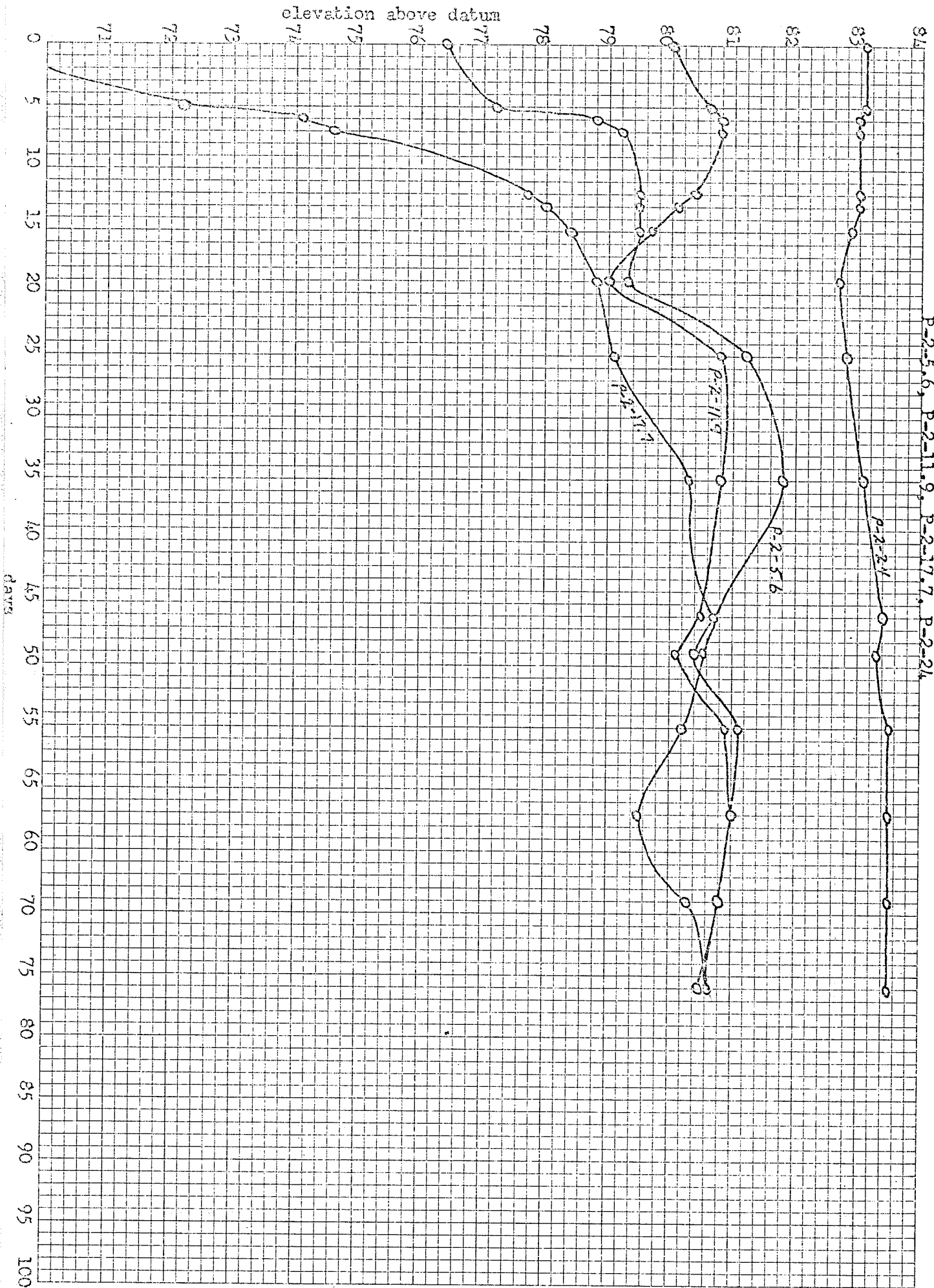
distance to water level from top of standpipe (feet)



P-2-5.4, P-2-11.9, P-2-17.7, P-2-24
Installed 24/7/68
SHEET #1 to 13/8/68

last point 13/8/68

Station (miles)

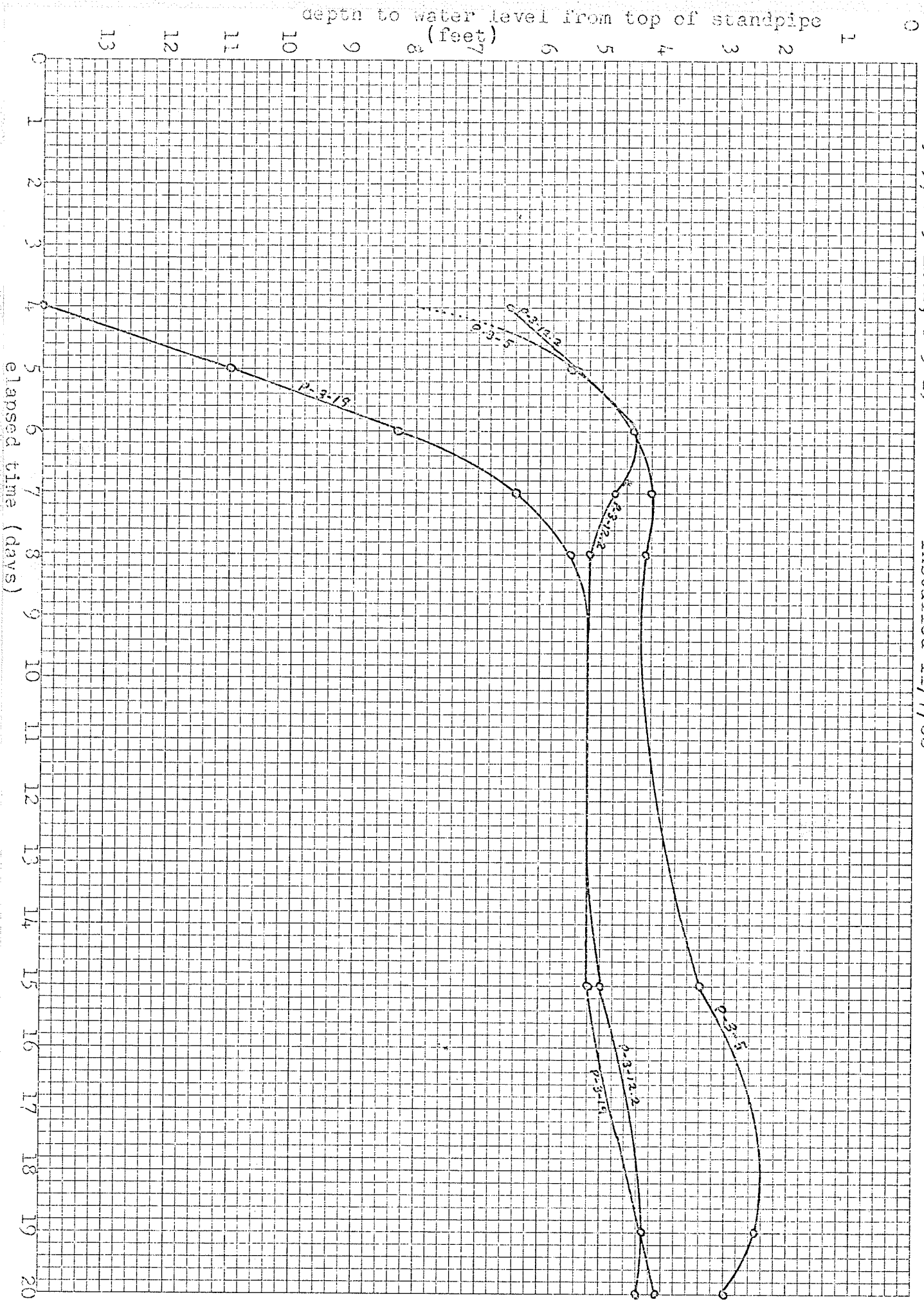


5 X 5 PER HALF INCH

EGGERT DIE CASTING CO.
MADE IN U. S. A.

P-3-5, P-3-12.2, P-3-19
Installed 11/7/68

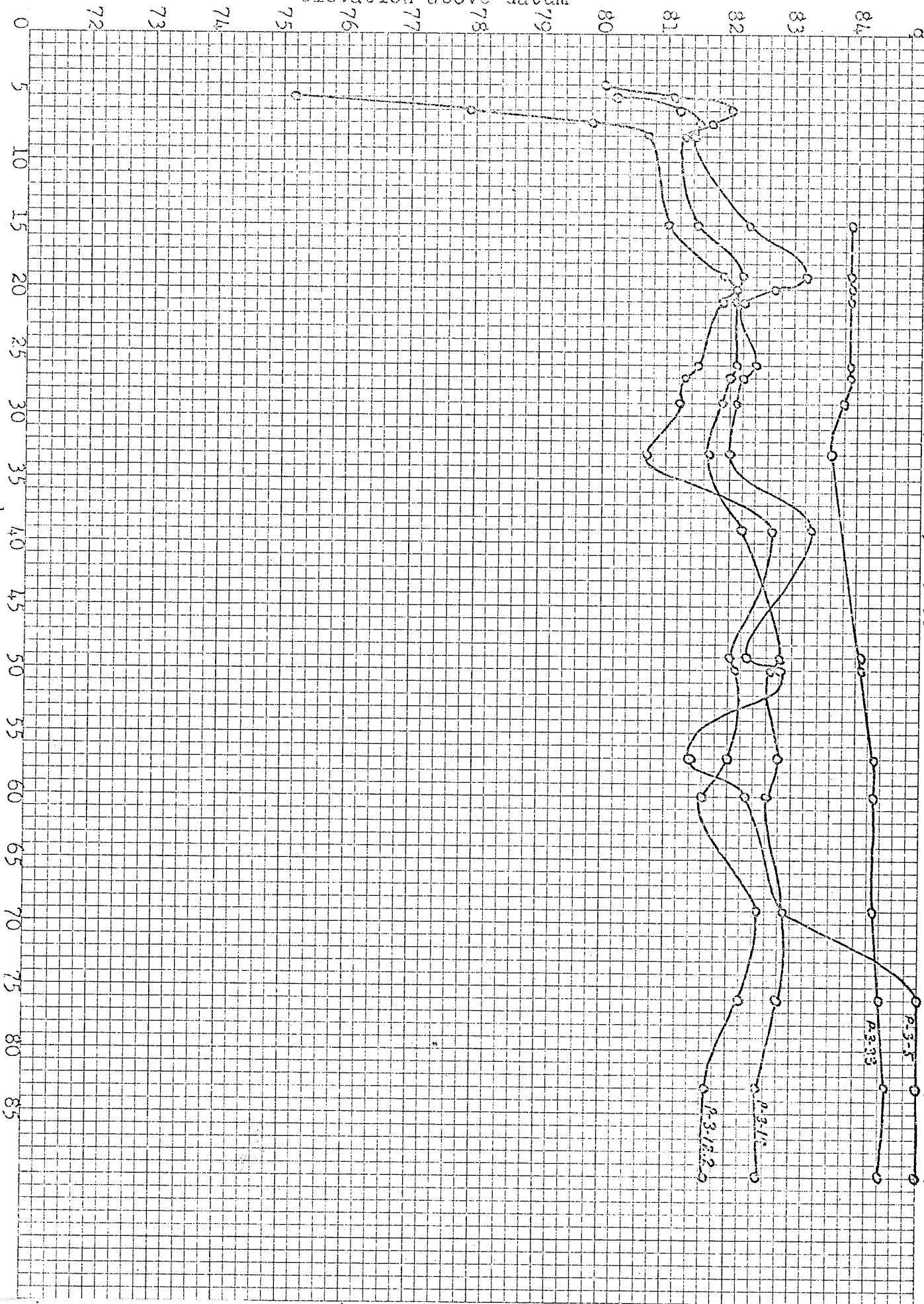
SHEET #1



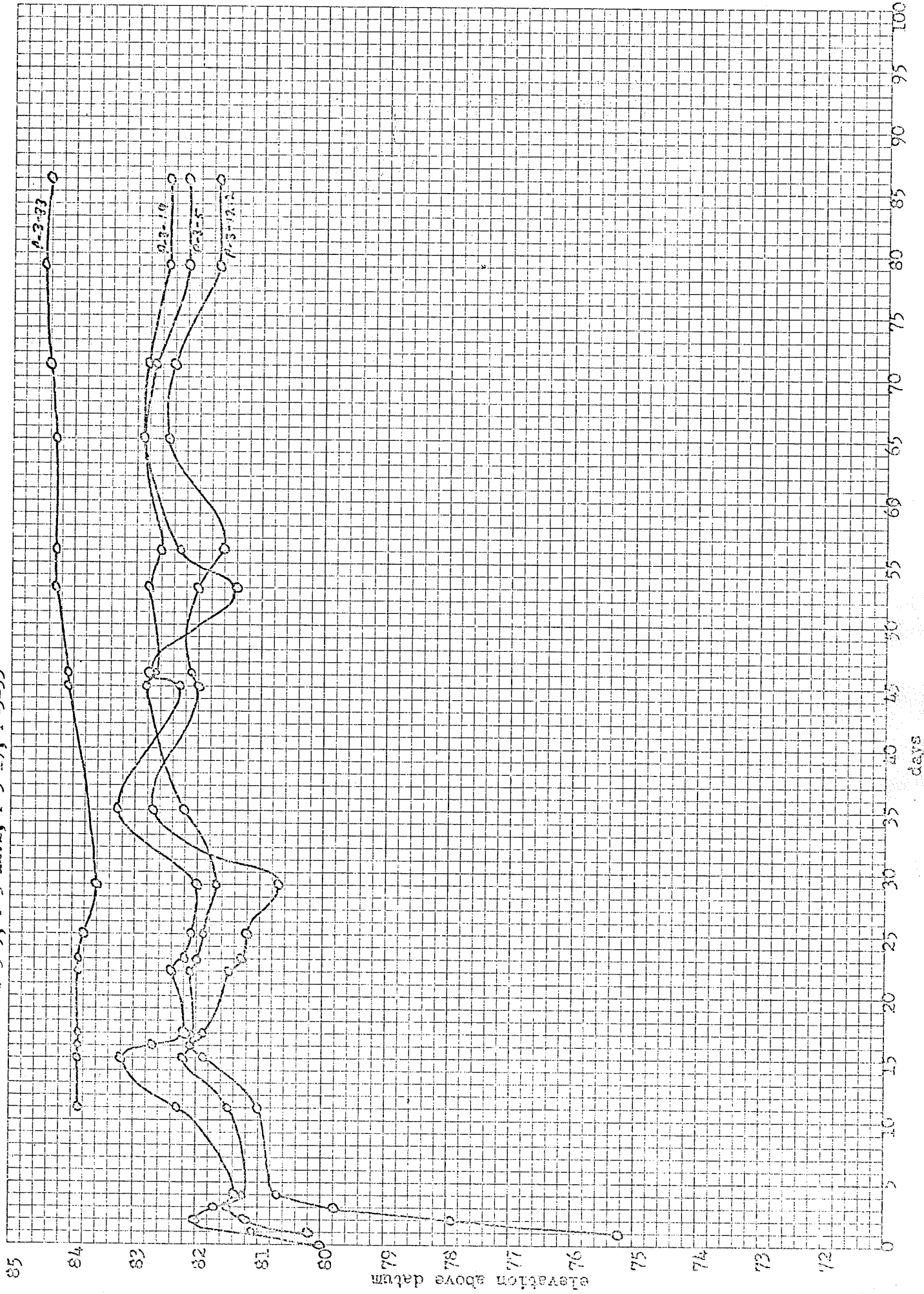
last point 31/7/68

P-3-5, P-3-12.2, P-3-19, P-3-33

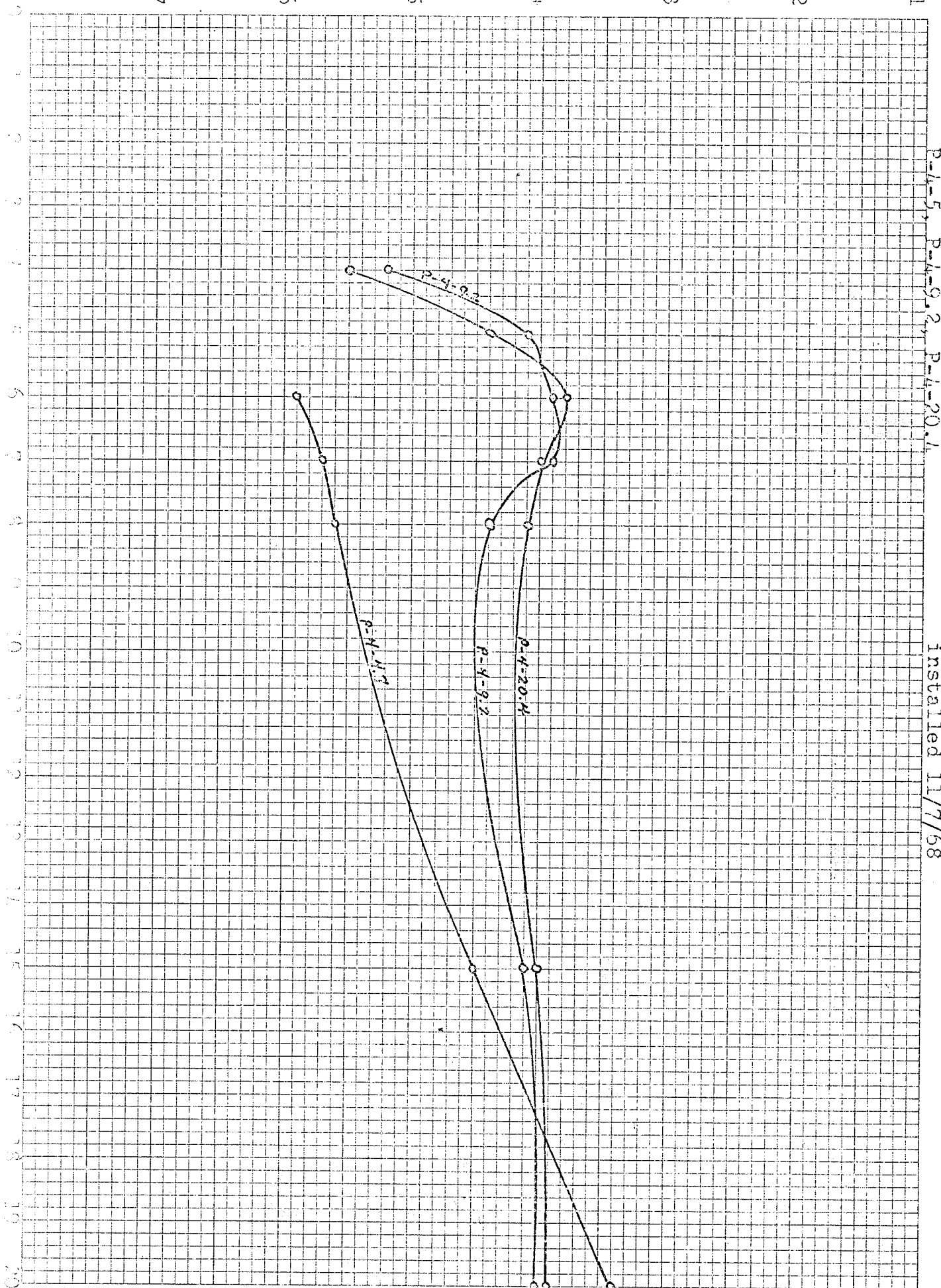
elevation above datum



P-3-5, P-3-12.2, P-3-19, P-3-33



distance to water level from top of standpipe(feet)



P-4-5, P-4-9, P-4-20.1

installed 11/7/68

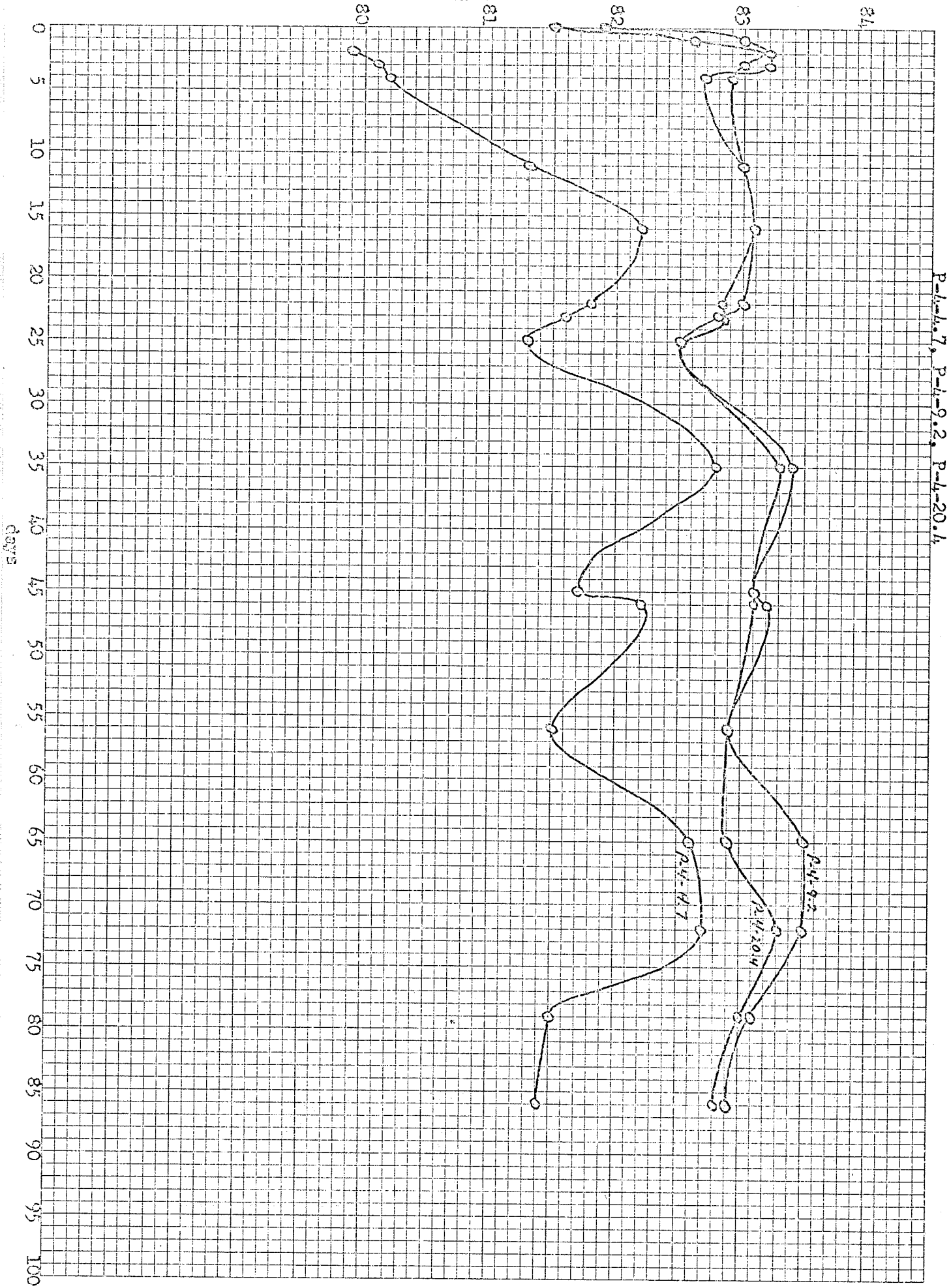
SHEET #1 to July 31

5 X 5 PER HALF INCH

EUGENE DIECKMANN CO.
MADE IN U. S. A.

last point 31/7/68

elevation above datum

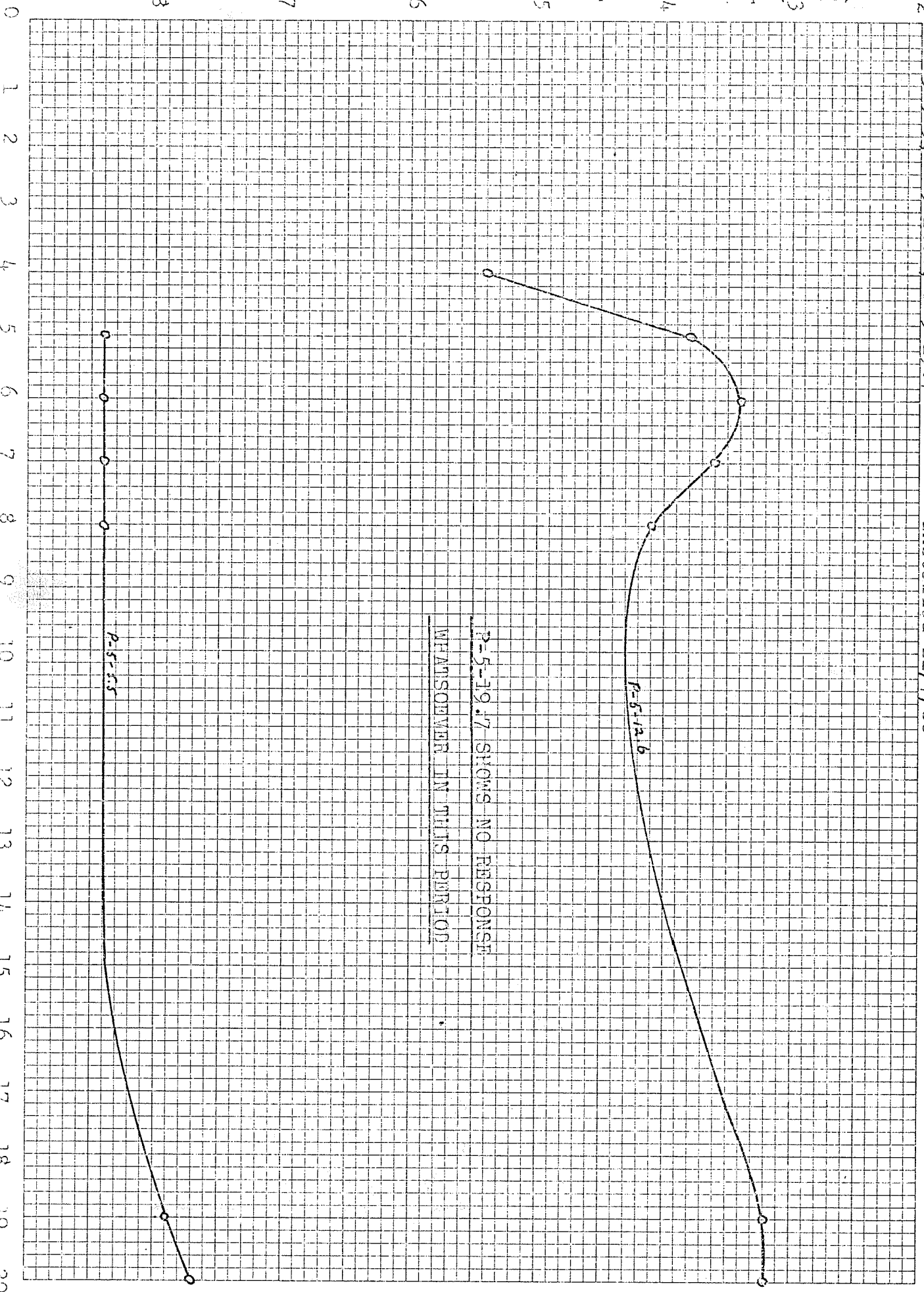


S X S PER HALF INCH

MADE IN U. S. A.

2 P-5-5.5, P-5-12.6, P-5-19.7 installed 11/7/68

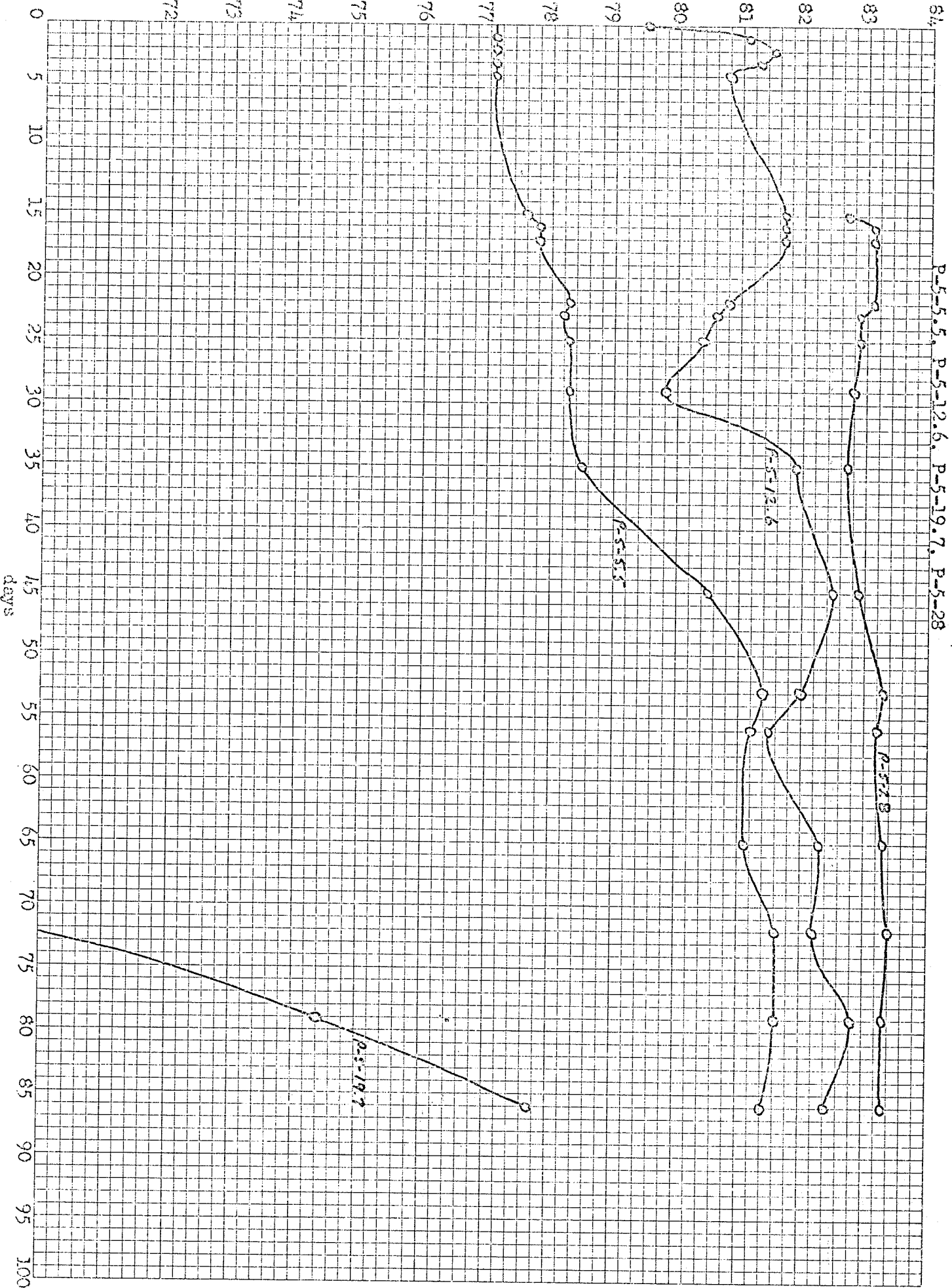
distance to water level from top of standpipe (feet)



P-5-19.7 SHOWS NO RESPONSE
WHATEVER IN THIS PERIOD

last point July 31/1968

elevation above datum



distance to water level from top of standpipe (feet)

6

5

4

3

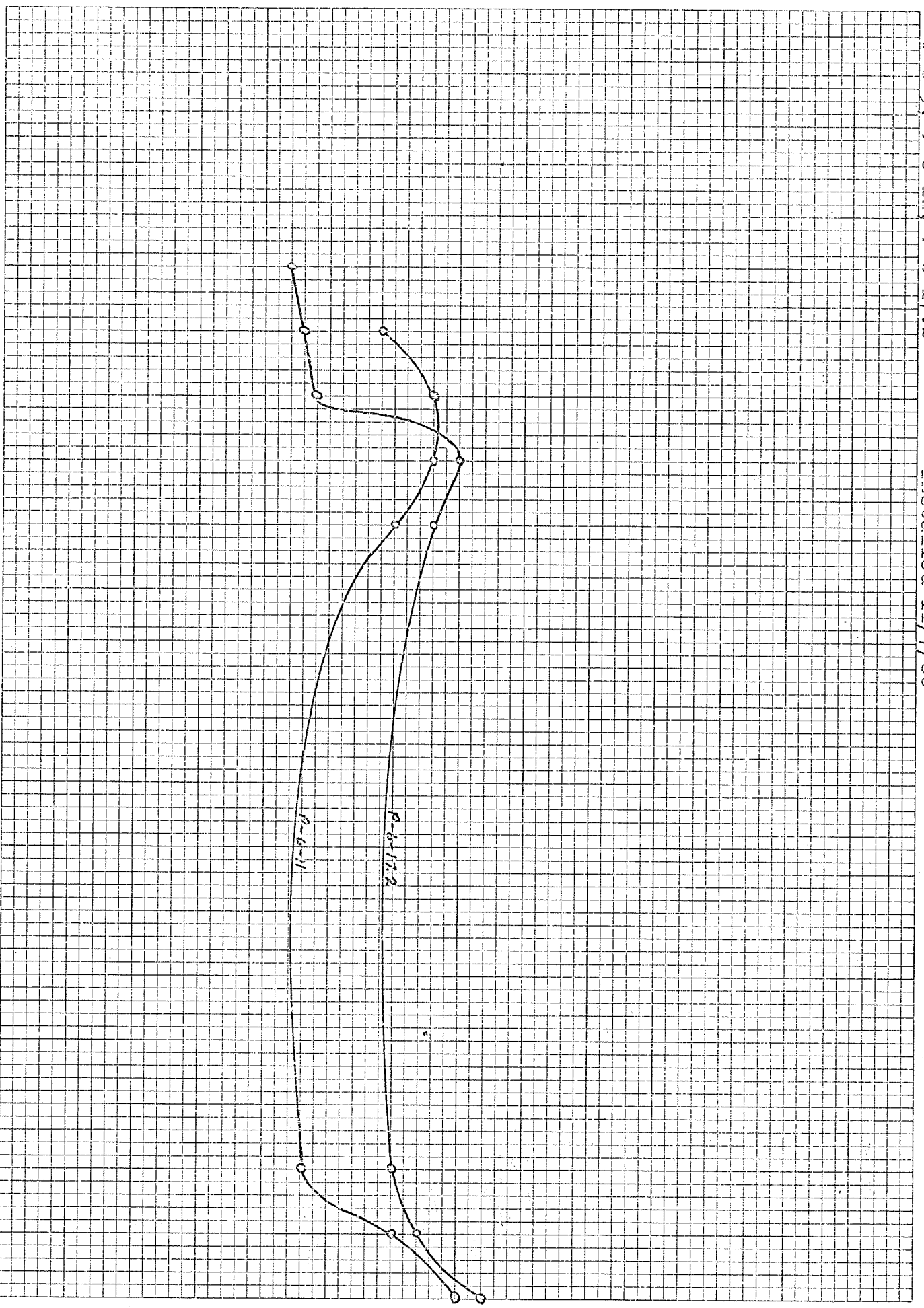
2

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P-6-5, P-6-11, P-6-17.2 installed 11/7/68



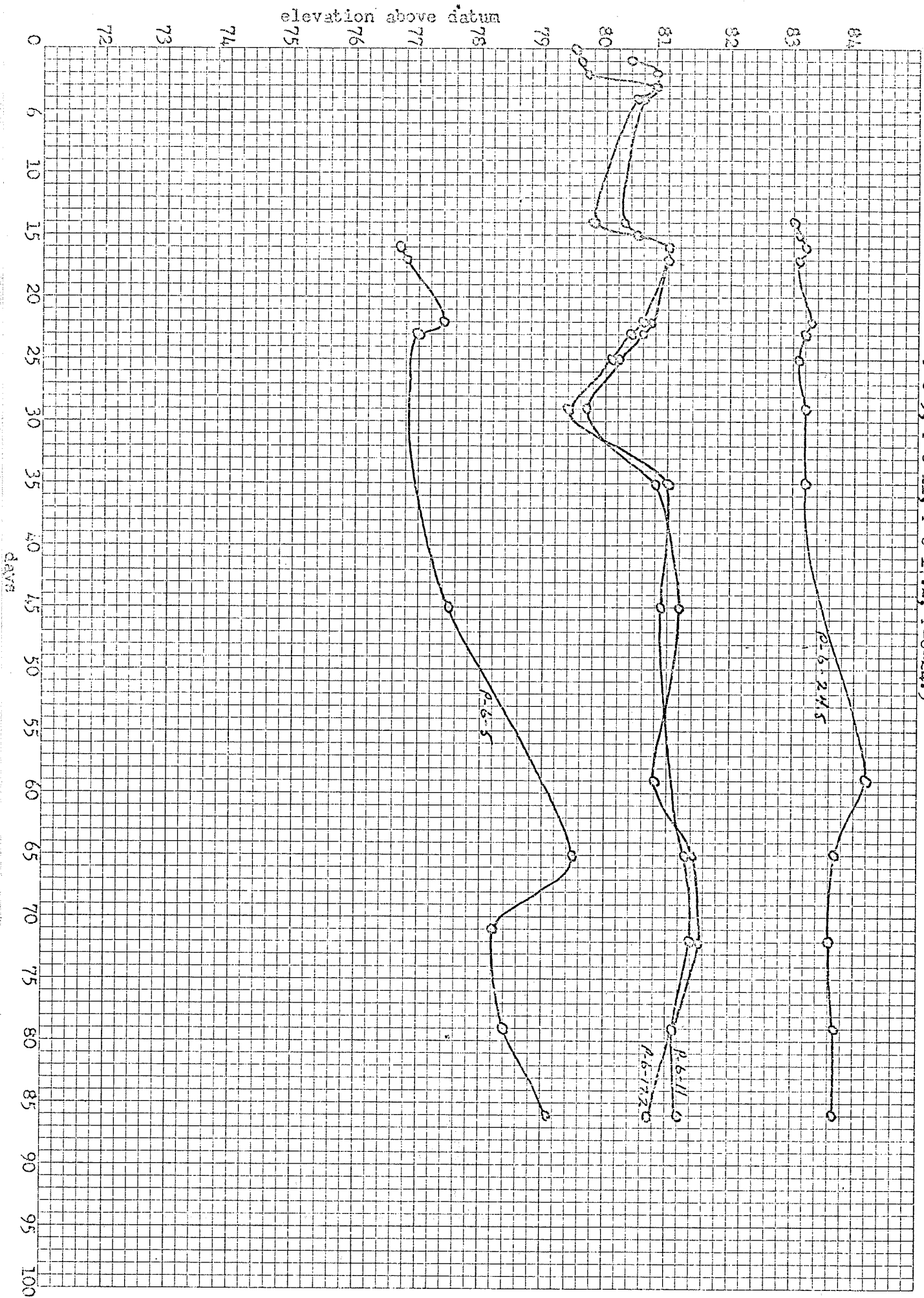
5 X 5 PER HALF INCH

MADE IN U. S. A.

SHEET #1 to July 31/1968

last point 31/7/68

P-6-5, P-6-11, P-6-17.2, P-6-24.5



distance to water level from top of standpipe (feet)

3

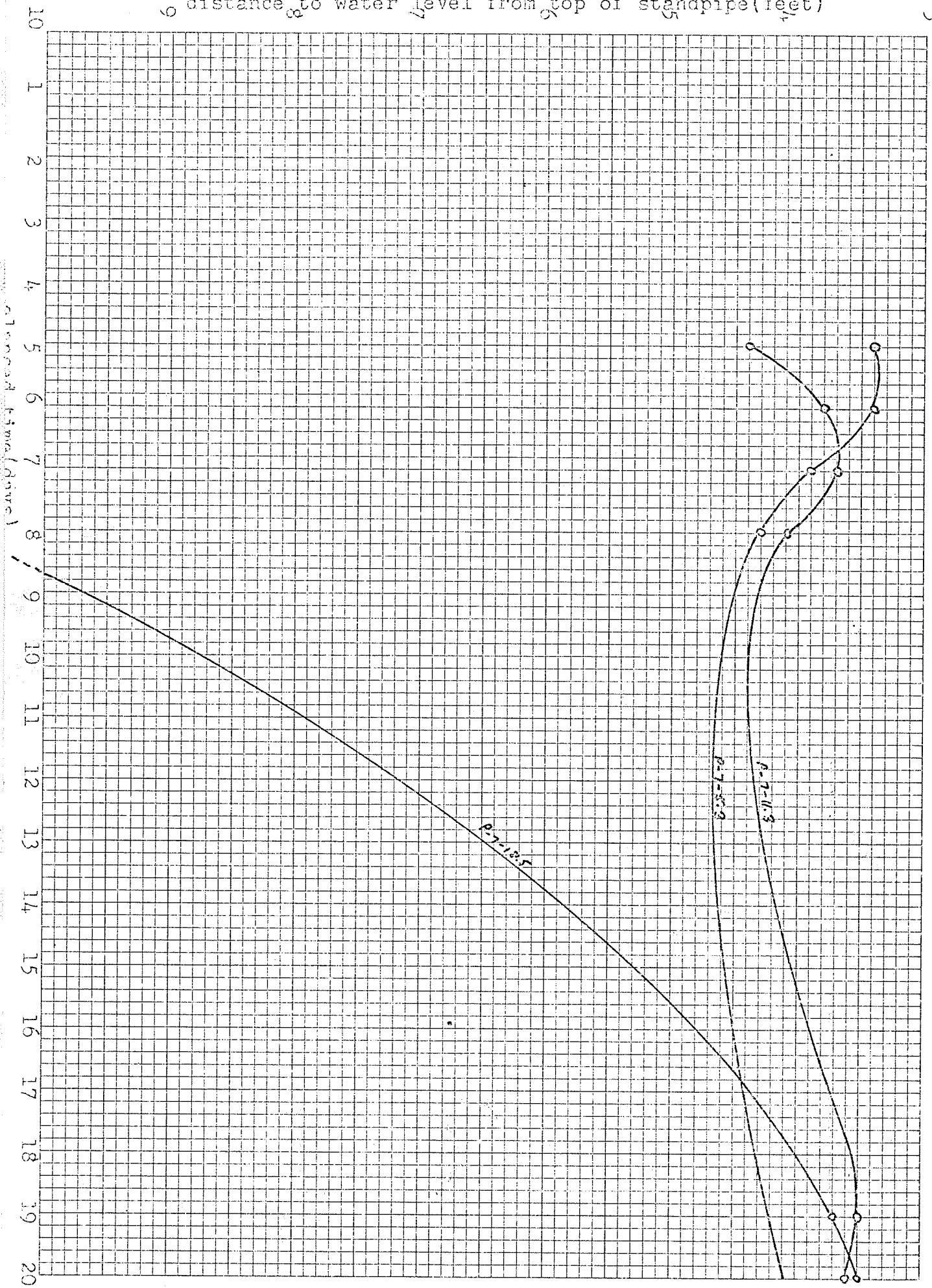
P-7-5.3, P-7-11.3, P-7-18.5

Installed 11/7/68

SHEET #1 to 31/7/68

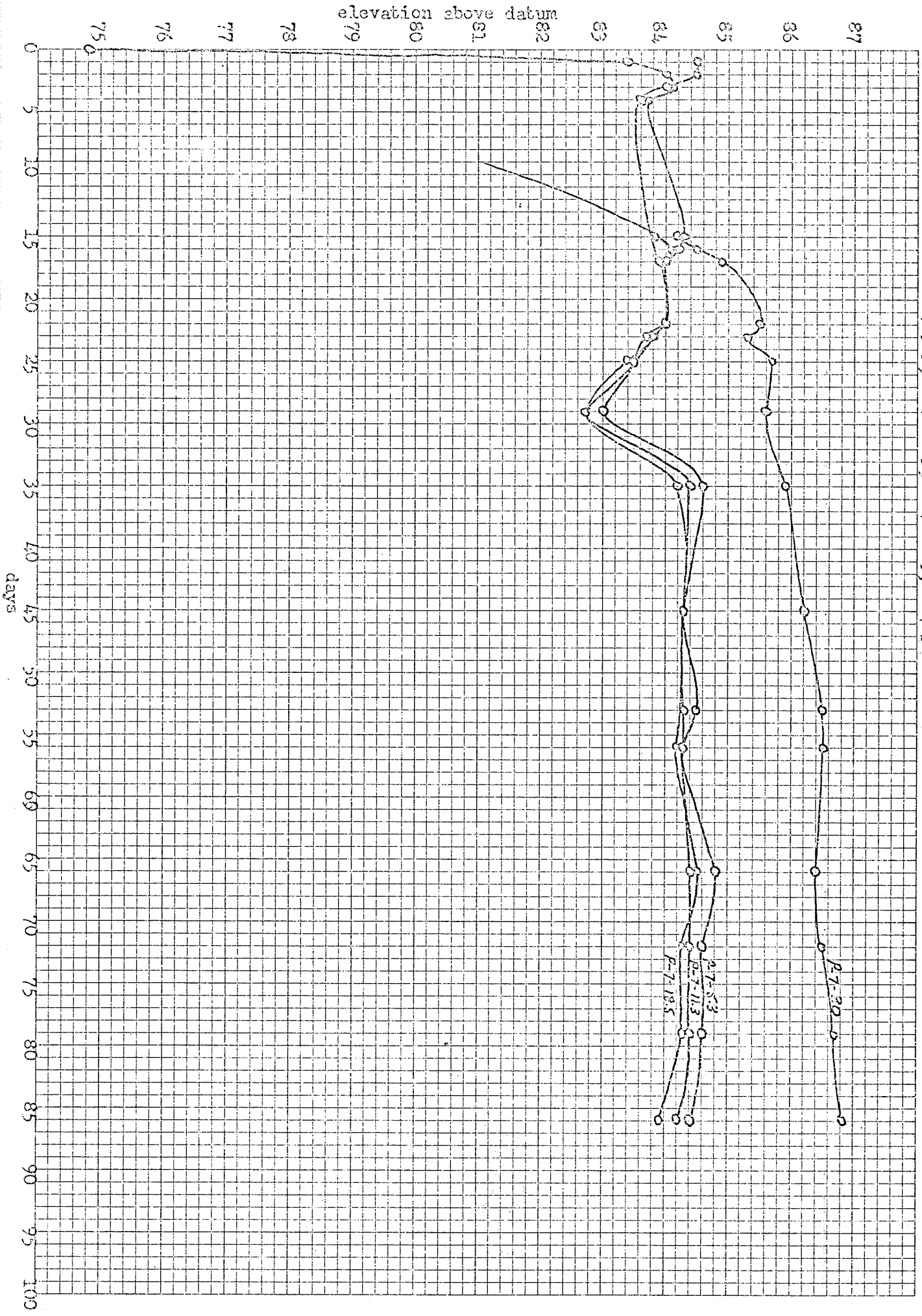
5 X 5 PER HALF INCH

MADE IN U. S. A.

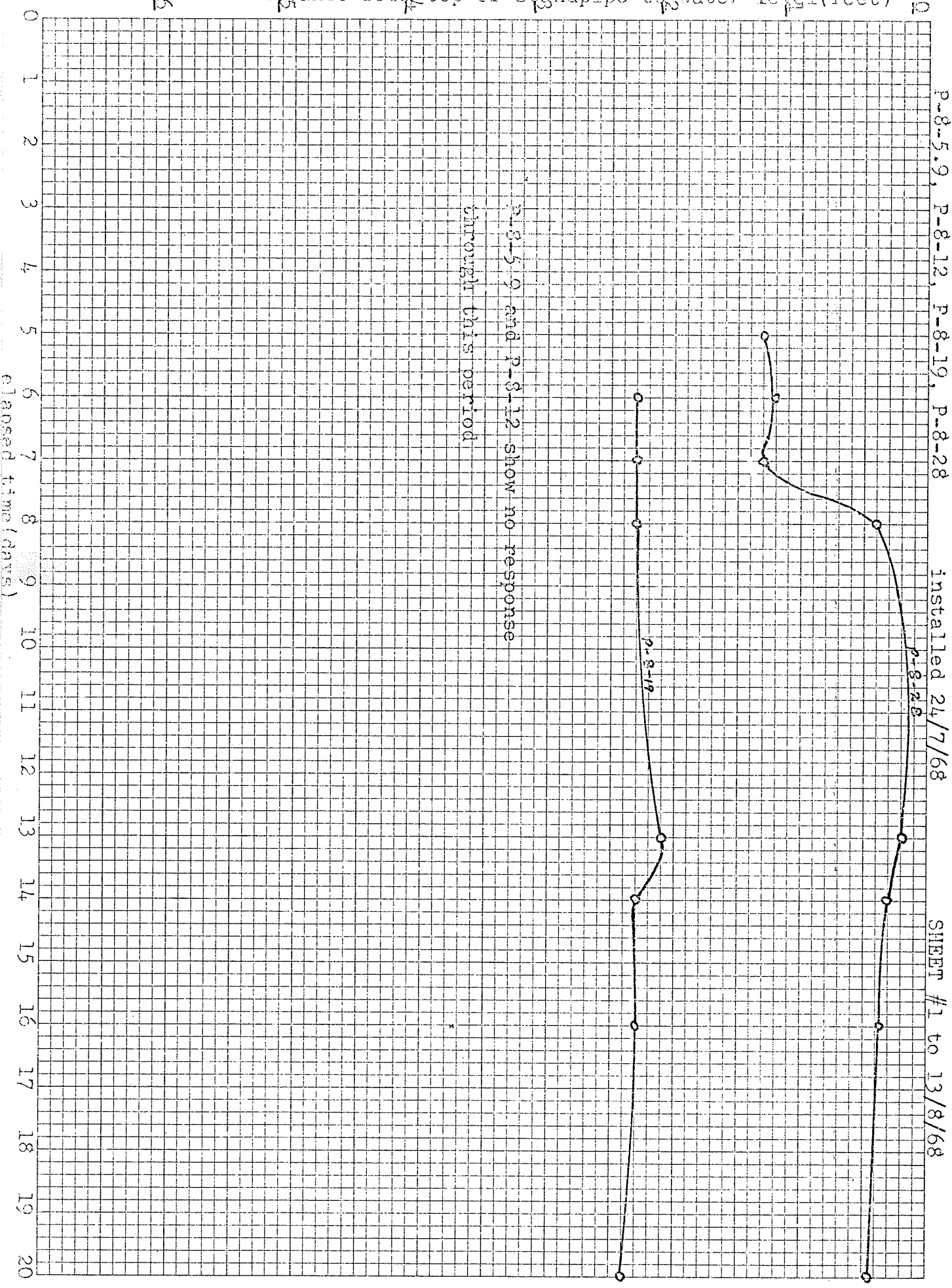


last point 31/7/68

P-7-5.3, P-7-11.3, P-7-18.5, P-7-30

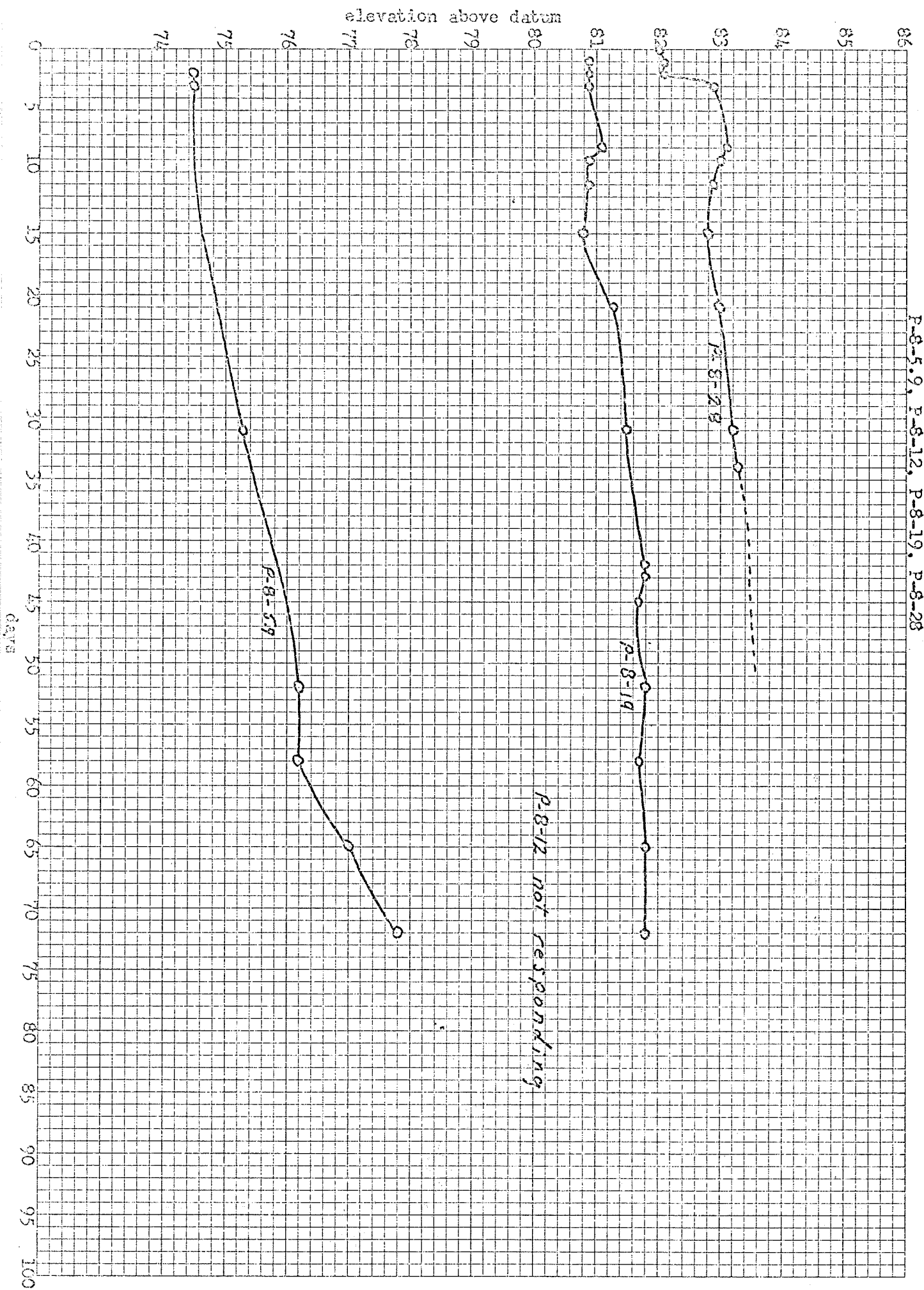


distance from top of standpipe to water level (feet)



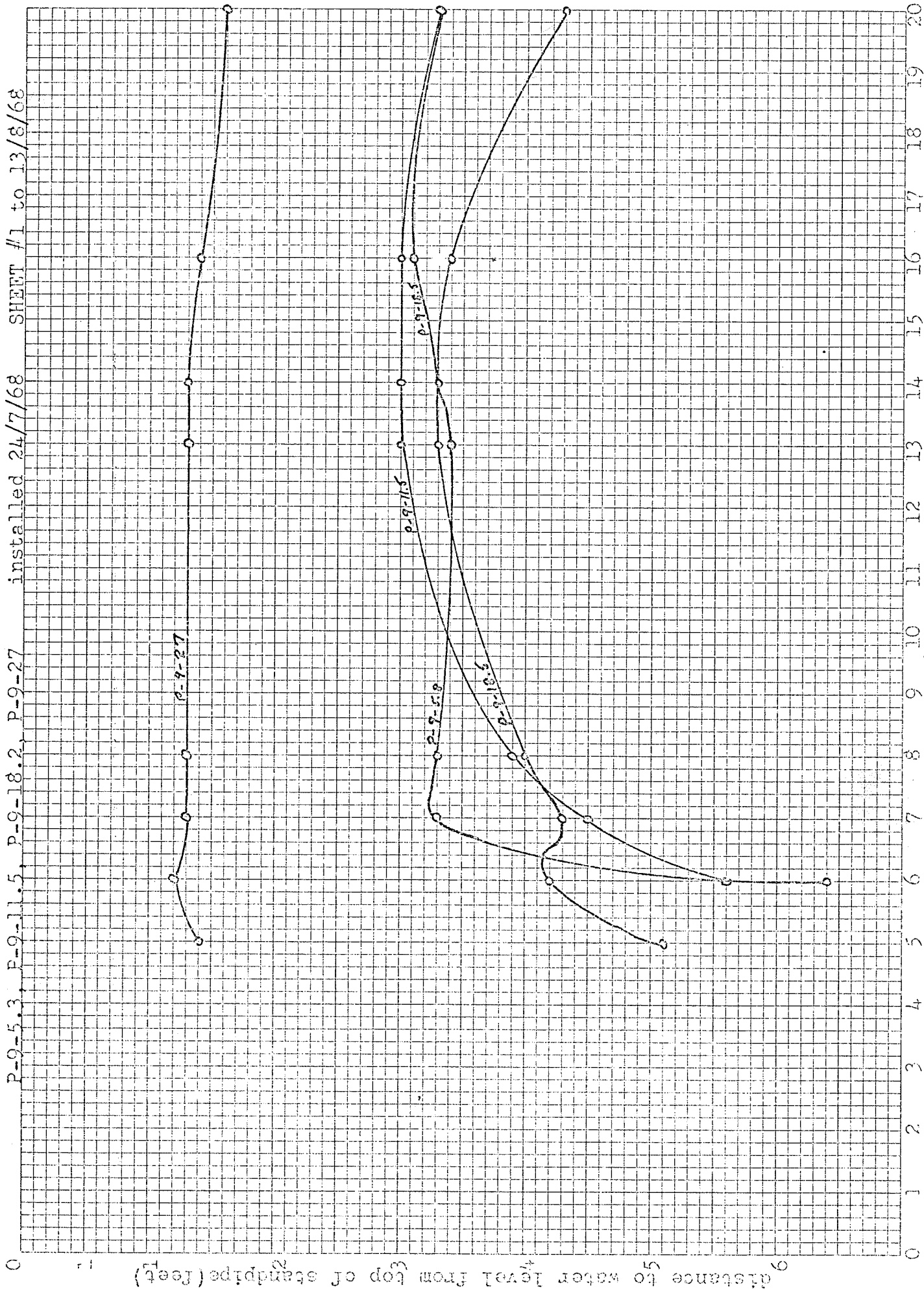
last point 13/8/68

SHEET #1 to 13/8/68



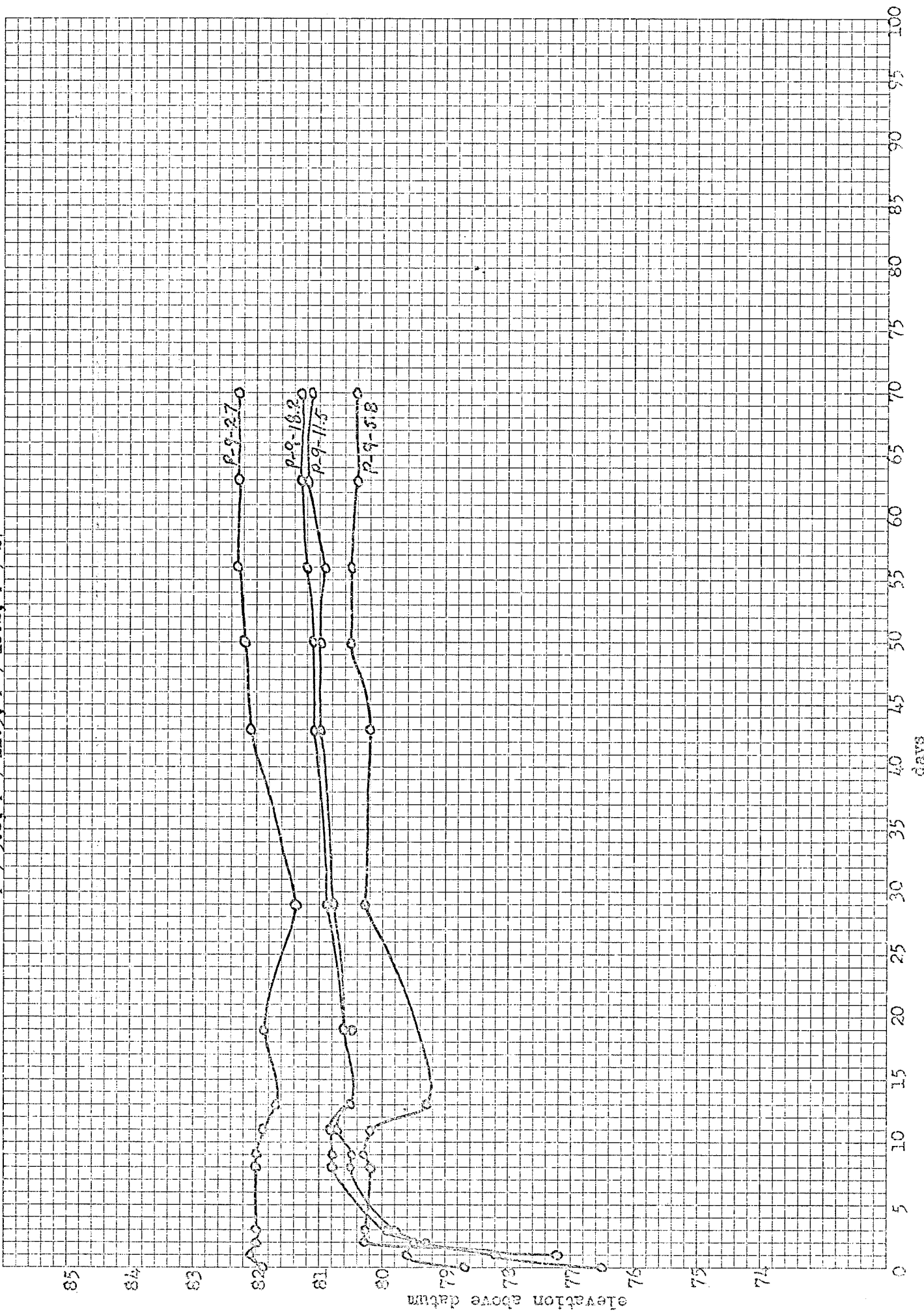
P-8-12 not responding

P-9-5.3, P-9-11.5, P-9-18.2, P-9-27 installed 24/7/68 SHEET #1 to 13/8/68



last point 13/8/68

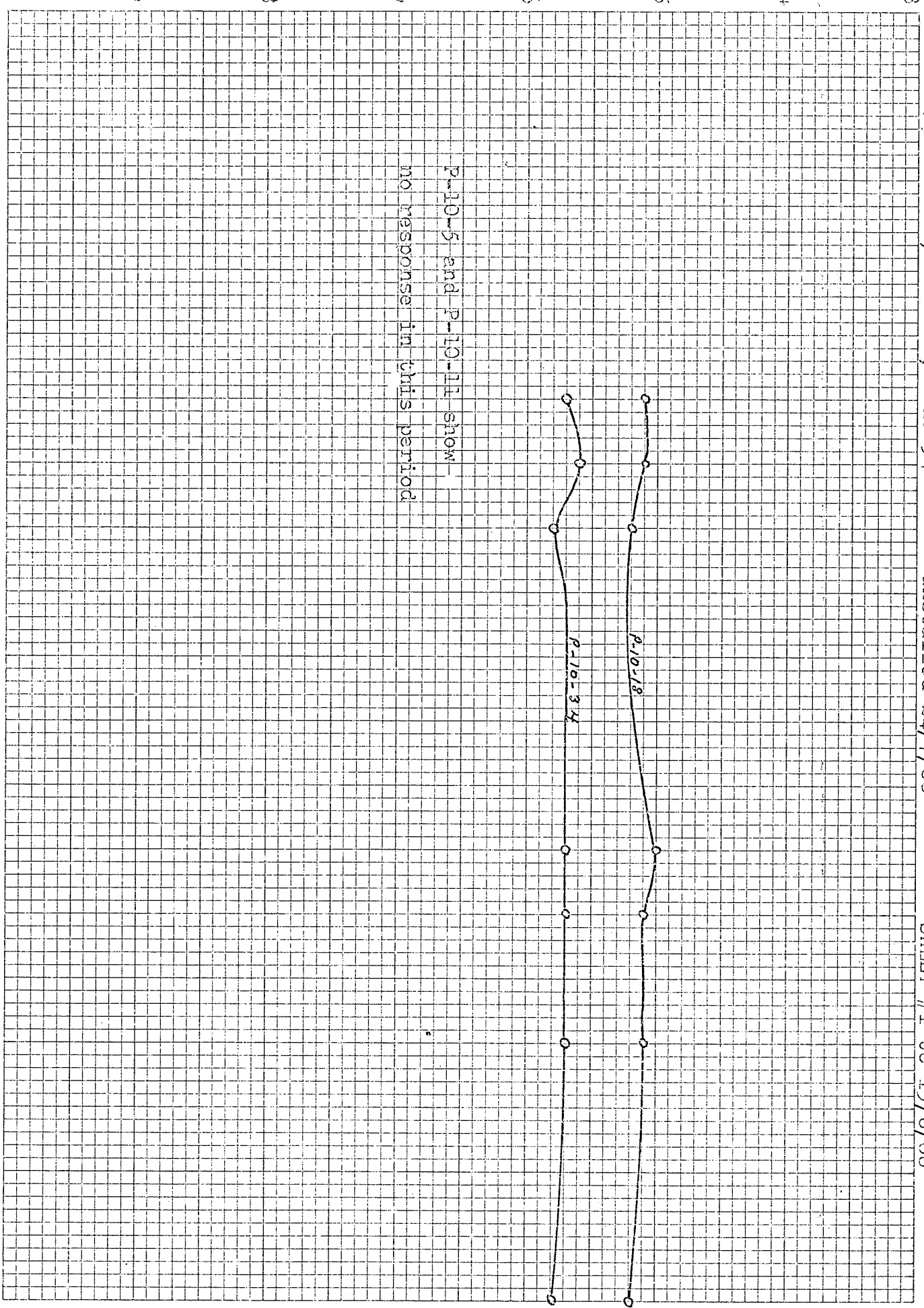
P-9-5.8, P-9-11.5, P-9-18.2, P-9-27



distance to water level from top of standpipe(feet)

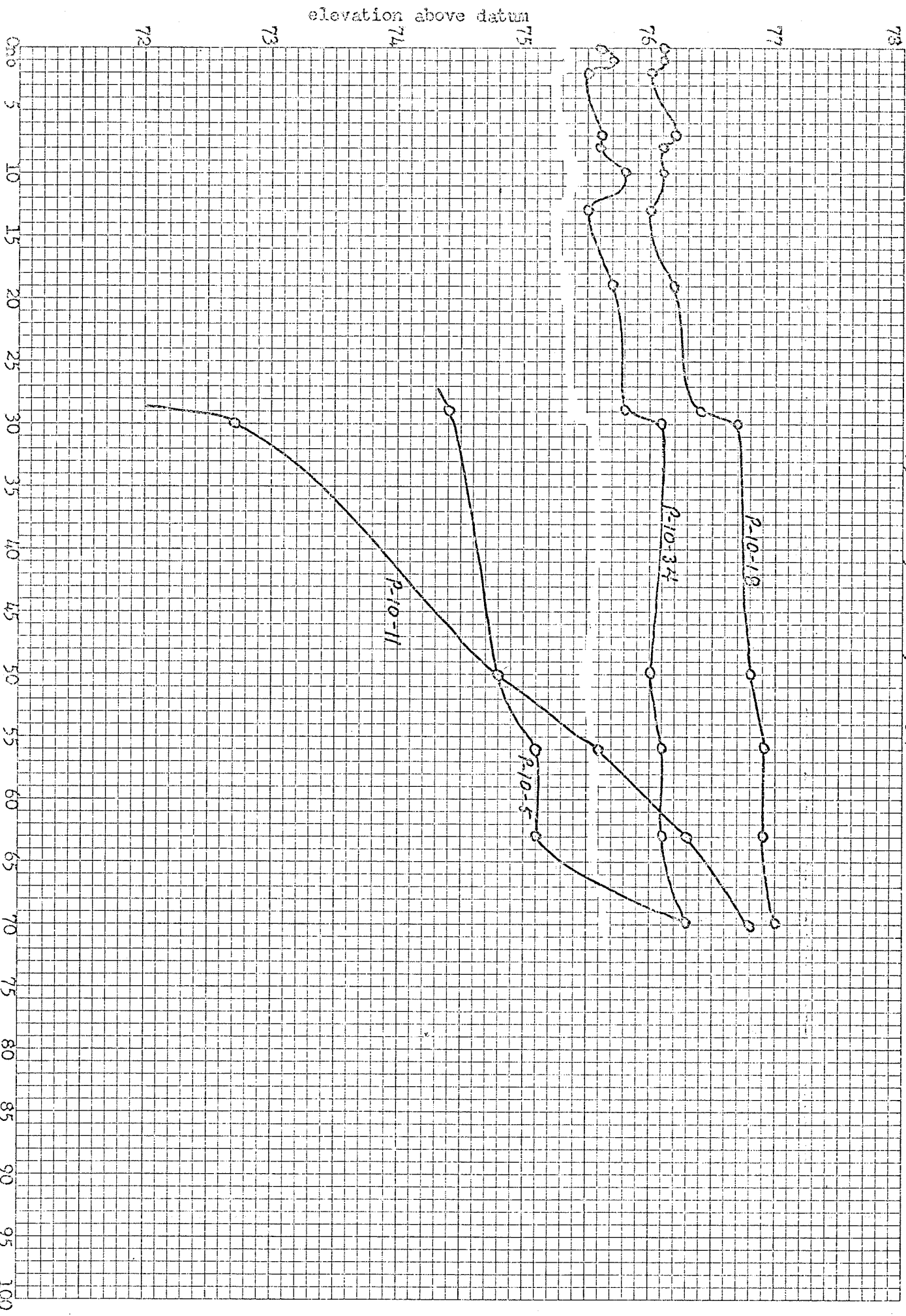
P-10-5, P-10-11, P-10-18, P-10-34 installed 24/7/68 SHEET #1 to 13/8/68

P-10-5 and P-10-11 show no response in this period



last point 13/8/68

P-10-5, P-10-11, P-10-18, P-10-34



P-11-5.5, P-11-13.3, P-11-22.8, P-11-35

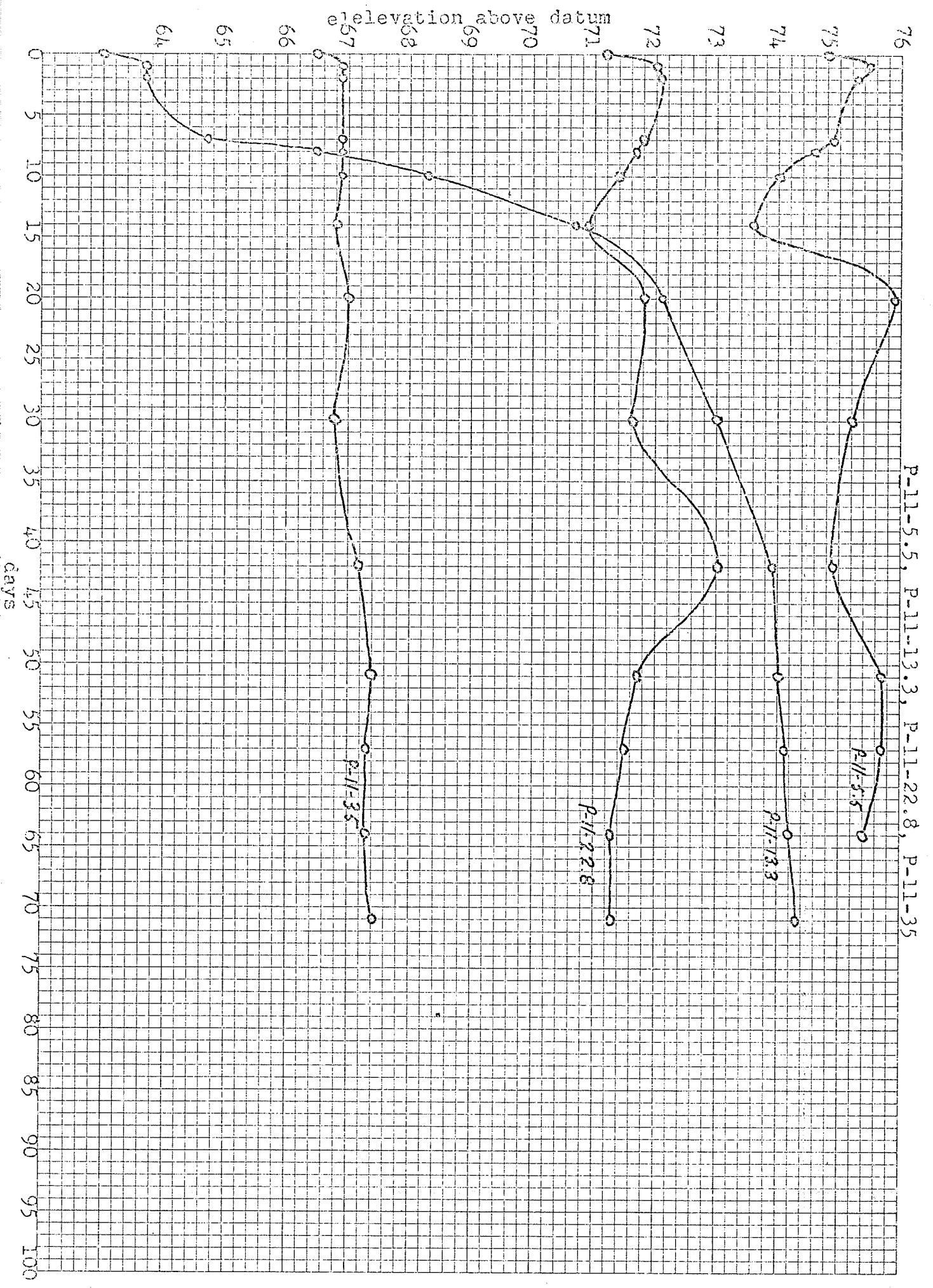
installed 24/7/68

SHEET #1 to 13/8/68

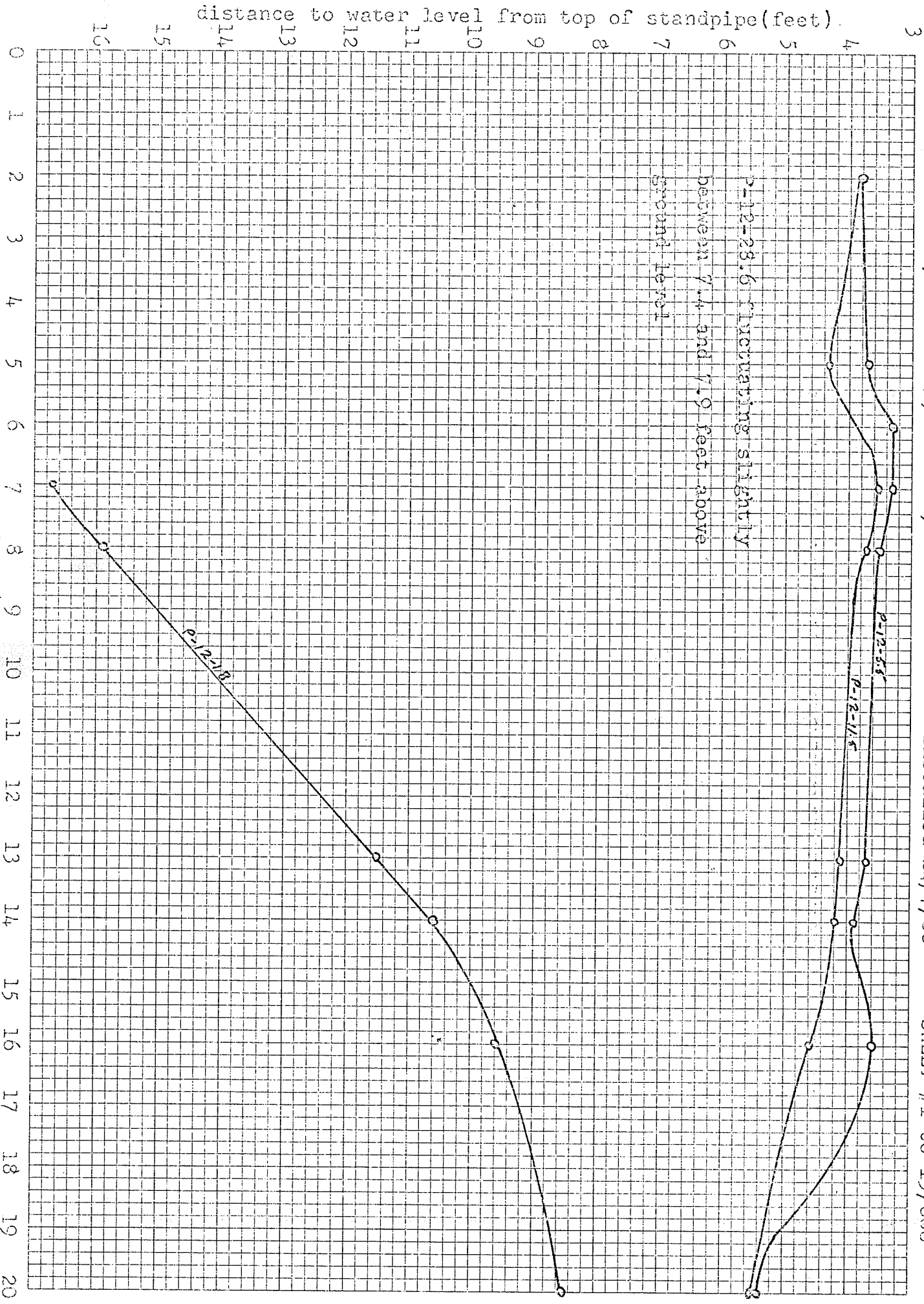
distance to water level from top of standpipe(feet)



last point 13/8/68

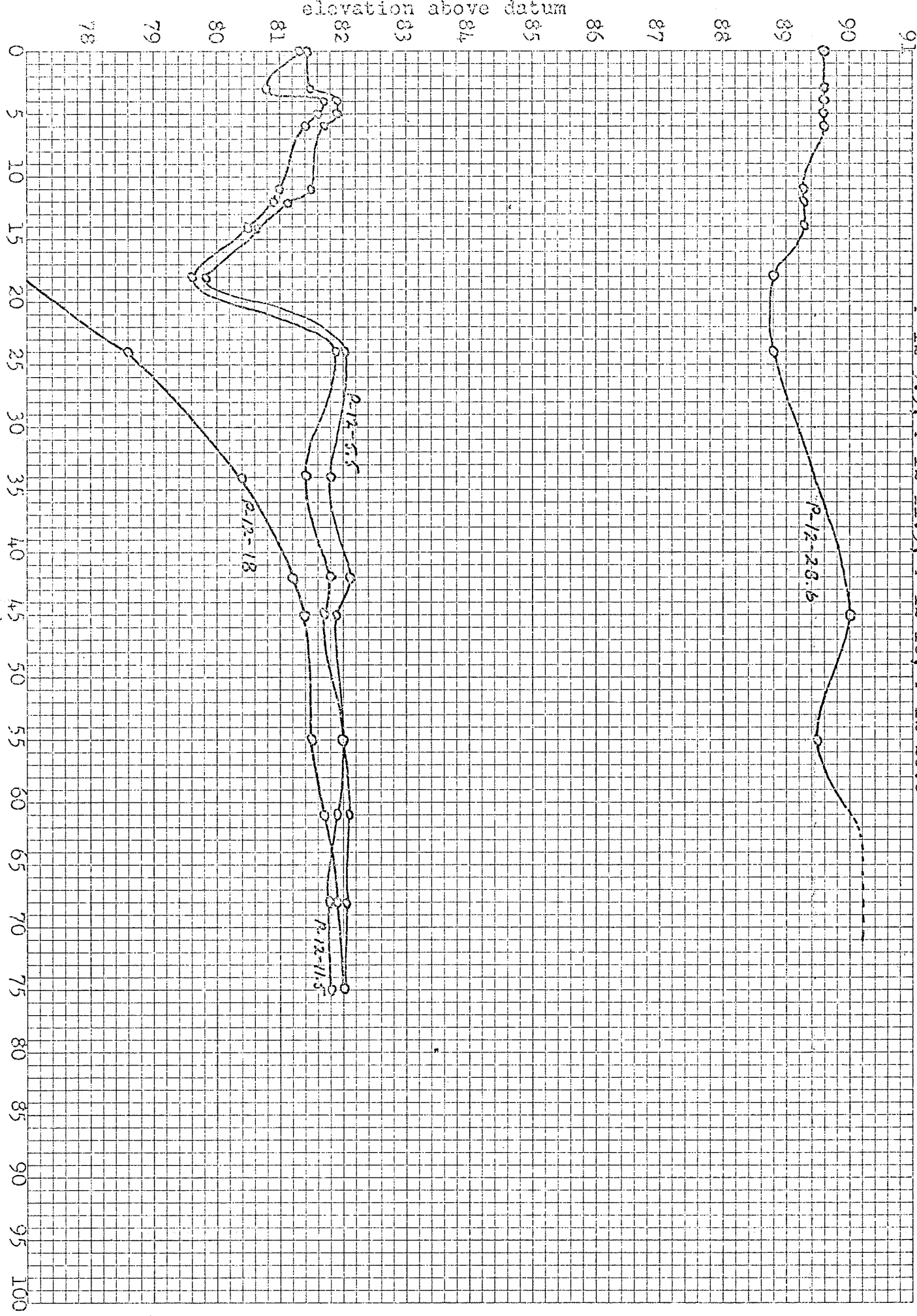


P-12-5.5, P-12-11.5, P-12-18, P-12-28.6 installed 24/7/68 SHEET #1 to 13/868

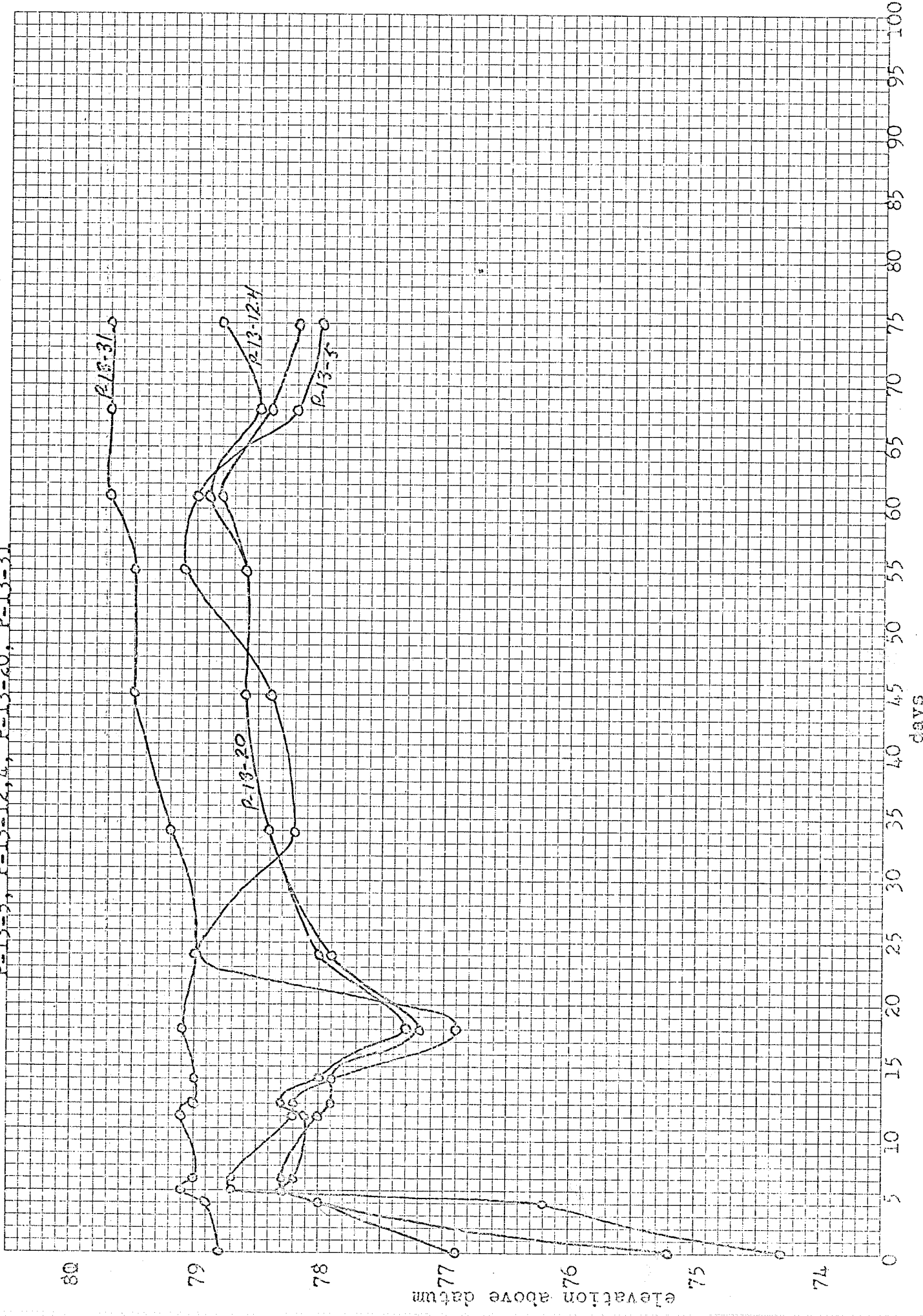


last point 13/8/68

P-12-5.5, P-12-11.5, P-12-18, P-12-28.6



P-13-5, P-13-12, 4, P-13-20, P-13-31



distance to water level from top of standpipe (feet)

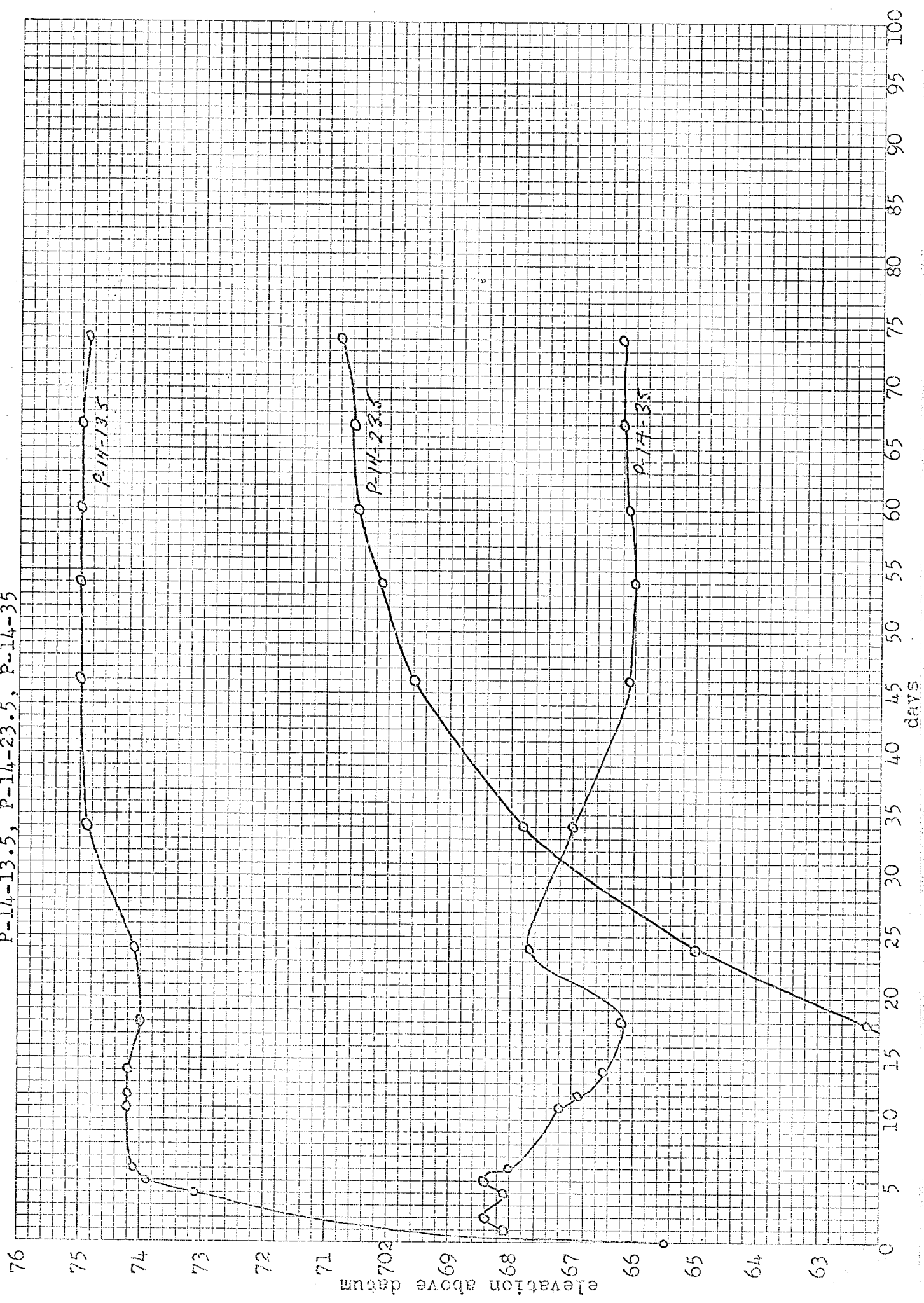


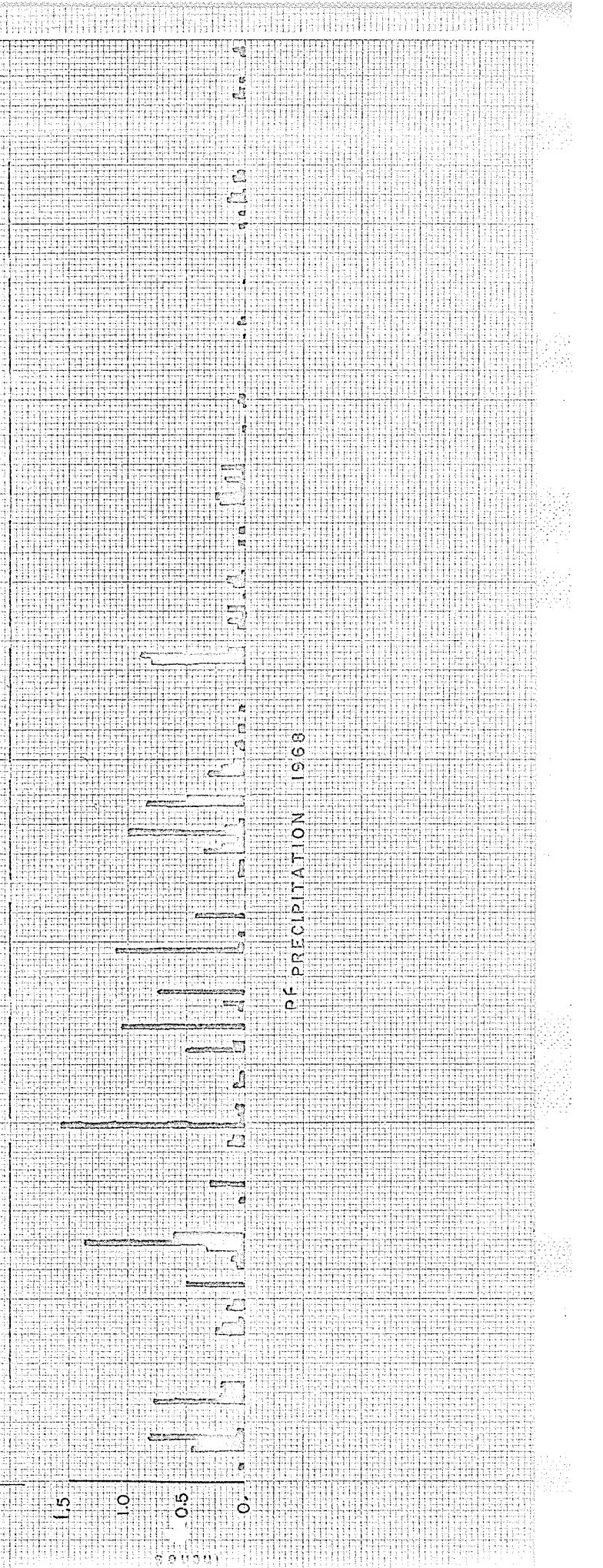
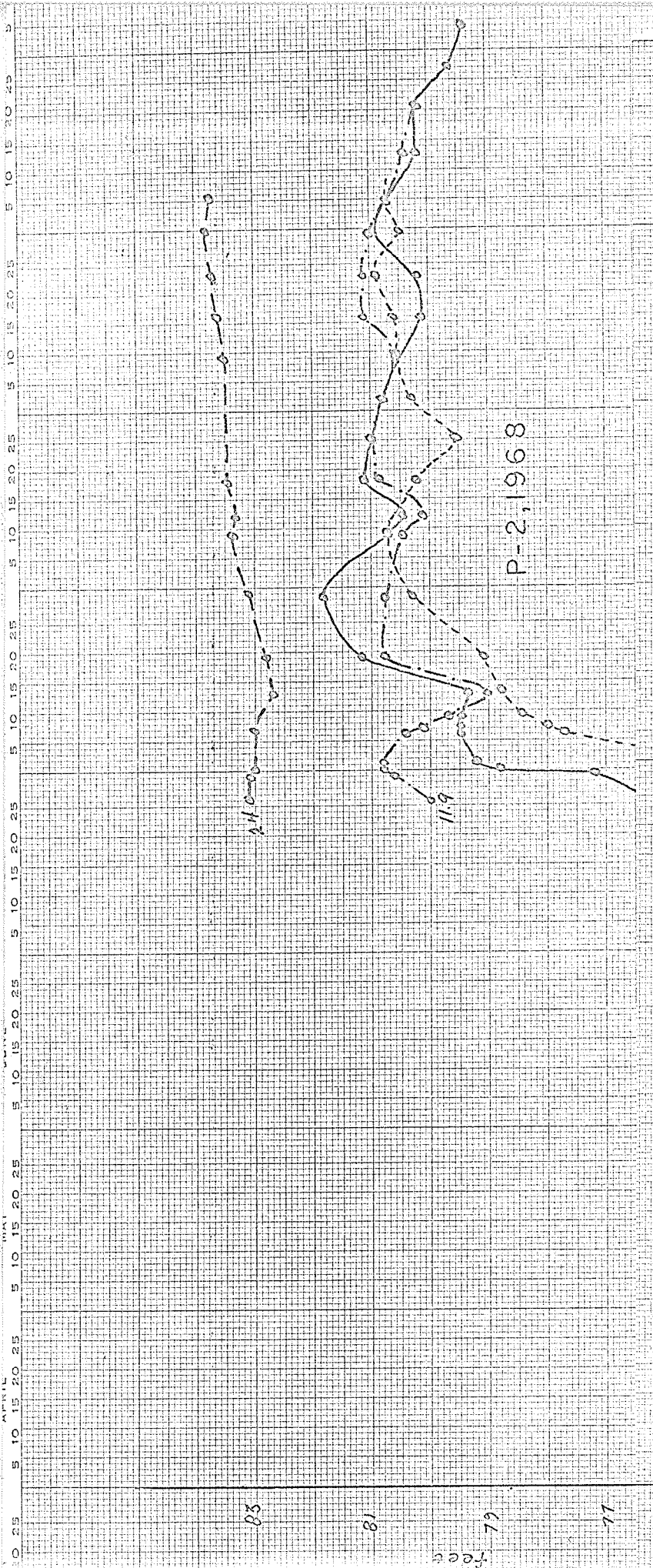
P-14-13.5, P-14-23.5, P-14-35 installed 25/7/68

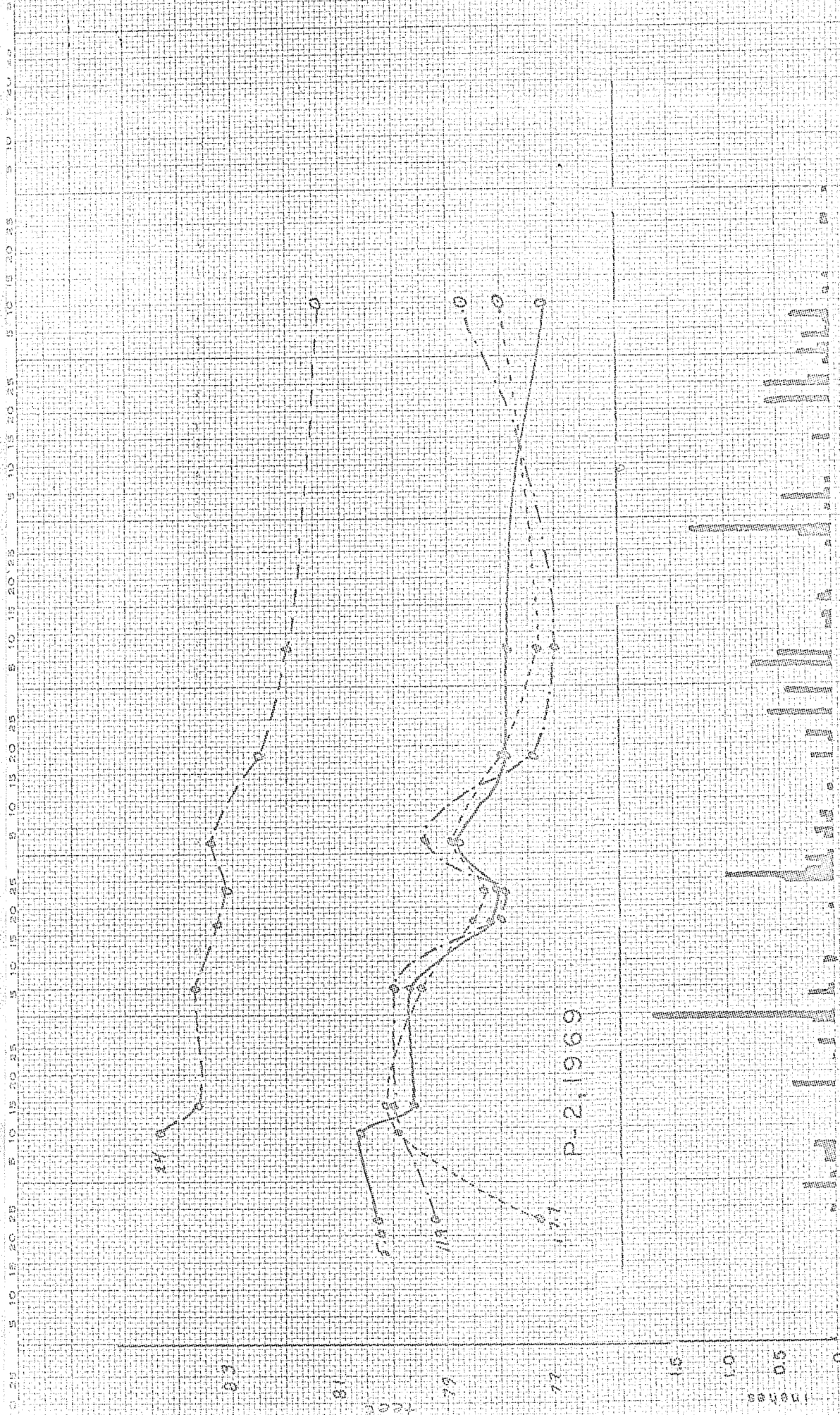
SHEET #1 to 14/8/68

last point 14/8/68

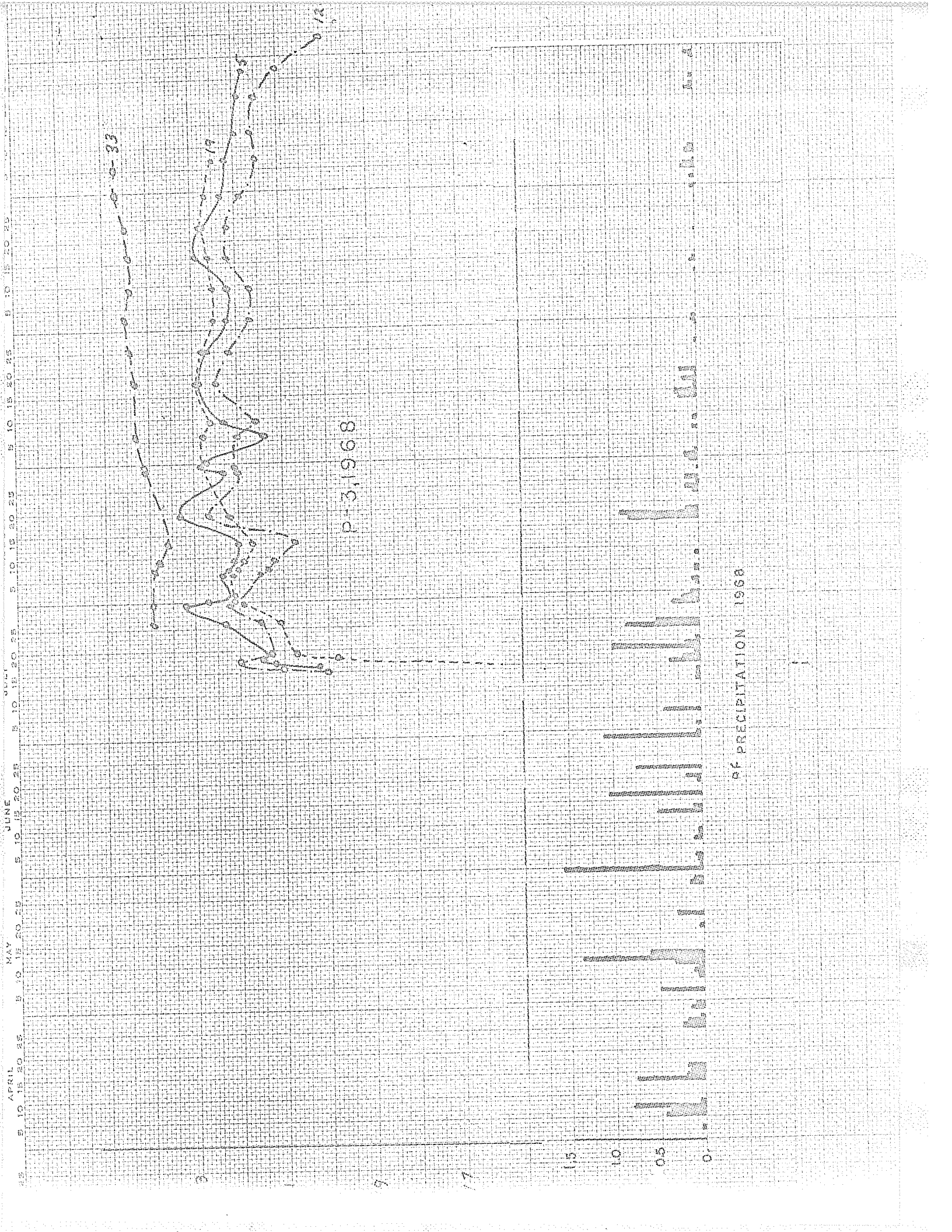
P-14-13.5, P-14-23.5, P-14-35

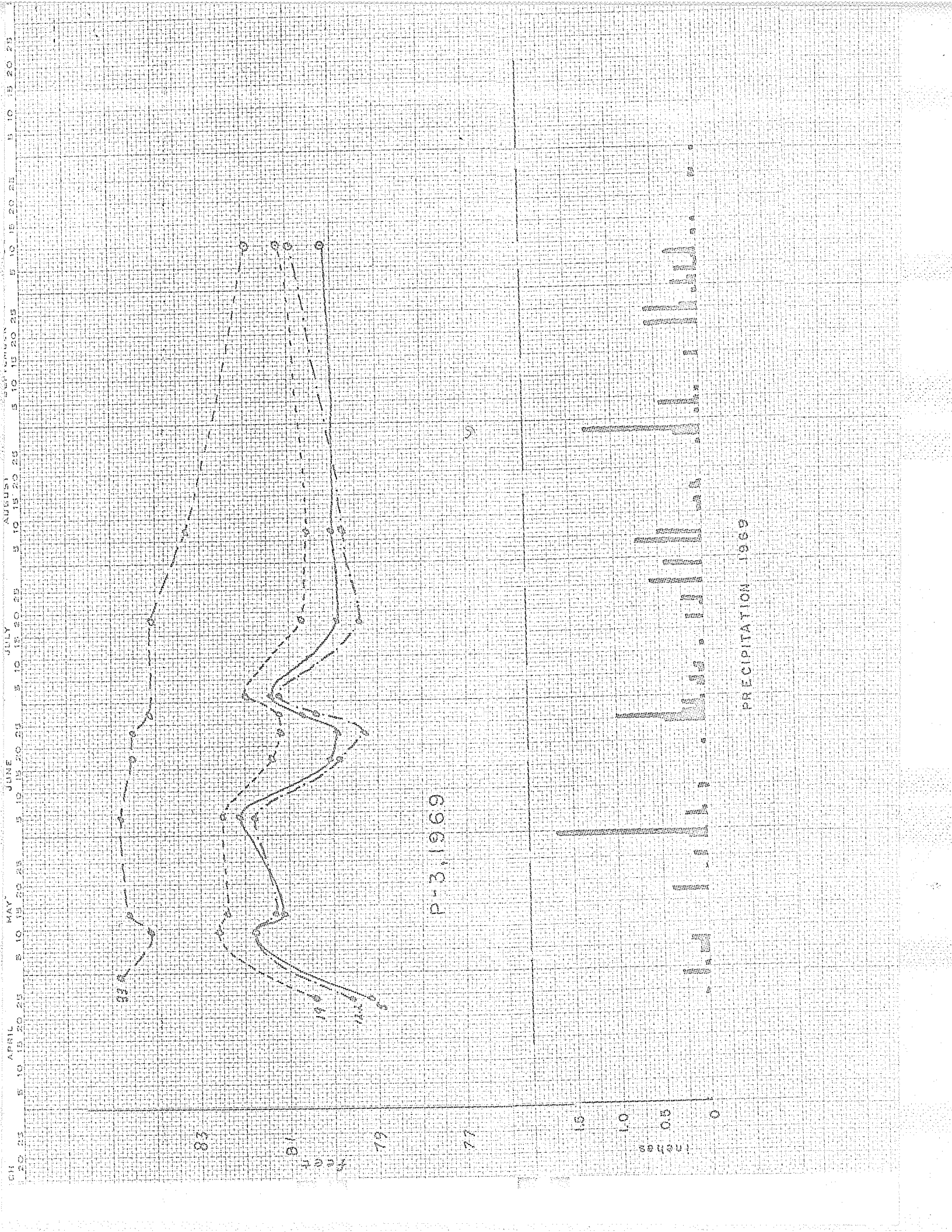


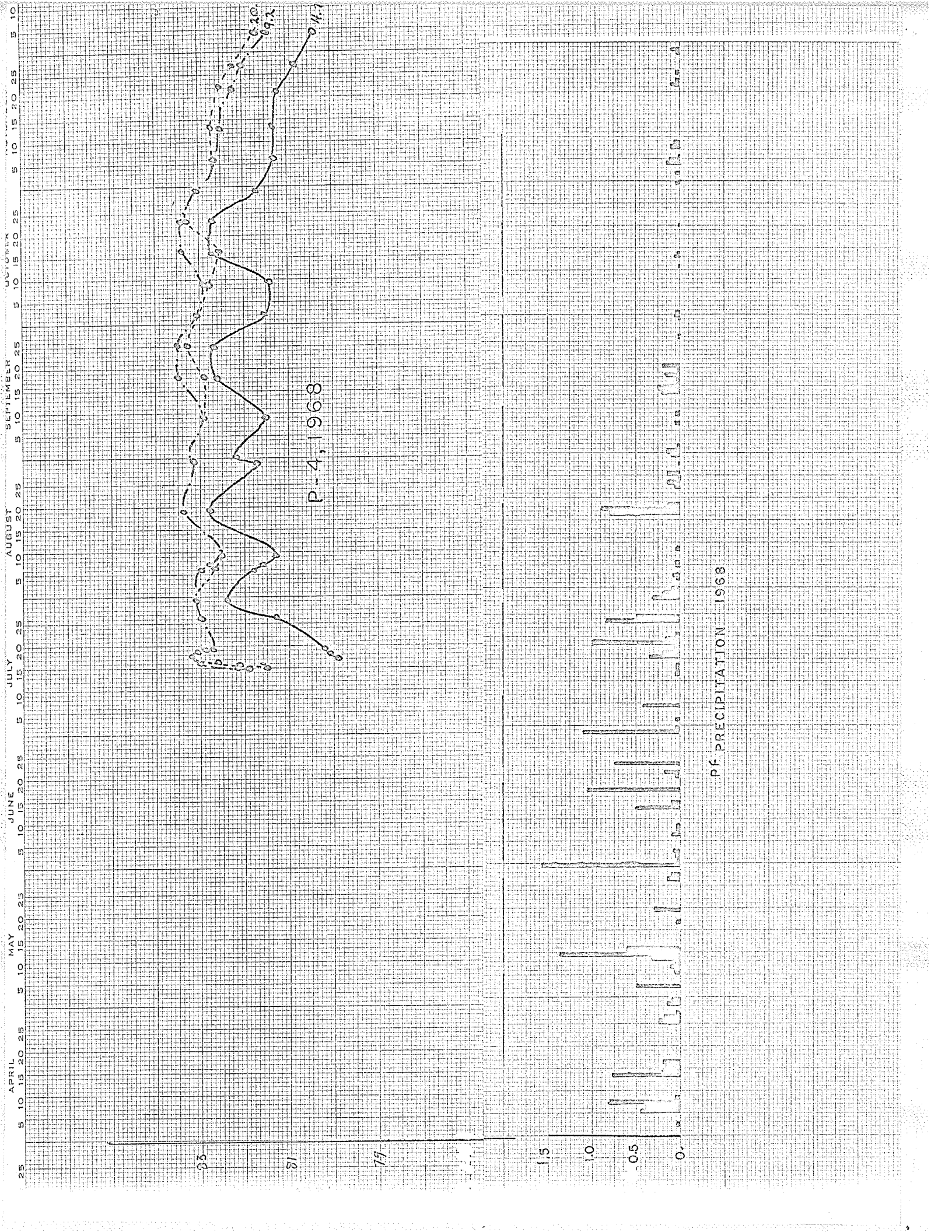


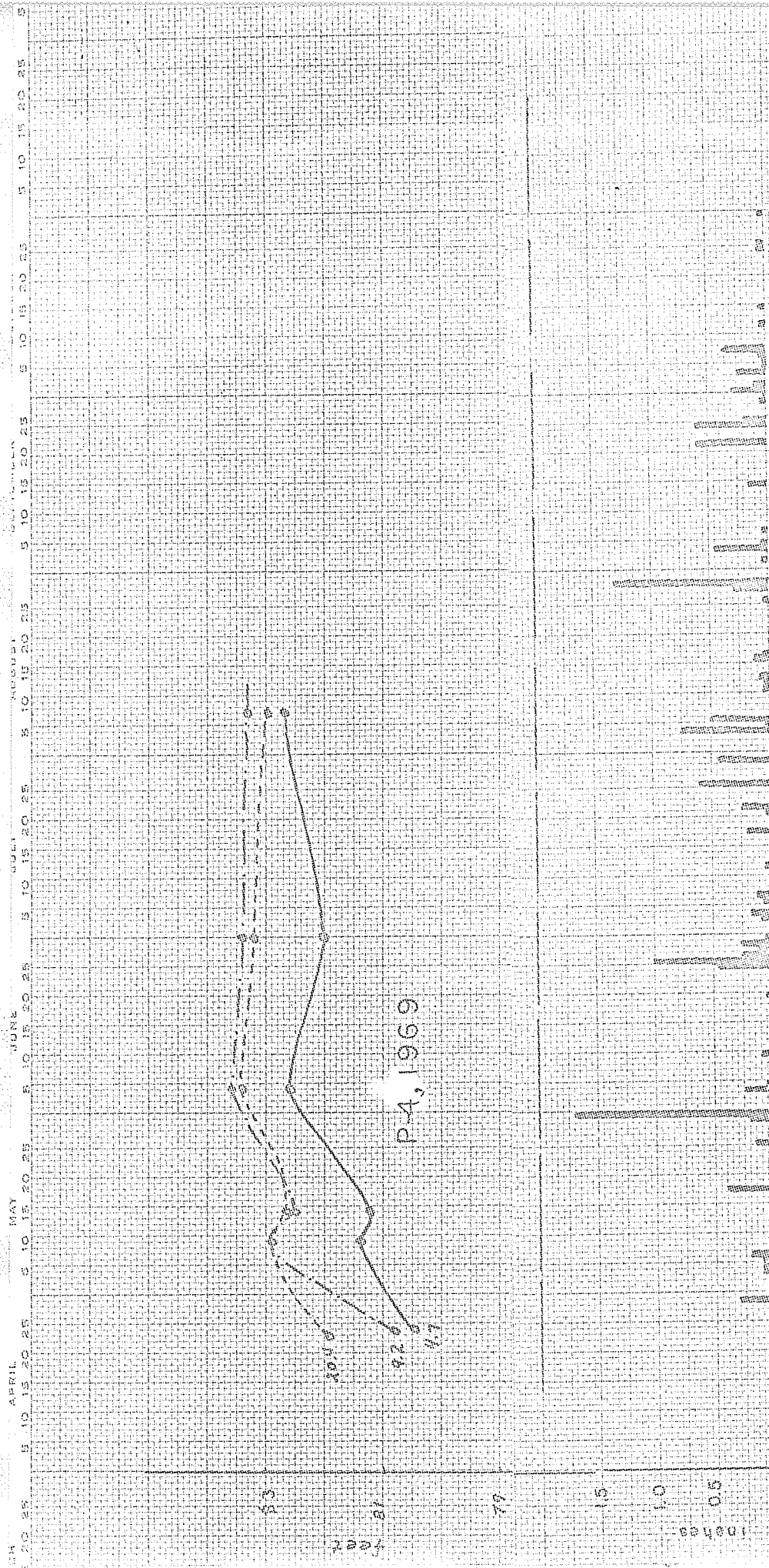


PRECIPITATION 1969



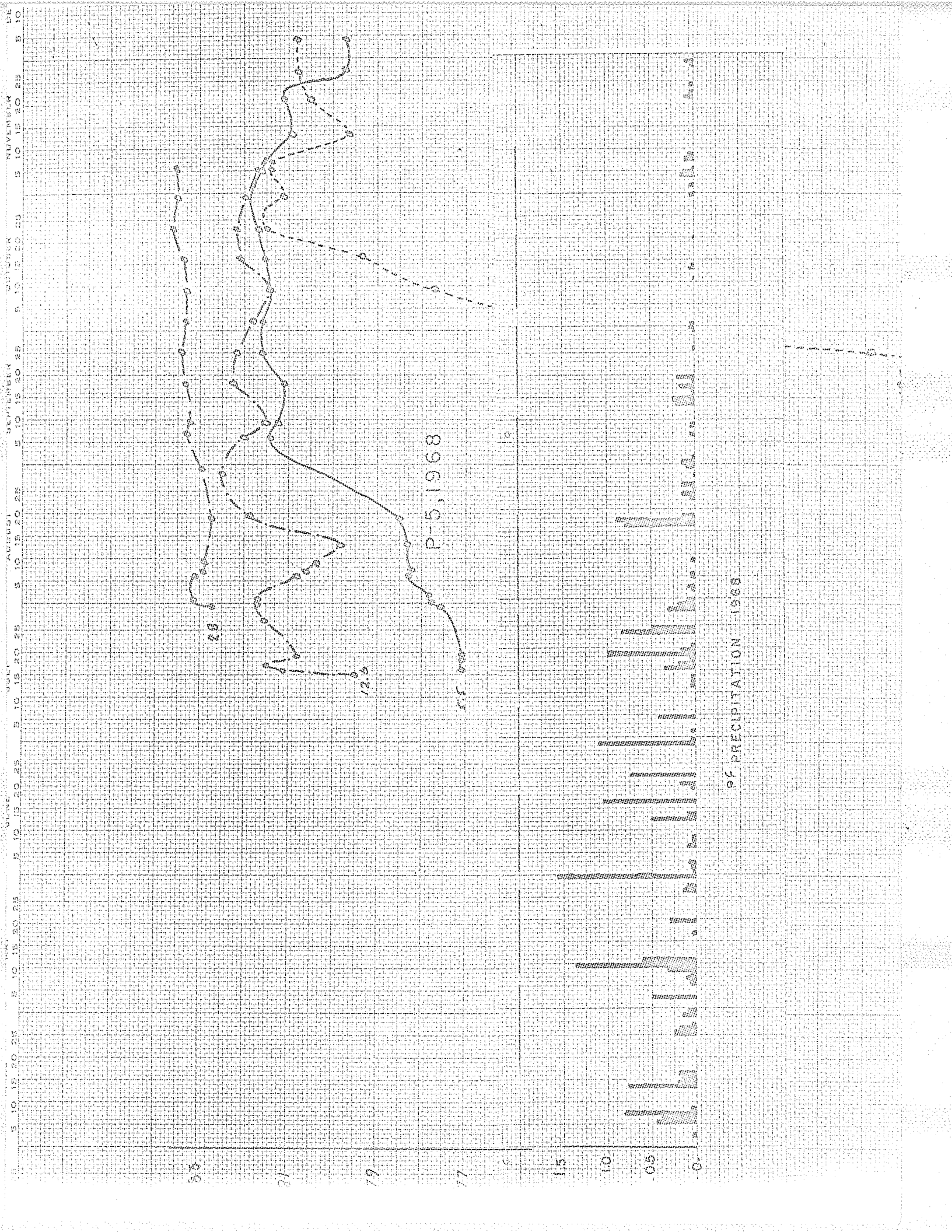


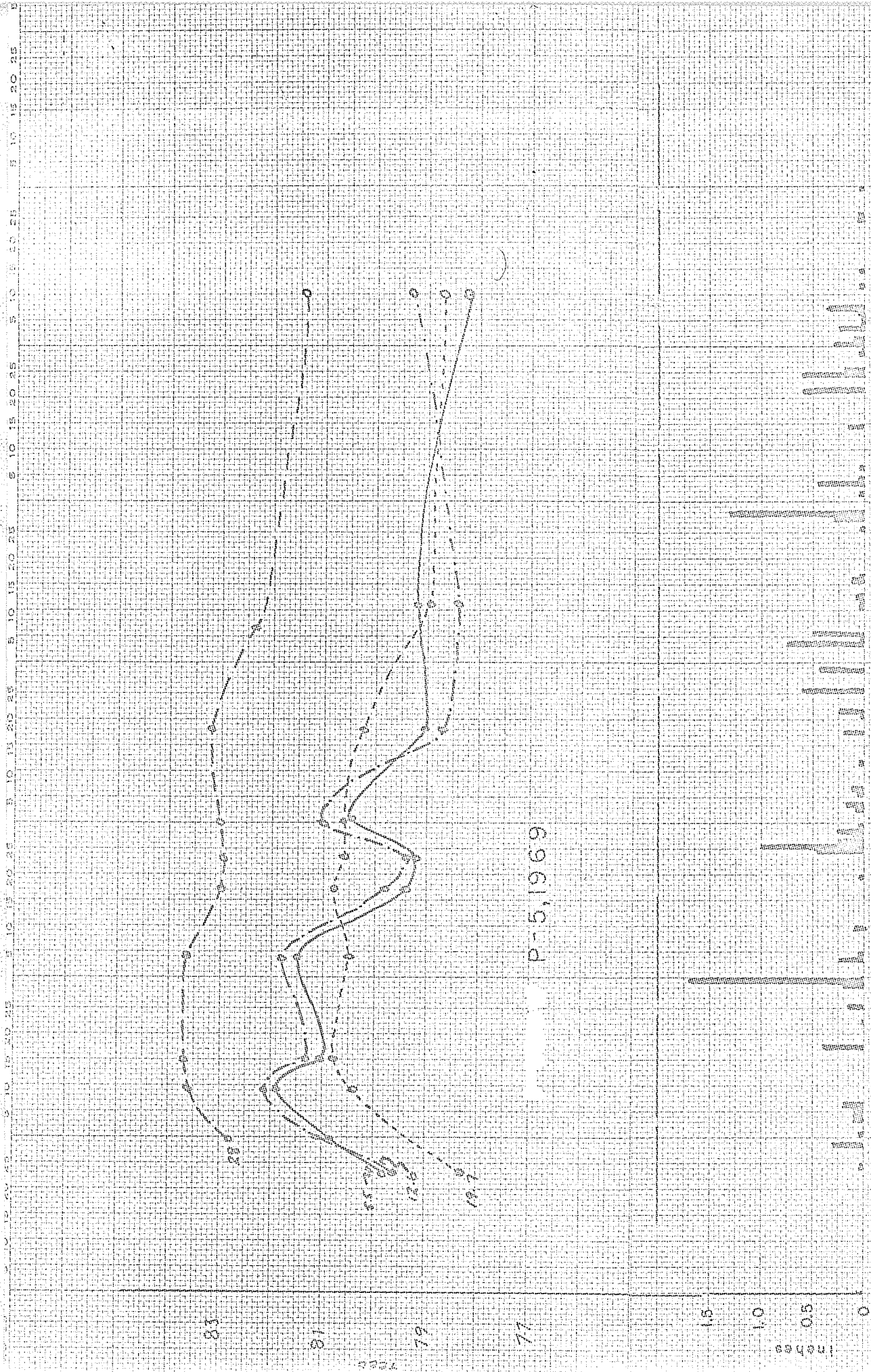




P-4, 1969

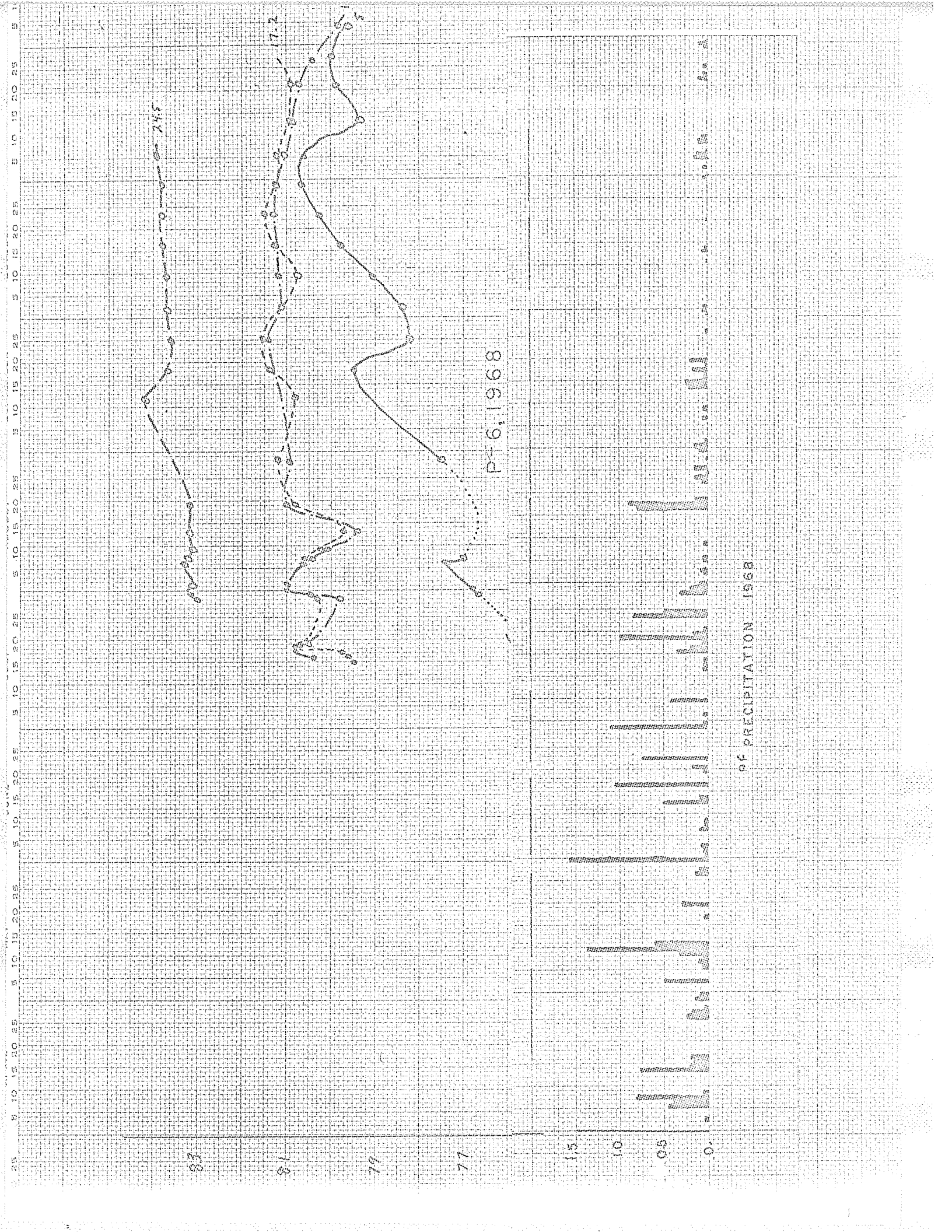
PRECIPITATION 1969

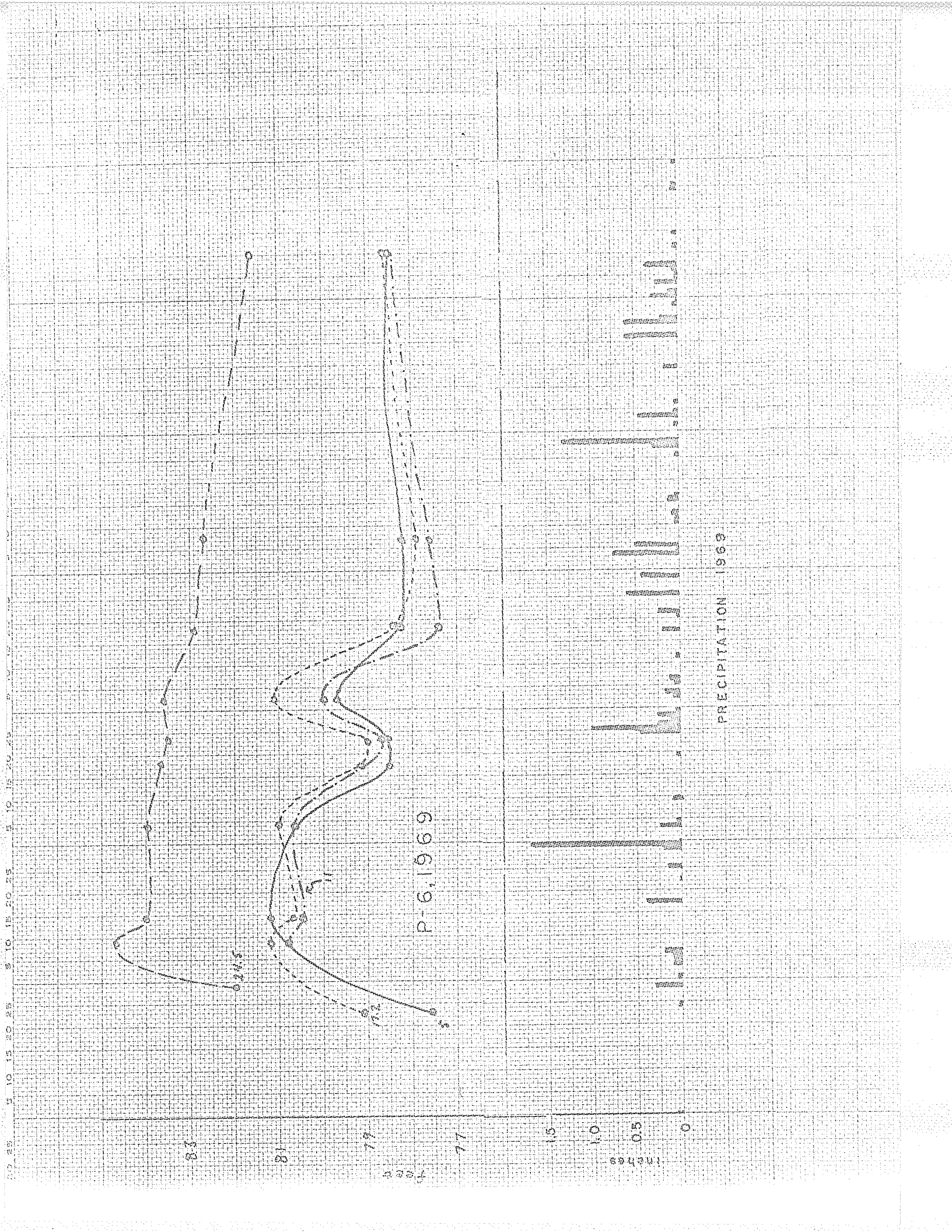


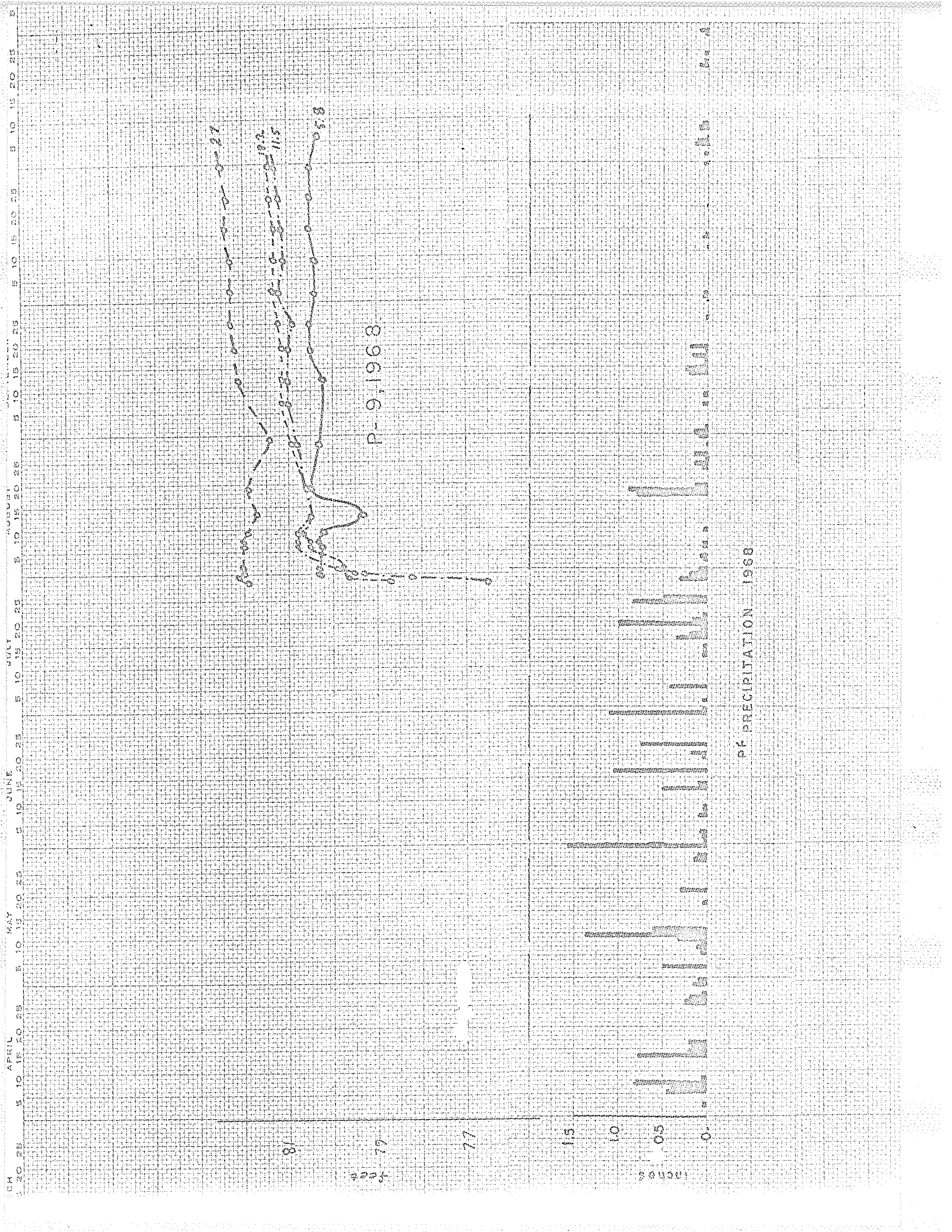


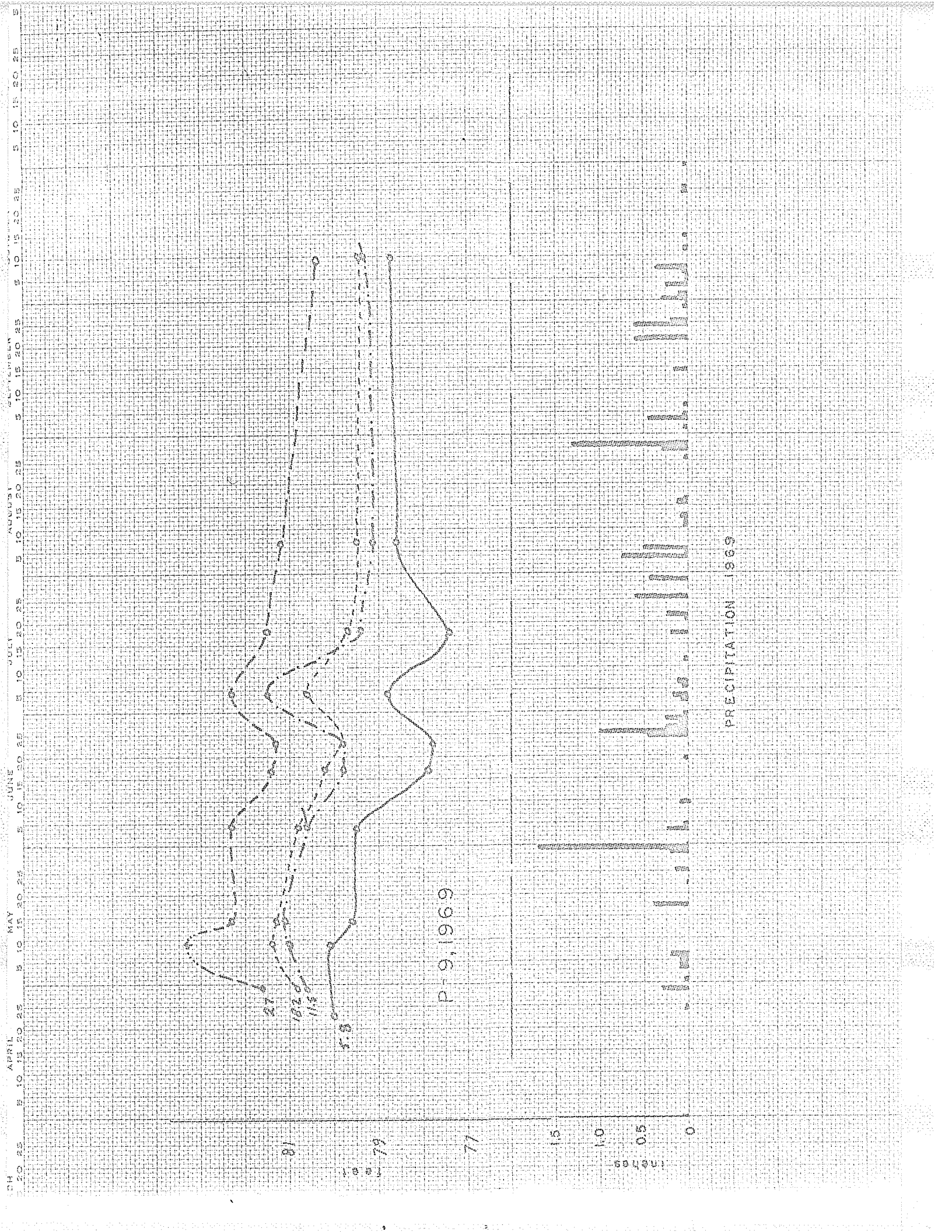
P-5, 1969

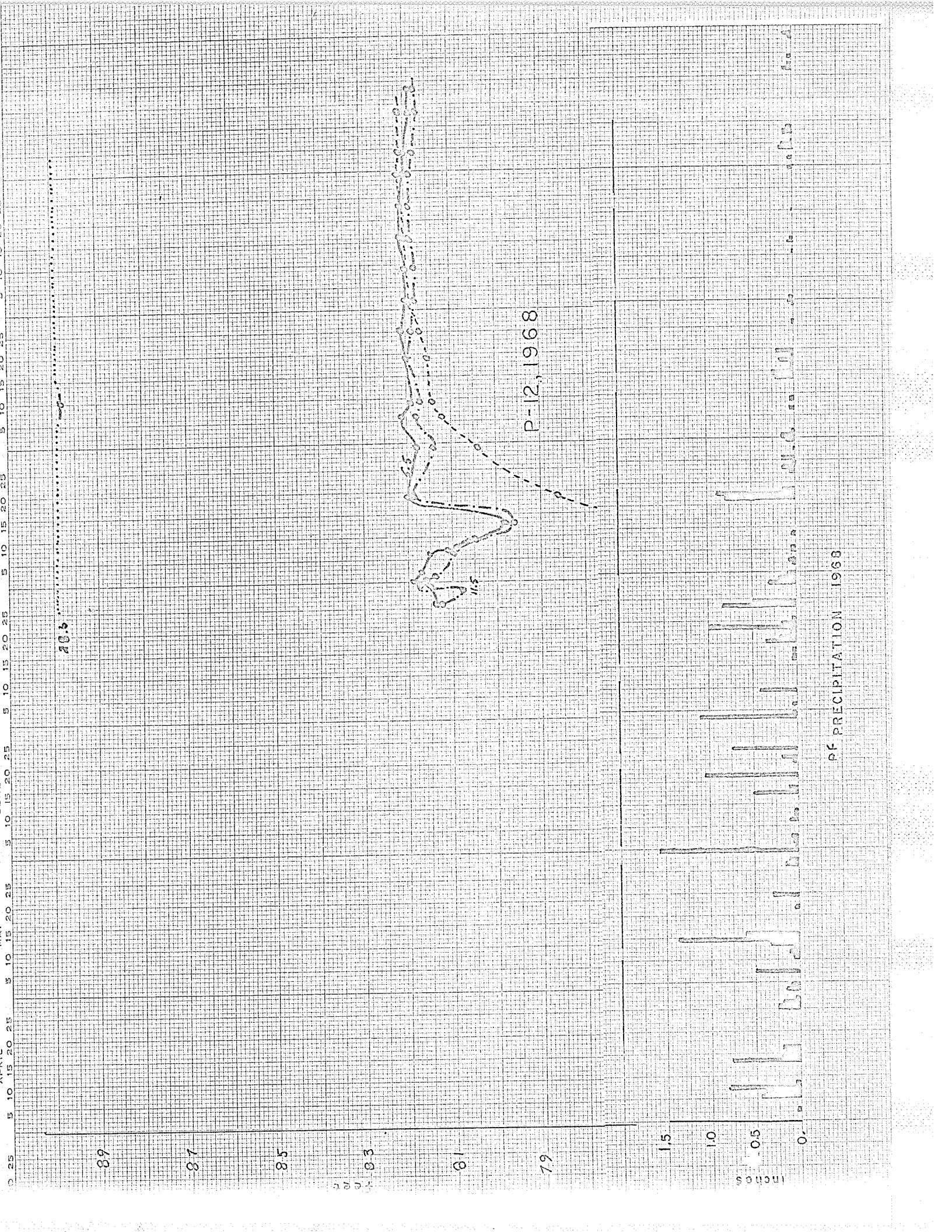
PRECIPITATION 1969











P-12, 1968

PRECIPITATION 1968

INCHES

89

87

85

83

81

79

1.5

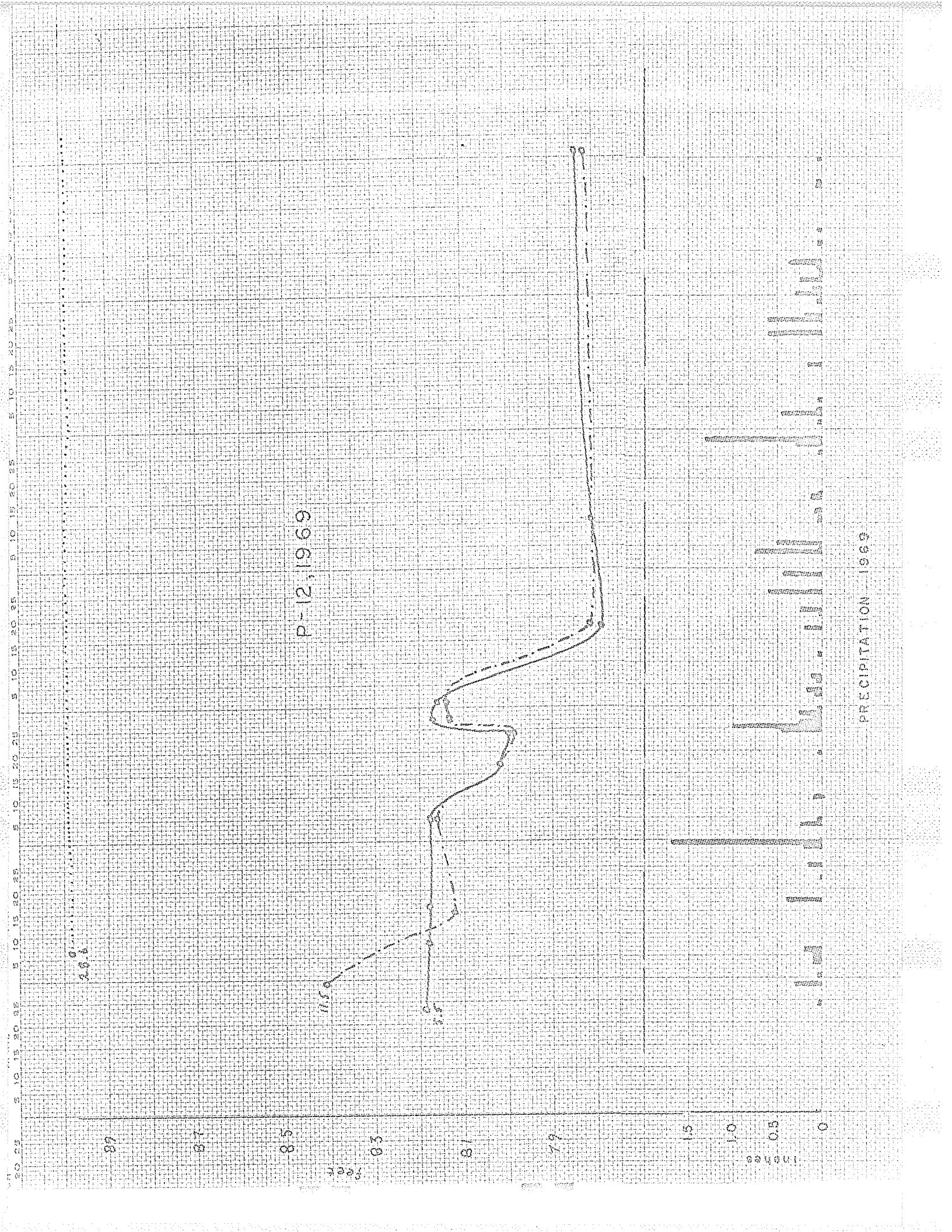
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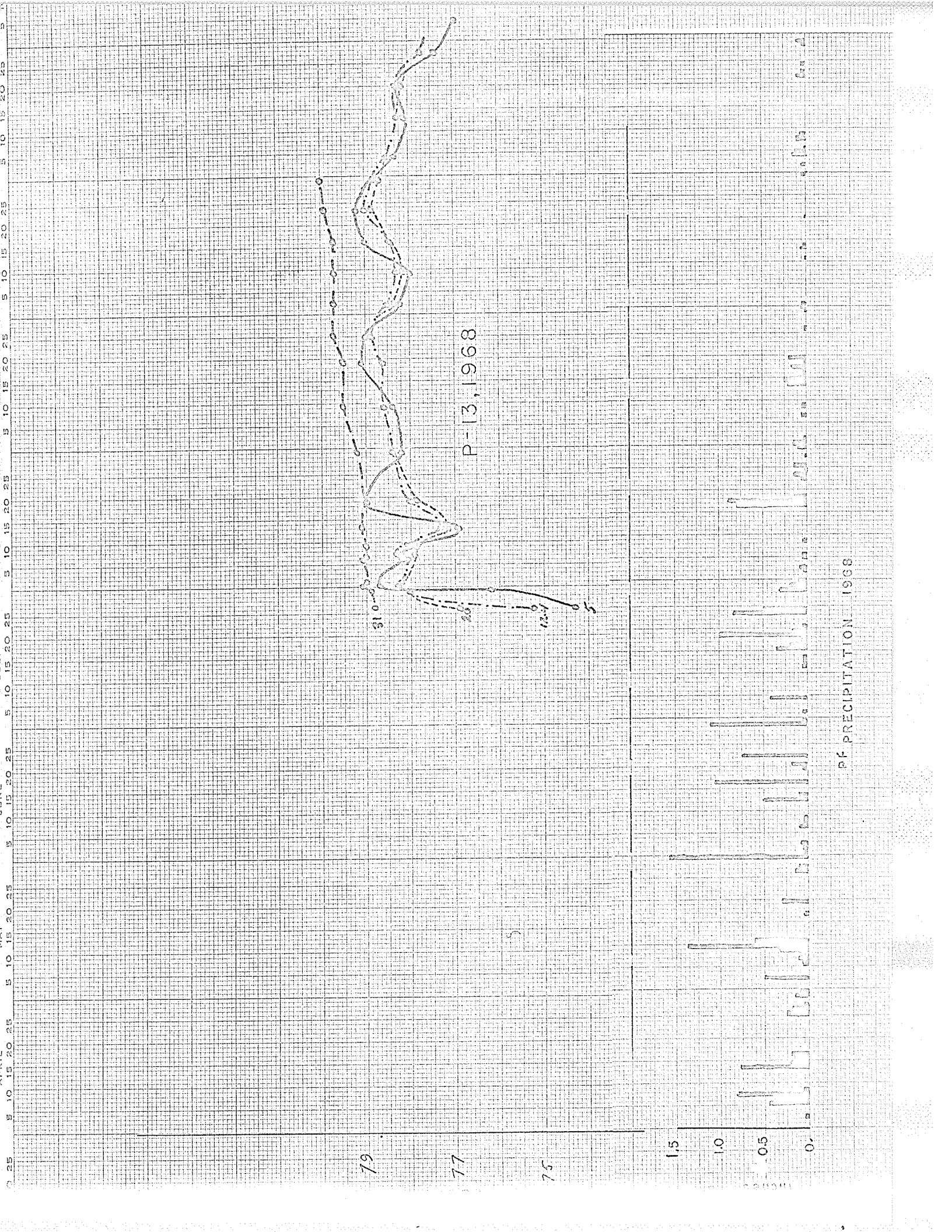
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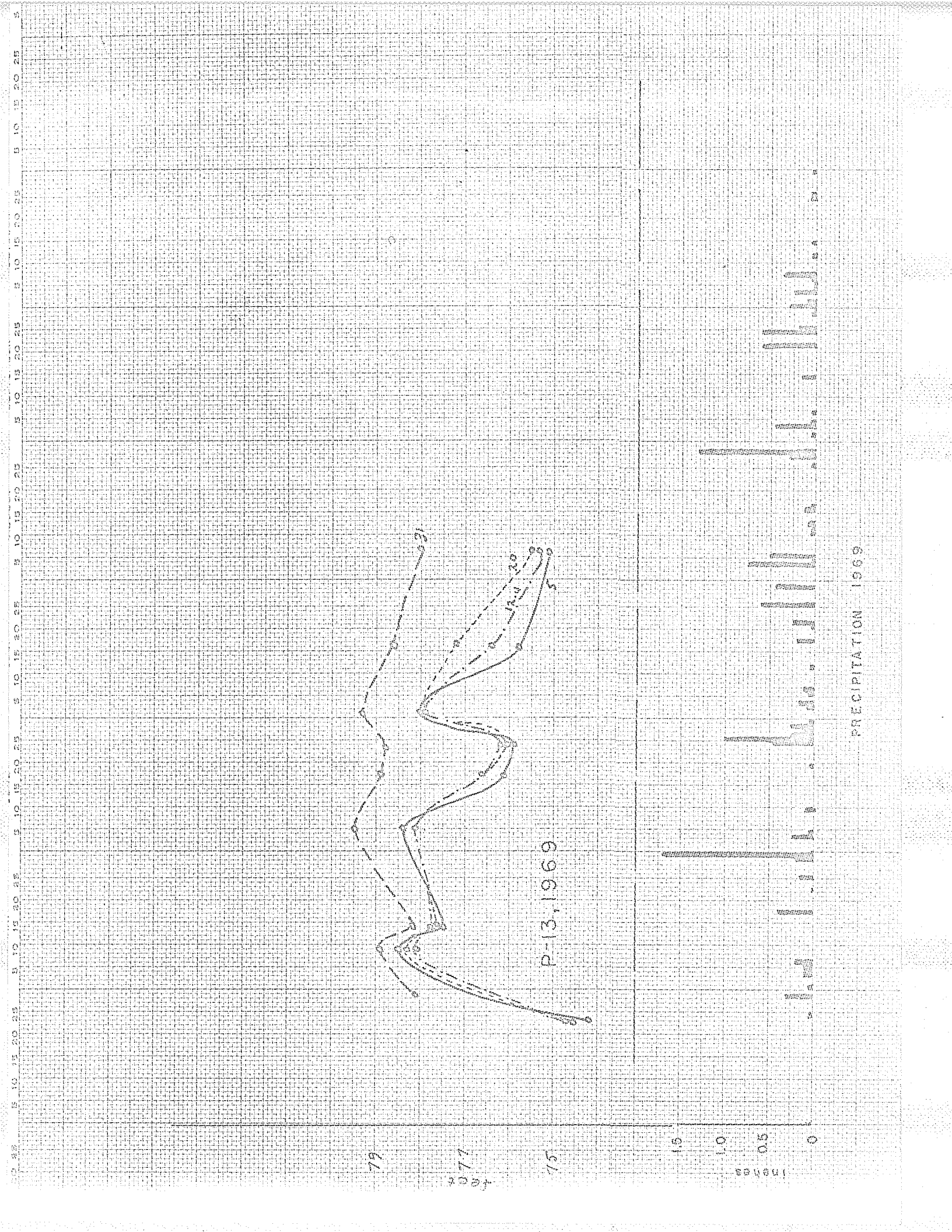
0

20.6

11.5







79

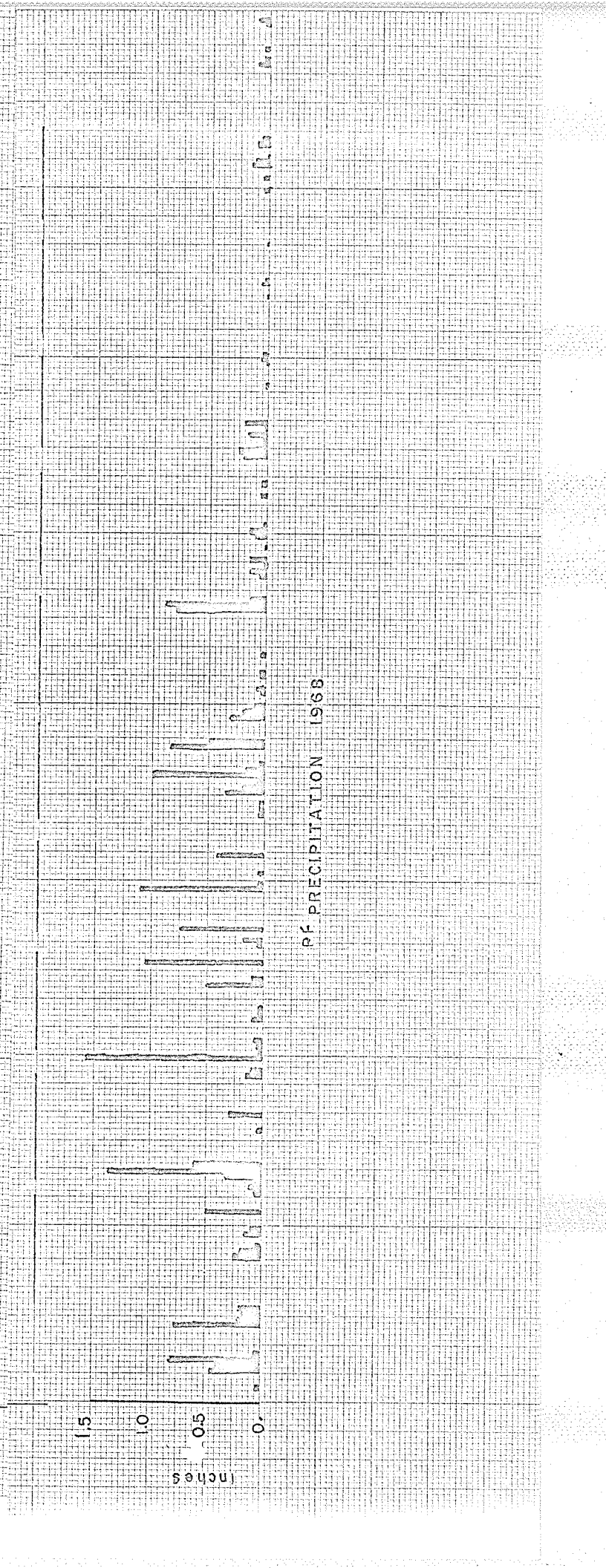
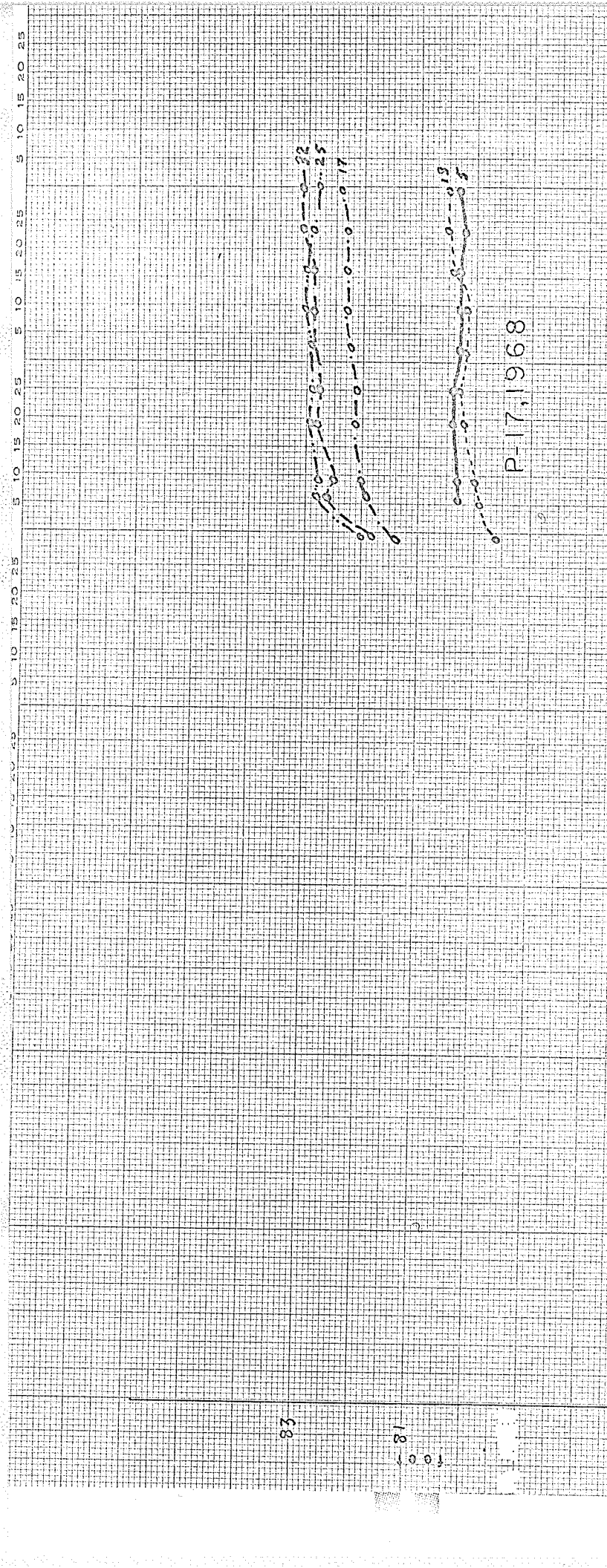
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125

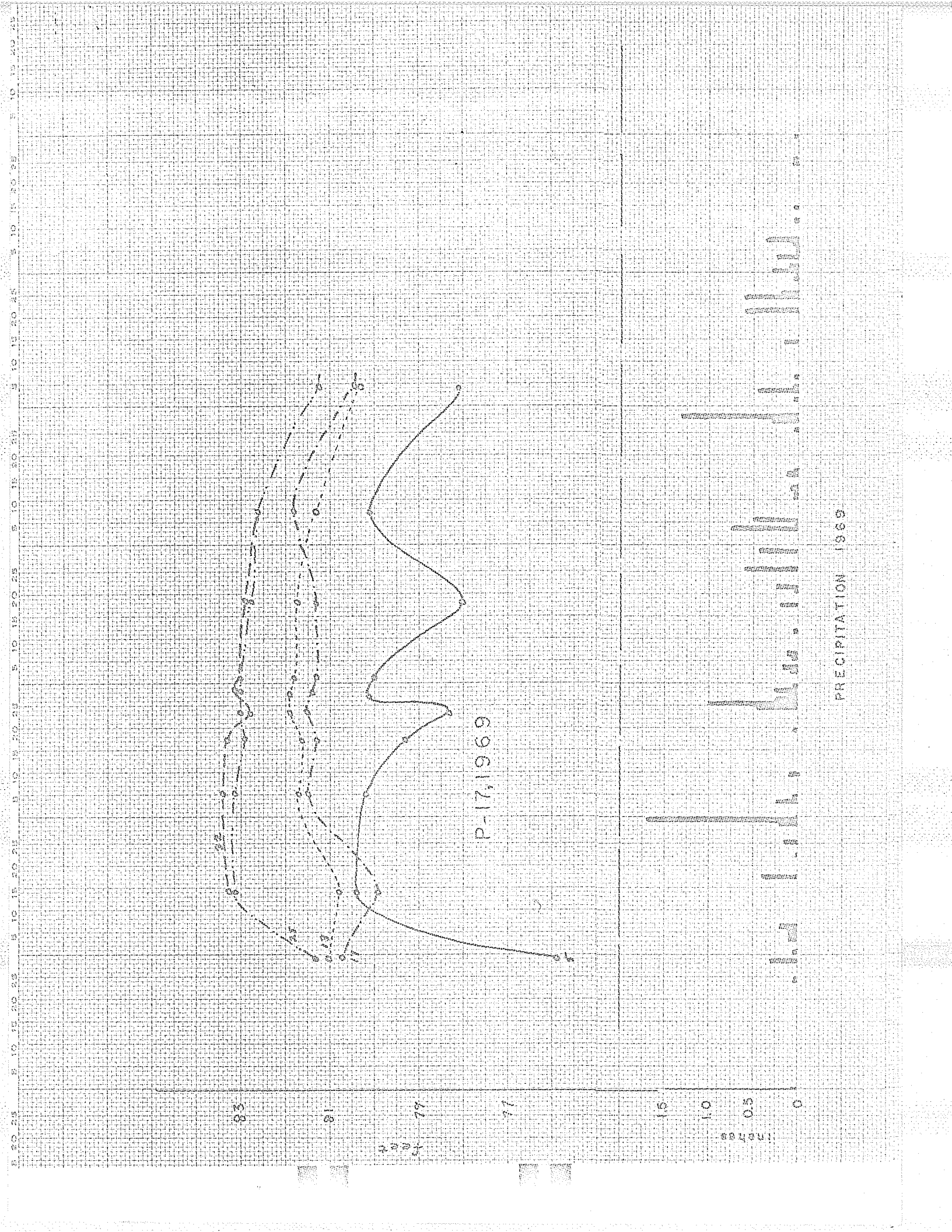
75

P-13, 1969

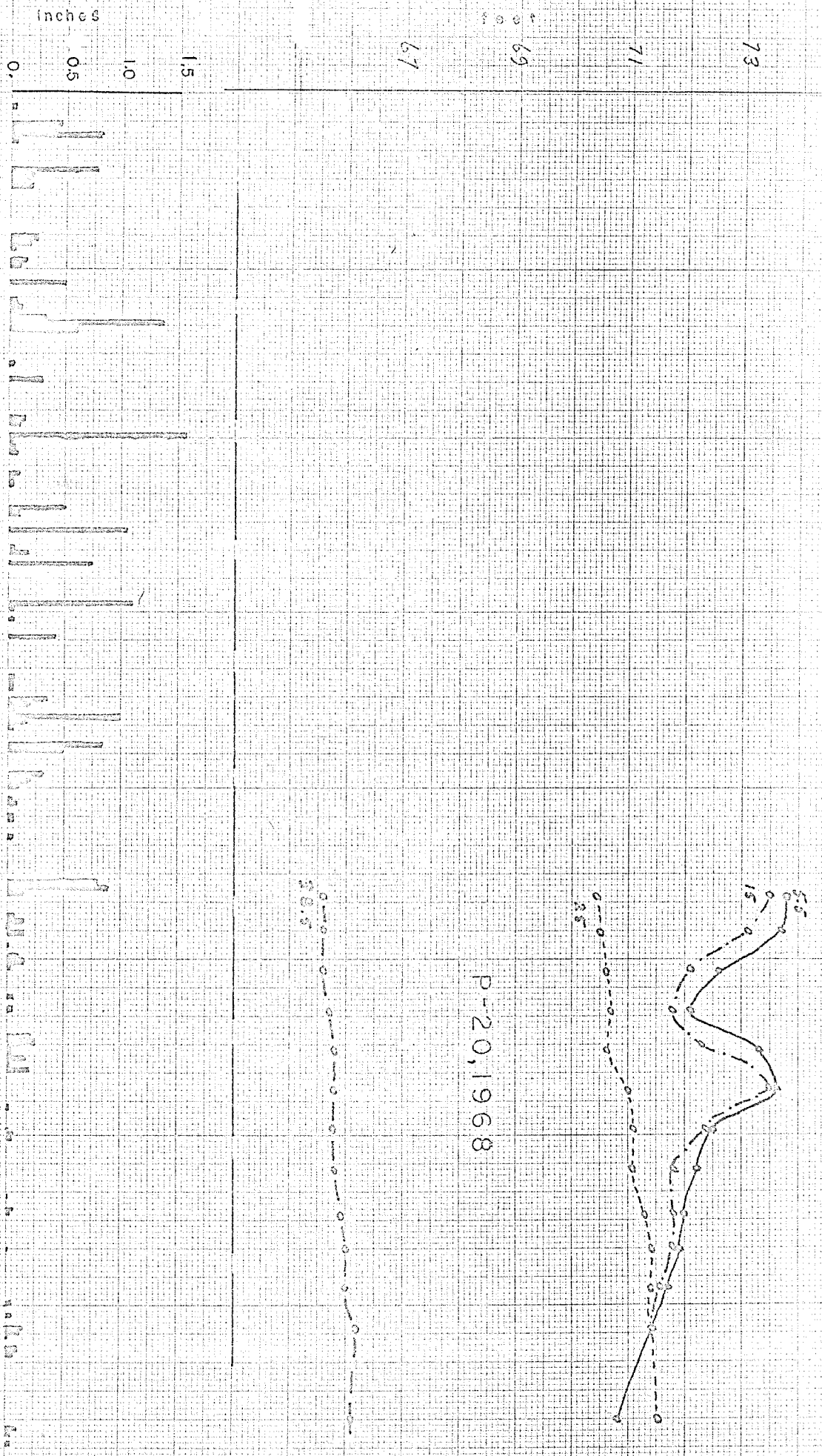
1.5
1.0
0.5
0

PRECIPITATION 1969





MAY 15 20 25
 JUNE 5 10 15 20 25
 JULY 5 10 15 20 25
 AUGUST 5 10 15 20 25
 SEPTEMBER 5 10 15 20 25
 OCTOBER 5 10 15 20 25
 NOVEMBER 5 10 15 20 25
 DECEMBER 5 10 15 20 25



Pf PRECIPITATION 1968



APPENDIX D

HYDROCHEMICAL ANALYSES

The hydrochemical analyses listed in Table 1 were conducted by the Whiteshell Nuclear Research Establishment, Research and Development Branch, in 1968 on samples obtained from relatively newly installed piezometers. The analyses listed in Table 2 were obtained in 1969 after piezometers had been stabilized, flushed, and drained, allowing true formation waters to be sampled with a minimum of contamination. Analyses for anions of the 1968 samples were conducted by the University of Manitoba, Department of Agriculture under the supervision of Dr. M. Zwarich, and the results are available although not listed in this appendix.

In Table 2, the samples listed under 'Special Samples', were isolated under low pressure from the atmosphere immediately after sampling.

Table 3 lists the field conductivities obtained by downhole methods during 1970.

<u>Sample Number</u>	<u>Na⁺</u> <u>mg/l</u>	<u>K⁺</u> <u>mg/l</u>	<u>Mg⁺⁺</u> <u>mg/l</u>	<u>Ca⁺⁺</u> <u>mg/l</u>
P- 1- 5	291.0	3.7	850.0	226.0
P- 1-11	109.5	6.05	182.5	110.0
P- 1-17	61.0	5.7	100.5	96.0
P- 1-24	31.5	6.3	50.0	51.7
P- 2- 5	205.0	5.25	530.0	268.0
P- 2-11	144.0	8.4	345.0	220.0
P- 2-17	149.5	12.3	215.0	210.0
P- 2-24	23.5	5.8	55.0	79.0
P- 6- 5	118.5	7.0	650.0	492.0
P- 6-11	71.0	16.65	210.5	197.0
P- 6-17	65.0	48.0	70.0	128.5
P- 6-24	22.5	5.9	79.0	92.0
P- 8- 5	223.5	5.5	650.0	354.0
P- 8-12	138.5	5.5	130.0	122.0
P- 8-19	127.0	5.25	66.0	78.0
P- 8-28	18.0	7.3	35.0	31.0
P- 9- 5	75.0	5.8	366.0	304.0
P- 9-11	34.0	5.85	25.0	62.5
P- 9-18	75.0	15.6	49.0	92.0
P- 9-27	44.0	5.85	149.0	139.0
P-21-12	271.0	595.0		726.0
P-21-22	181.5	817.5		137.0
P-21-28	71.0	162.5		850.0
P-21-39	14.5	9.35	24.0	17.4
P-23-19	109.5	117.0	155.0	139.0
P-23-28	21.0	10.4	46.5	45.5
P-23 ₁ -32	22.0	8.85	48.5	49.0
P-23 ₂ -32	20.0	9.0	48.5	62.5

Table 1. Analyses by REEL, 1968.

Sample Number	Na ⁺ mg/l	K ⁺ mg/l	Mg ⁺⁺ mg/l	Ca ⁺⁺ mg/l	SO ₄ ⁼ mg/l	CO ₃ ⁼ mg/l	HCO ₃ ⁻ mg/l	Cl ⁻ mg/l	TDS mg/l	Lab Conductivity (umhos/cm)
P-1-5.5	330	4	670	180	2800		844.2	3.0	4881	
-11.4	75	4	160	70	485		473.4	6.0	1273	
-17.7	55	5	85	85	65		502.6	6.5	804	
-24	10	5	90	65	1180			2.0	1352+	
P-2-5.6	160	5	300	135	1490		392.8	3.5	2487	
-11.9	160	8	280	180	1440		466	5.0	2539	
-17.7	130	9	185	185	50		361	7.5	928	
-24	15	5	50	40	225		373	3.5	712	
P-3-5	165	5	535	235	2830		241.6	4.0	4016	
-12.2	135	7	145	135	840		307	5.0	1574	
-19	20	7	45	40	65		334	8.0	519	
-33	5	8	15	55	40		210	3.0	331	
P-4-4.7	130	6	370	270	2180		359	4.0	3319	
-9.2	135	9	320	335	2250		268	4.5	3322	
-20.4	125	7	140	105	610			5.0	610+	
P-5-5.5	105	7	305	265	1820		366	3.5	2872	
-12.6	65	8	165	105	770		405	5.5	1524	
-19.7	30	9	60	70	185		368	7.5	730	
-28	5	-	35	70	70		364	3.5	543	
P-6-5	160	25	520	235	2800		293	3.0	4036	
-11	175	9	180	40	950		346	4.5	1705	
-17.2	35	27	70	100	30		425	5.0	692	
-24.5	20	7	45	68	50		427	3.5	621	
P-7-5.3	105	11	455	385	2540		381	6.0	3883	
-11.3	165	8	440	265	1960		401	8.0	3247	
-18.5	155	15	215	195	1260			10.0	1850+	
-30	235	25	25	60	30		447	9.0	831	
P-8-5.9	240	5	40	370	3650		439	4.0	4748	
-12	105	5	165	50	685		632	5.5	1648	
-19	30	4	625	140	80		339	5.0	1223	
-28	5	7	35	90	110		295	2.0	539	

Table 2. Analyses by AECI, 1969

Sample Number	Na ⁺ mg/l	K ⁺ mg/l	Mg ⁺⁺ mg/l	Ca ⁺⁺ mg/l	SO ₄ ⁼ mg/l	CO ₃ ⁼ mg/l	HCO ₃ ⁻ mg/l	Cl ⁻ mg/l	TDS mg/l	Lab Conductivity (umhos/cm)
P- 9-5.8	70	4	335	390	2130		322	4.0	3255	
-11.5	20	6	40	70	25		383	6.5	551	
-18.2	55	14	40	95	215		332	9.0	760	
-27	15	4	50	9	135		342	4.5	560	
P-10-5	210	2	190	80	745		639	2.5	1869	
-11	265	9	600	375	3390		564	7.5	5211	
-18	165	10	330	290	1850		625	5.5	3276	
-34	115	-	25	10	250		405	3.5	809	
P11-5.5	45	2	150	75	530		444	2.5	1249	
-13.3	210	13	435	320	2530		683	11.0	4202	
-22.8	375	19	140	145	1300		396	7.0	2376	
-35	165	14	200	215	1330		298	9.5	2231	
P-12-5.5	75	3	255	190	1630		229	1.0	2383	
-11.7	5	6	30	70	150		107	-	363	
P-13-5	435	5	1110	205	4220		561	14.0	6550	
-12.4	230	15	400	340	2490		307	11.0	3793	
-20.0	200	10	30	225	1730		532	5.0	2732	
-31	20	5	220	65	20		327	2.5	660	
P-14-13.5	195	11	290	240	1210		805	16.0	2767	
-23.5	220	16	260	270	1640		566	19.0	2991	
-35	220	15	200	75	1380		503	19.0	2412	
P-15-4	10	6	40	145	30		493	3.0	727	
P-16-6	45	6	170	145	860		337	4.0	1567	
-10	105	9	235	310	1770		244	5.0	2678	
-14.5	30	7	165	155	955		229	5.0	1546	
-20	15	7	50	70	55		415	2.5	615	
-34	55	9	40	80	100		327	3.0	564	
P-17-5	140	3	275	15	710		712	4.5	1860	
-13	15	4	50	50	40		361	7.0	527	
-17	15	5	45	55	30		346	7.0	507	
-25	20	10	50	60	40		425	5.5	602	
-32	5	7	40	60	10		366	3.0	481	

Table 2, continued

Sample Number	Na ⁺ mg/l	K ⁺ mg/l	Mg ⁺⁺ mg/l	Ca ⁺⁺ mg/l	SO ₄ ⁼ mg/l	CO ₃ ⁼ mg/l	HCO ₃ ⁻ mg/l	Cl ⁻ mg/l	TDS mg/l	Lab Conductivity (umhos/cm)
P-18-5	10	6	150	125	195		854	9.5	1349	
-17	145	-	125	170	705		630	22.0	1797	
-32	230	-	140	165	1190		444	22.0	2191	
-39.5	190	36	230	270	1680		464	34.0	2904	
-47	95	16	150	165	705		512	13.0	1656	
P-19-12	100	35	160	115	560		732	12.0	1714	
-22	190	225	125	160	1010		512	15.0	2237	
-32	210	130	115	155	1190		205	18.0	2023	
-44	95	120	65	35	265		454	6.0	1040	
P-20-5	475	7	850	35	3650		693	5.5	5716	
-15	430	60	560	285	3690		522	20.0	5567	
-25	400	17	600	350	3560		590	32.0	5549	
-38.5	270	16	370	310	2230		454	37.0	3667	
P-22-5	5	2	50	55	5		376	2.5	491	
-8	10	4	55	60	40		376	4.5	550	
-16	10	5	25	65	10		273	3.0	391	
P-23-17	70	11	115	95	540		307	7.0	1145	
-28	20	7	40	70	60		342	6.0	545	
-32.5	25	5	40	65	115		288	5.0	543	
<u>Special Samples</u>										
P- 7-30	170	10	15	30	110		429	7.0	771	770
P-11-22.8	215	15	110	150	930		351	7.0	1778	1850
P-11-34	95	15	125	140	760		332	5.0	1472	1500
P-12-23.6	5	11	30	95	5		434	1.0	571	540
P-13-12.9	250	3	450	325	2700		459	5.0	4192	3810
P-13-31	15	10	30	65	30		337	2.0	489	470
P-14-13.5	240	14	210	190	1210		581	17.0	2462	2400
P-14-23.5	205	14	270	340	1790		537	17.0	3173	2990
P-14-35	265	18	210	320	1800		420	18.0	3051	2950

Table 2, continued

<u>Sample Number</u>	<u>Na⁺ mg/l</u>	<u>K⁺ mg/l</u>	<u>Mg⁺⁺ mg/l</u>	<u>Ca⁺⁺ mg/l</u>	<u>SO₄⁼ mg/l</u>	<u>CO₃⁼ mg/l</u>	<u>HCO₃⁻ mg/l</u>	<u>Cl⁻ mg/l</u>	<u>TDS mg/l</u>	<u>Lab Conductivity (umhos/cm)</u>
P-16-14	35	7	165	350	905		356	4.0	1822	1640
P-16-20	5	5	50	95	40		468	3.0	661	670
P-16-34	5	6	50	100	80		473	3.5	713	690
P-20-15	350	17	65	335	3670		488	27.0	4952	5100
P-21-22	60	23	15	90	310		132	6.5	637	740
P-21-28	10	3	25	70	5		337	1.0	446	460
P-21-39	5	3	25	70	5		342	1.5	447	460
P-22-16	15	17	35	90	15		449	6.0	627	640
P-26-28	365	355	245	290	2340		259	35.0	3889	4100
P-26-43	180	14	200	270	2320		249	17.0	2320	2530
P-27-58	115	80	4	520	1250		464	18.0	2451	3200
P-28-18	175	34	380	390	2290		781	12.0	4062	3650
P-28-40	225	275	4	200	1080		181	20.0	1985	2250
P-28-71	110	-	70	135	820		166	37.0	1338	1630
P-29-18	130	10	300	495	2140		429	8.0	3512	3340
P-29-35	140	14	275	475	2050		356	9.0	3319	3000
P-29-54	135	13	245	490	1980		342	7.5	3213	3000
P-31-10	25	3	90	140	200		468	5.0	931	3350
P-31-25	145	35	145	345	1290		400	18.0	2378	2250
P-31-44	255	36	190	285	1400		454	13.0	2633	2800
IWP-3-20	435	15	360	530	2590		634	21.0	4585	4000
IWP-3-(11-15)	380	18	505	455	3590		439	29.0	5416	5000
IWP-3-(15-20)	350	14	470	500	3270		698	29.0	5331	4700
IWP-3-15	245	12	345	550	2600		615	16.0	4383	3900
P-33-18	50	4	200	105	1644		976	20.0	1644	1600
P-33-35	305	26	80	300	1600		97.6	37.0	2446	2800
P-33-48	305	39	250	370	1990		288	44.0	3286	3200
IWP-2-(20-36)	320	15	495	410	3190		547	29.0	5006	4500
IWP-2-8	325	29	0.1	190	575		786	7.5	1913	3400
P-35-9	160	4	190	120	885		405	5.0	1769	1800
P-35-17	390	20	550	230	3360		791	20.0	5360	5000

Table 2, continued

<u>Sample Number</u>	<u>Na⁺ mg/l</u>	<u>K⁺ mg/l</u>	<u>Mg⁺⁺ mg/l</u>	<u>Ca⁺⁺ mg/l</u>	<u>SO₄⁼ mg/l</u>	<u>CO₃⁼ mg/l</u>	<u>HCO₃⁻ mg/l</u>	<u>Cl⁻ mg/l</u>	<u>TDS mg/l</u>	<u>Lab Conductivity (umhos/cm)</u>
P-35-27	410	45	425	250	2800		254	21.0	4205	4200
P-35-42	385	480	240	275	2350		205	34.0	3969	4000
P-39-19	35	17	65	100	220		405	7.0	849	920
P-40-20	5	4	25	80	15		327	2.5	454	510
P-40-45	5	3	20	60	25		220	2.5	331	370
P-36-15	130	17	260	210	1360		700	8.0	2685	2800
P-36-24	110	2	60	75	595		229	10.0	3766	4390
P-36-36	5	4	25	55	5		339	2.5	431	450
P-37-10	345	6	700	300	3540		917	8.5	5817	5300
P-37-15	310	11	510	345	3010		732	10.0	4928	4600
P-37-25	270	13	405	340	2550		644	11.5	4337	4050
P-37-36	65	10	55	45	260		268	5.5	709	790
P-38-15	45	12	125	125	1416		859	5.0	1416	1400
P-38-22	80	9	225	290	1210		725	3.0	2542	2500
P-38-33	240	25	185	235	1670		215	14.0	2584	3000

Table 2, continued

APPENDIX E

GRAPHICAL PRESENTATION
OF GROUNDWATER TEMPERATURE
VARIATIONS

ANNUAL METEOROLOGICAL SUMMARY

53, 1966

1968

WNRE, Pinawa, Manitoba

Health and Safety Branch

Environmental Control Section

Month	Temperature (°F)						Heating Degree Days	Wind		
	Mean Max.	Mean Min.	Mean	Ex-treme Max.	Date	Ex-treme Min.		Date	Average Speed (M.P.H.)	Pre-ailing Directio
January	8.1	-10.1	- 1.0	42	Jan. 21	-42	Jan. 6	2046	6.4	SSE
February	12.4	-11.4	0.5	34	Feb. 3, 29	-34	Feb. 22	1870	6.3	NNW
March	35.1	13.5	24.3	57	Mar. 26	-18	Mar. 11	1261	6.6	N
April	48.8	26.7	37.8	76	Apr. 11	8	Apr. 5	817	7.1	NNW
May	58.6	37.7	48.2	78	May 1	17	May 5	522	8.1	NNW
June	67.6	48.8	58.2	84	June 3	37	June 14	210	6.7	NNW
July	73.3	52.5	62.9	88	July 6	39	July 9	102	6.1	NNW
August	67.7	51.1	59.4	80	Aug. 5	35	Aug. 17	190	6.3	WNW
September	65.2	46.1	55.7	82	Sept. 14	34	Sept. 9	290	5.3	SSE
October	49.6	35.1	42.3	66	Oct. 13	23	Oct. 24	703	8.3	NNW
November	32.9	21.4	27.2	50	Nov. 3	1	Nov. 29	1135	6.4	SSE
December	14.5	0.0	7.3	36	Dec. 2	-29	Dec. 25	1791	5.6	N
Year	44.5	26.0	35.2	88	July 6	-42	Jan. 6	10,937	6.6	

Month	Precipitation (Inches)						
	Rain	Snow	Total Precipitation*	Greatest 24 Hour Precipitation			
				Rain	Date	Snow	Date
January	0.10	16.5	1.75	.09	Jan. 24	5.1	Jan. 11
February	0.01	2.6	0.27	.01	Feb. 26	1.2	Feb. 5
March	0.15	7.7	0.92	.10	March 30	3.9	Mar. 19
April	0.78	3.5	1.13	.39	Apr. 20	2.5	Apr. 1
May	3.27	4.2	3.69	.78	May 8	2.2	May 17
June	5.23	-	5.23	1.54	June 30	-	-
July	4.40	-	4.40	1.11	July 29	-	-
August	4.31	-	4.31	1.01	Aug. 18	-	-
September	2.46	-	2.46	0.89	Sept. 16	-	-
October	1.40	0.3	1.43	0.26	Oct. 15	0.2	Oct. 26
November	0.03	2.2	0.25	0.03	Nov. 1	1.1	Nov. 12
December	-	7.9	0.79	-	-	1.7	Dec. 4
Year	22.14	44.90	26.63	1.54	June 30	5.1	Jan. 11

* Sum of rainfall plus one-tenth of snowfall

ANNUAL METEOROLOGICAL SUMMARY

1969

WNRE, Pinawa, Manitoba

Health and Safety Branch

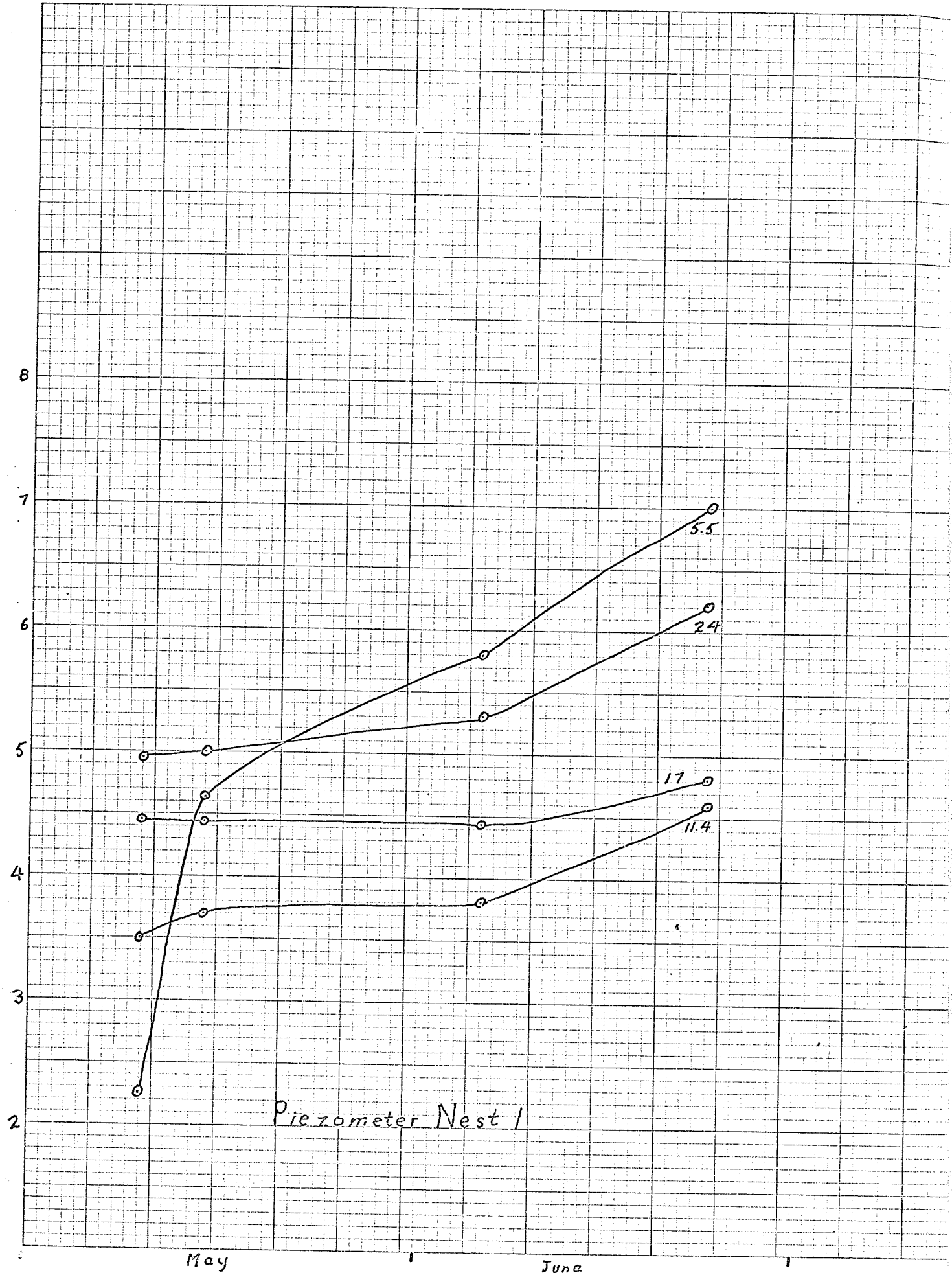
Environmental Control Section

Month	Temperature						Heating Degree Days	Wind		
	Mean Max.	Mean Min.	Mean	Ex-treme Max.	Date	Ex-treme Min.		Date	Average Speed	Pre-vailling Direction
Jan.	4.9	-13.4	-4.2	27	Jan. 15	-38	Jan. 26	2147	6.5	NNW
Feb.	20.0	- 1.6	9.2	36	Feb. 23	-40	Feb. 3	1562	5.0	SSE
Mar.	26.0	4.0	15.0	41	Mar. 21	-15	Mar. 29	1549	5.6	N
Apr.	54.6	29.4	42.0	70	Apr. 25	- 2	Apr. 2	691	6.1	S
May	60.8	39.4	50.1	85	May 26	21	May 19	474	7.8	NNW
June	62.9	41.1	52.0	77	June 9	30	June 13	390	6.6	N
July	74.5	53.5	64.0	93	July 12	38	July 6	87	5.0	SSE
Aug.	79.2	56.6	67.9	90	Aug. 22	46	Aug. 1	35	5.4	SSE
Sept	64.1	45.5	54.8	84	Sept 13	28	Sept. 28	331	5.5	SE
Oct.	41.2	30.0	35.6	54	Oct. 3	15	Oct. 23	913	5.9	SSE
Nov.	33.4	18.8	26.1	59	Nov. 6	- 3	Nov. 20	1167	7.1	NW
Dec.	20.3	8.7	14.5	50	Dec. 1	-19	Dec. 22	1566	5.1	SSE
Year	45.2	26.0	35.6	93	July 12	-40	Feb. 3	10,912	6.0	SSE

Month	Precipitation						
	Rain	Snow	Total Precipitation*	Greatest 24 Hour Precipitation			
				Rain	Date	Snow	Date
January	0.01	27.7	2.78	0.01	Jan. 14	5.6	Jan. 15
February		3.1	0.31			0.7	Feb. 10
March		1.9	0.19			0.8	March 7
April	0.35		0.35	0.28	April 30		
May	2.72	1.5	2.87	1.54	May 31	1.5	May 31
June	2.60		2.60	0.98	June 26		
July	2.06		2.06	0.62	July 26		
August	3.29		3.29	1.33	Aug. 29		
September	2.46		2.46	.60	Sept. 24		
October	.91	1.3	1.04	.31	Oct. 7	0.4	Oct. 24-2
November	.01	3.7	0.38	.01	Nov. 11	0.8	Nov. 23
December		11.9	1.19			2.6	Dec 7
Year	14.41	51.10	19.52	1.54	May 31	5.6	Jan. 15

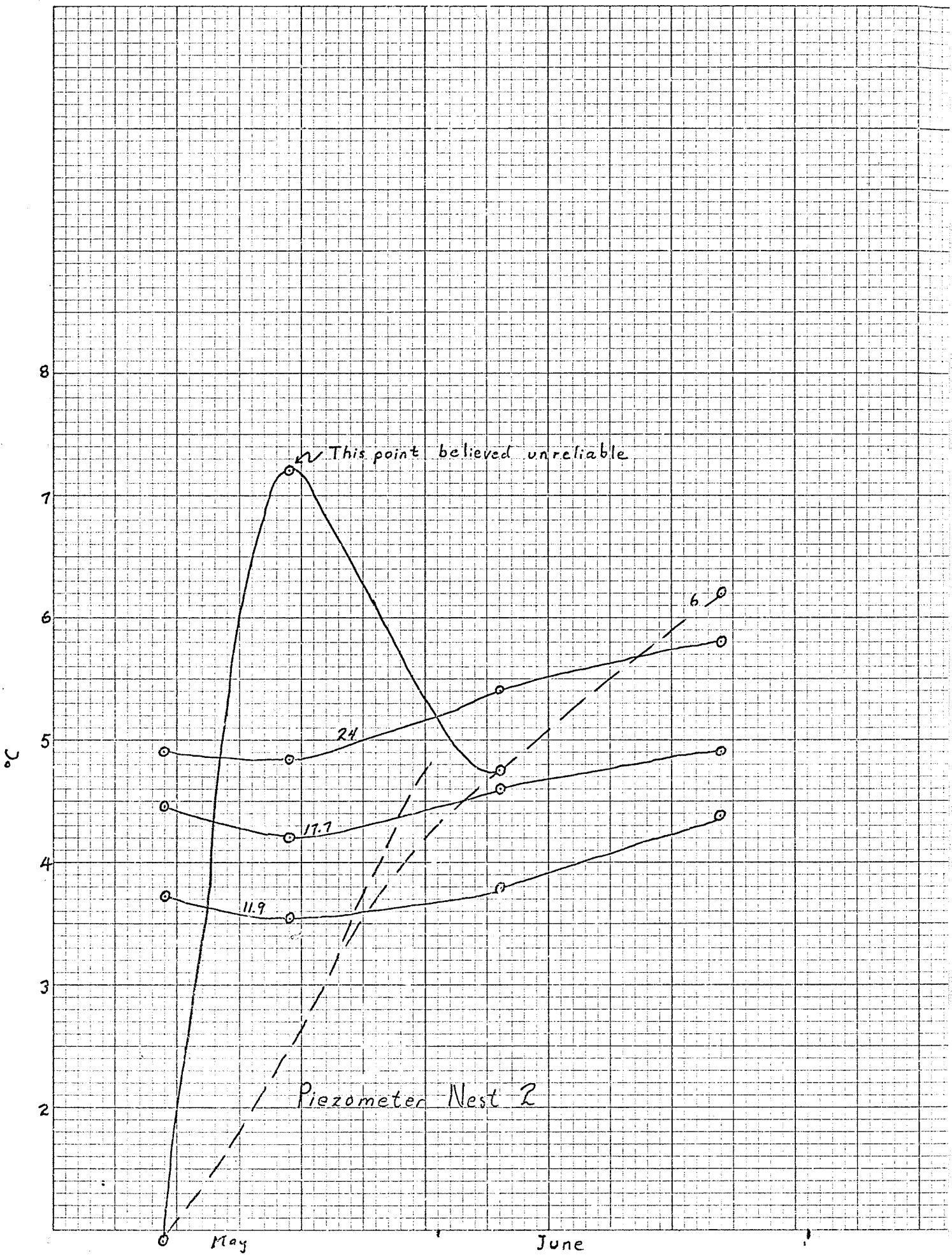
* Sum of rainfall plus one-tenth of snowfall

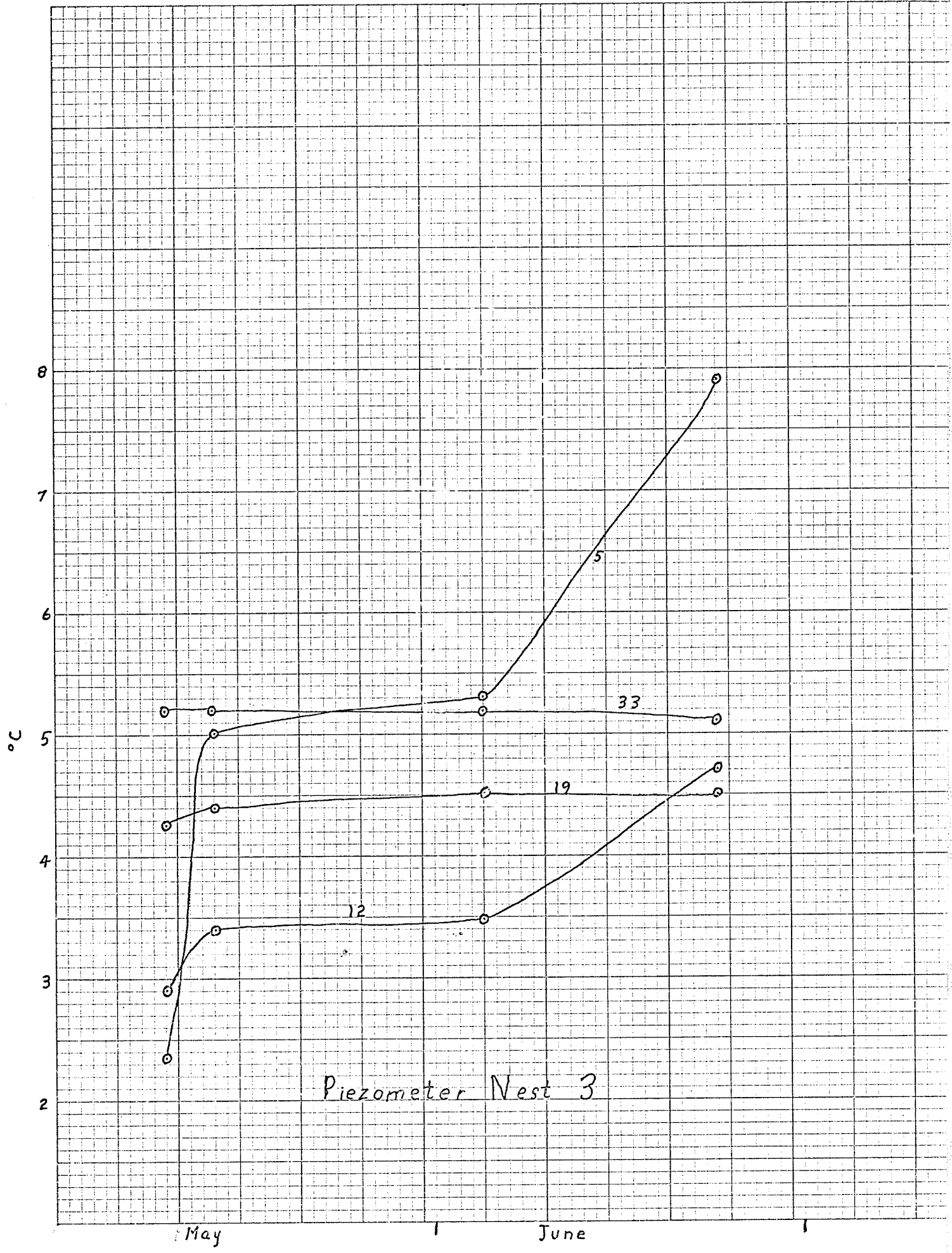
KEUFFEL & ESSER CO.



Piezometer Nest 1

MADE IN U.S.A.
KEUFFEL & ESSER CO.





Piezometer Nest 3

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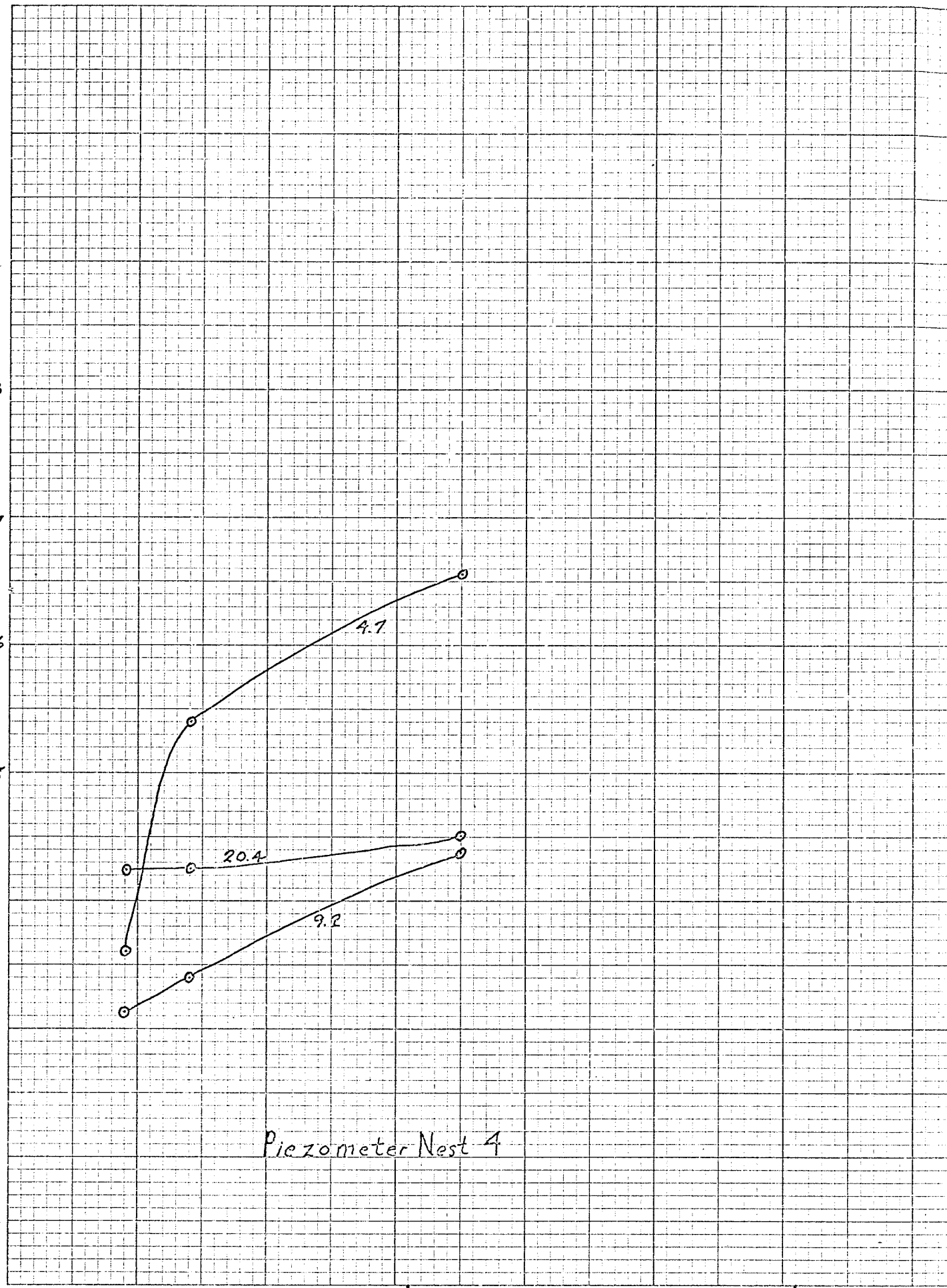
June

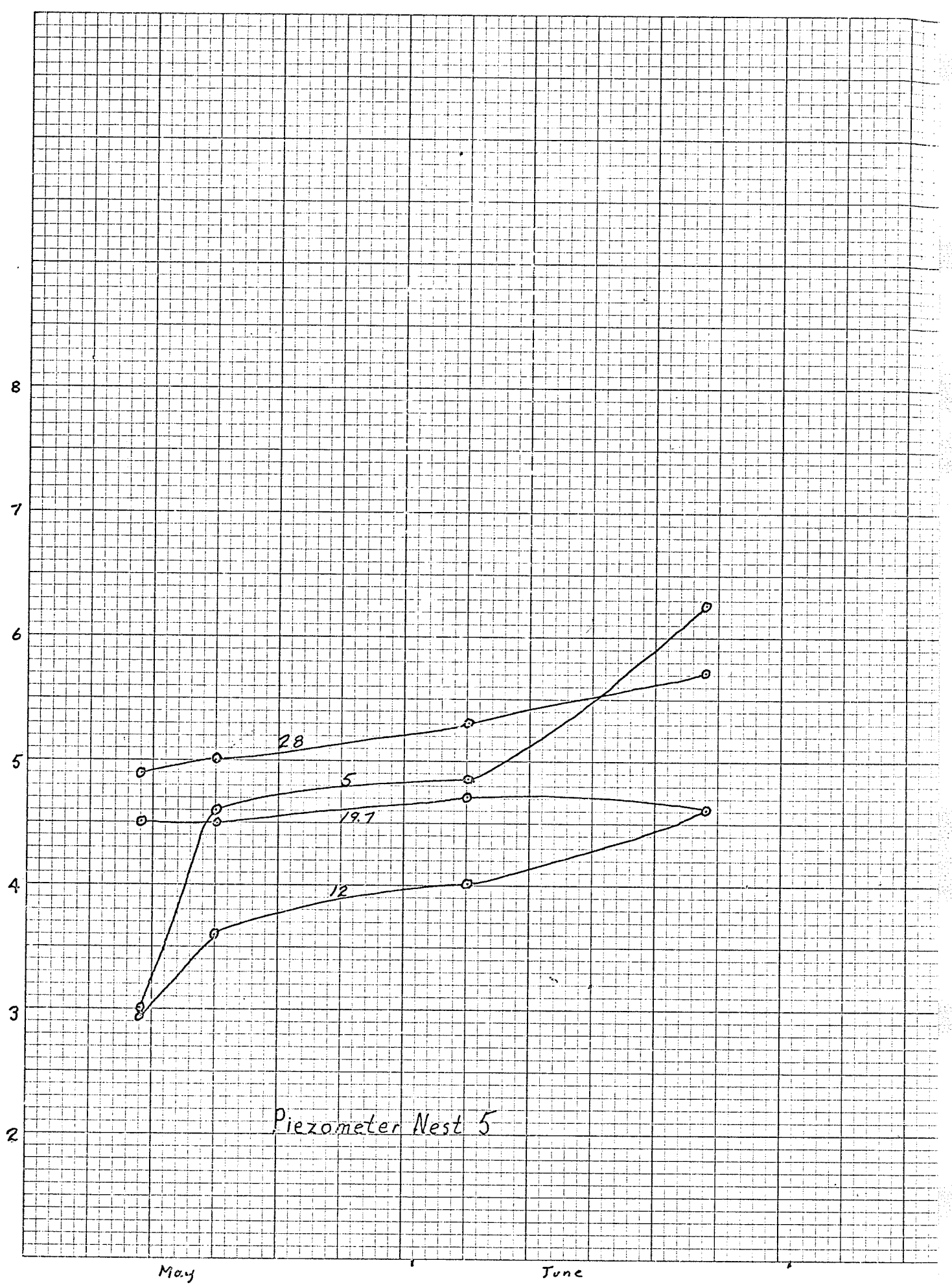
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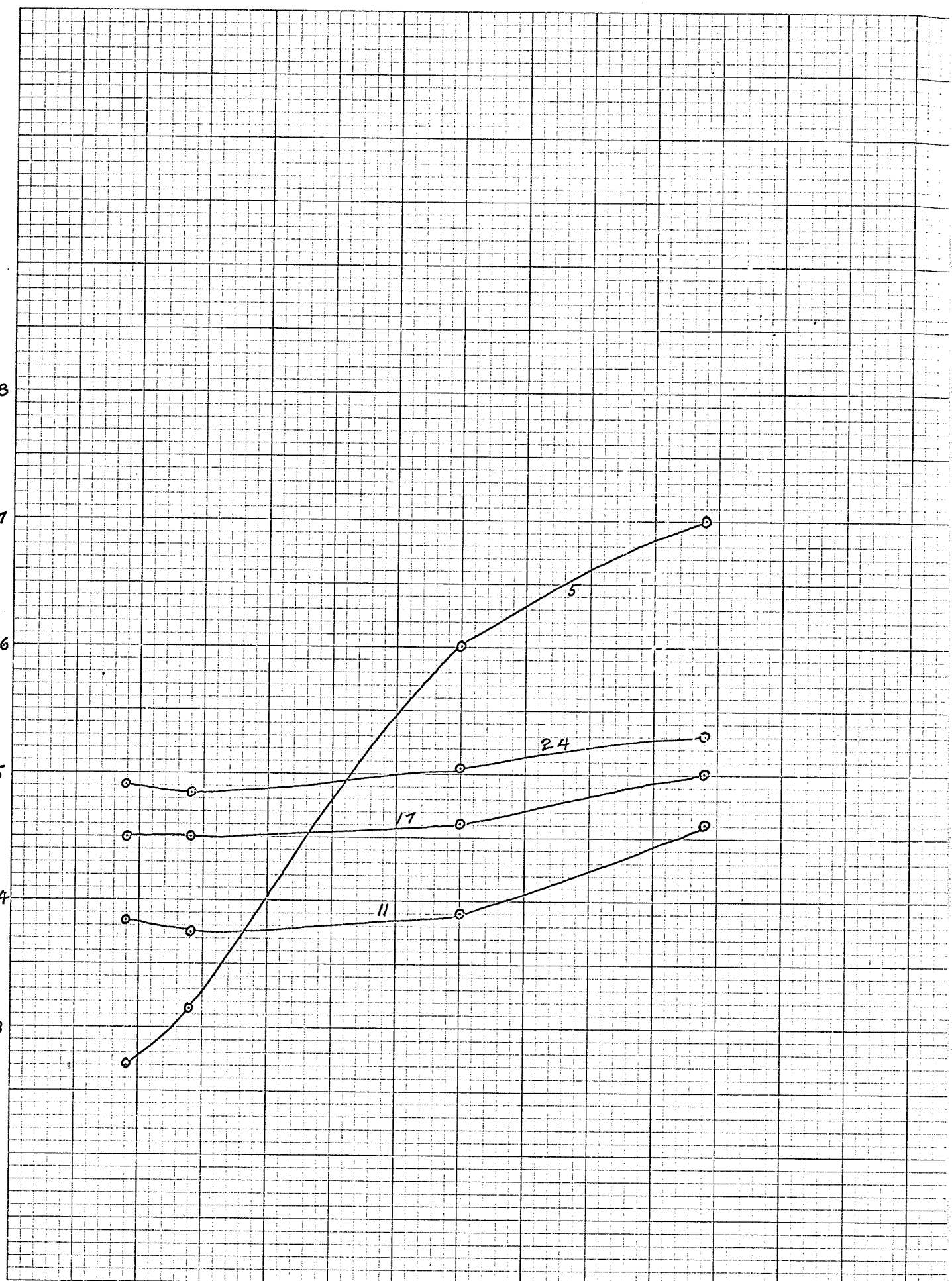


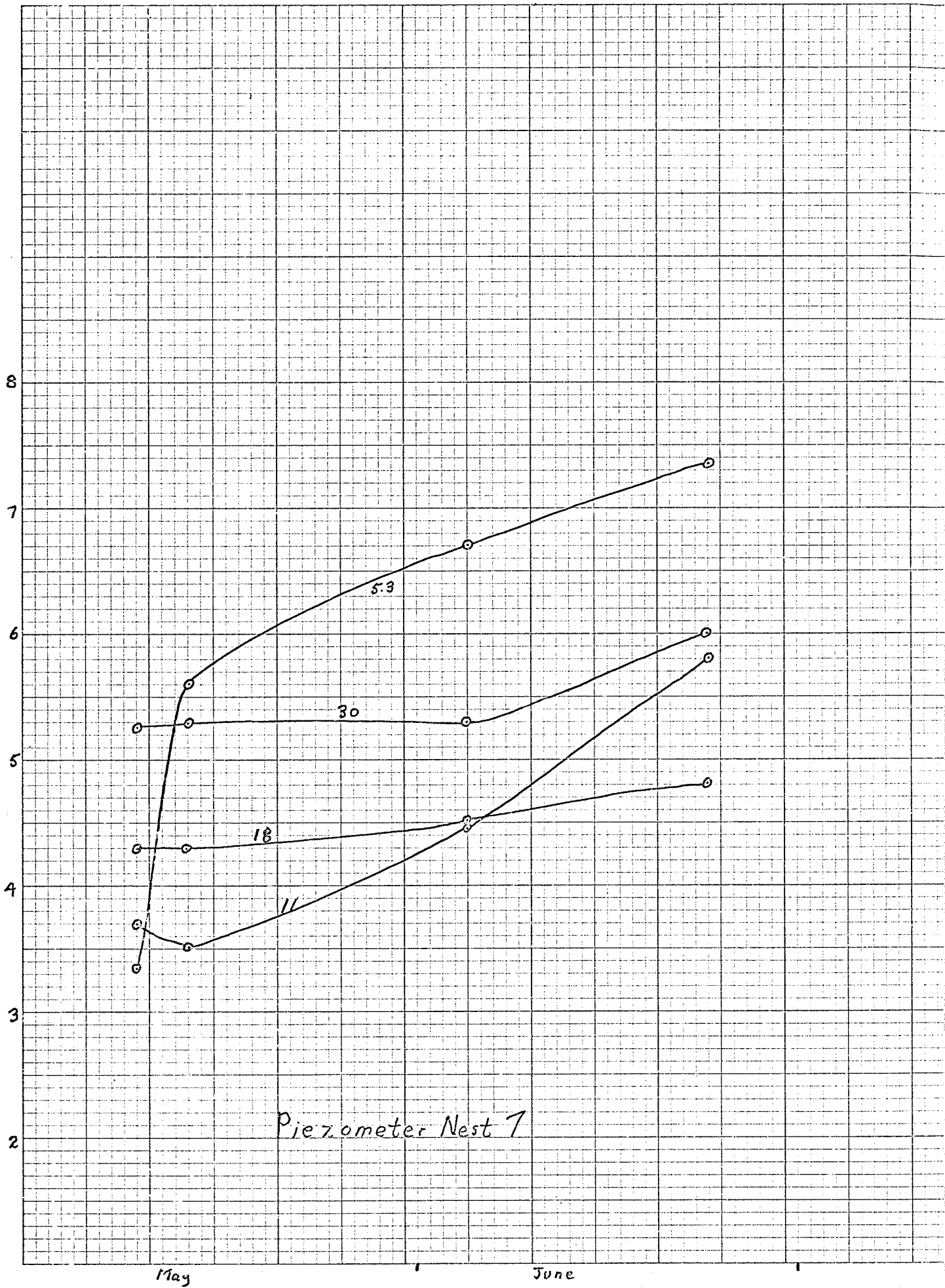
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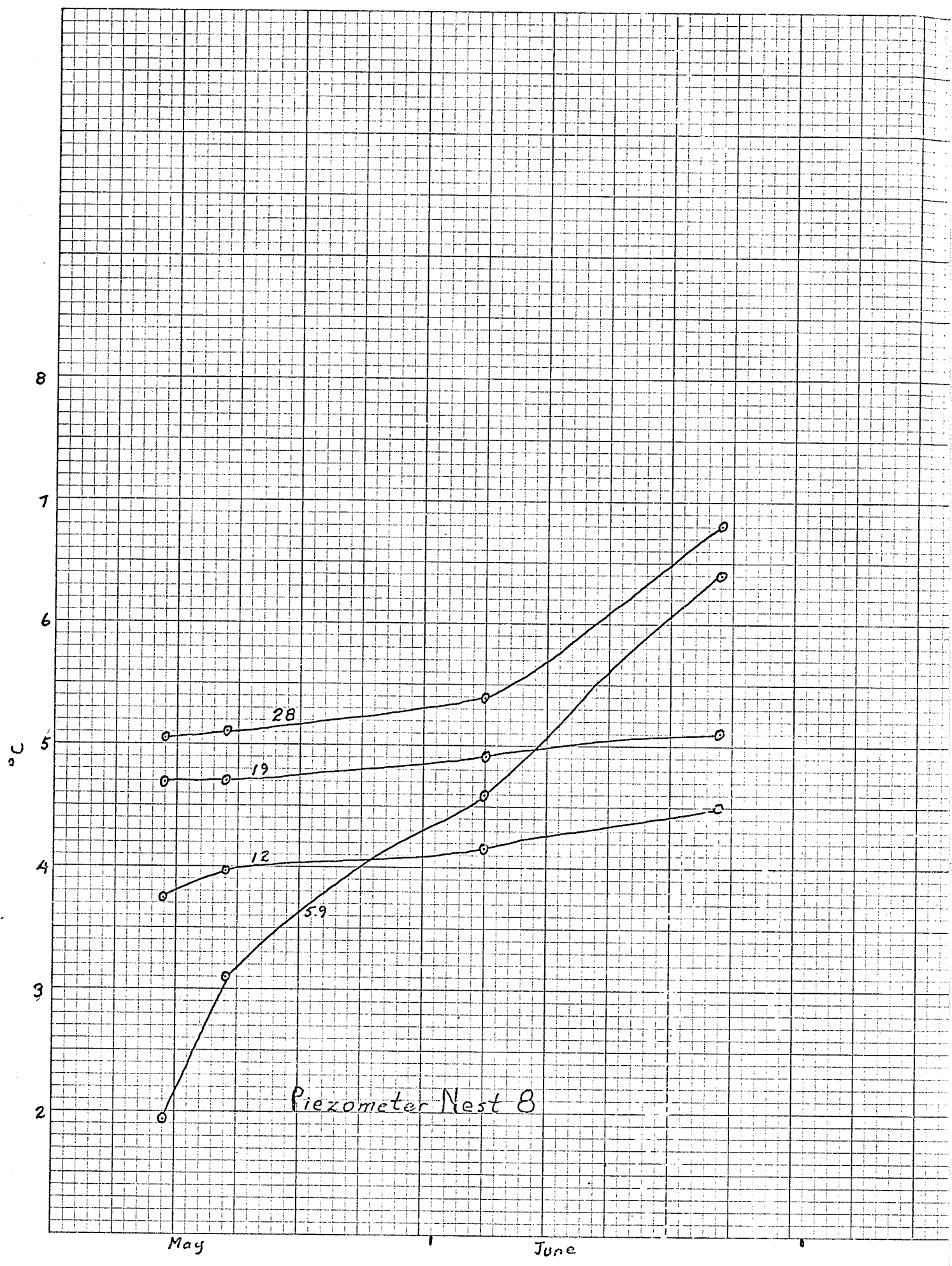
May

June

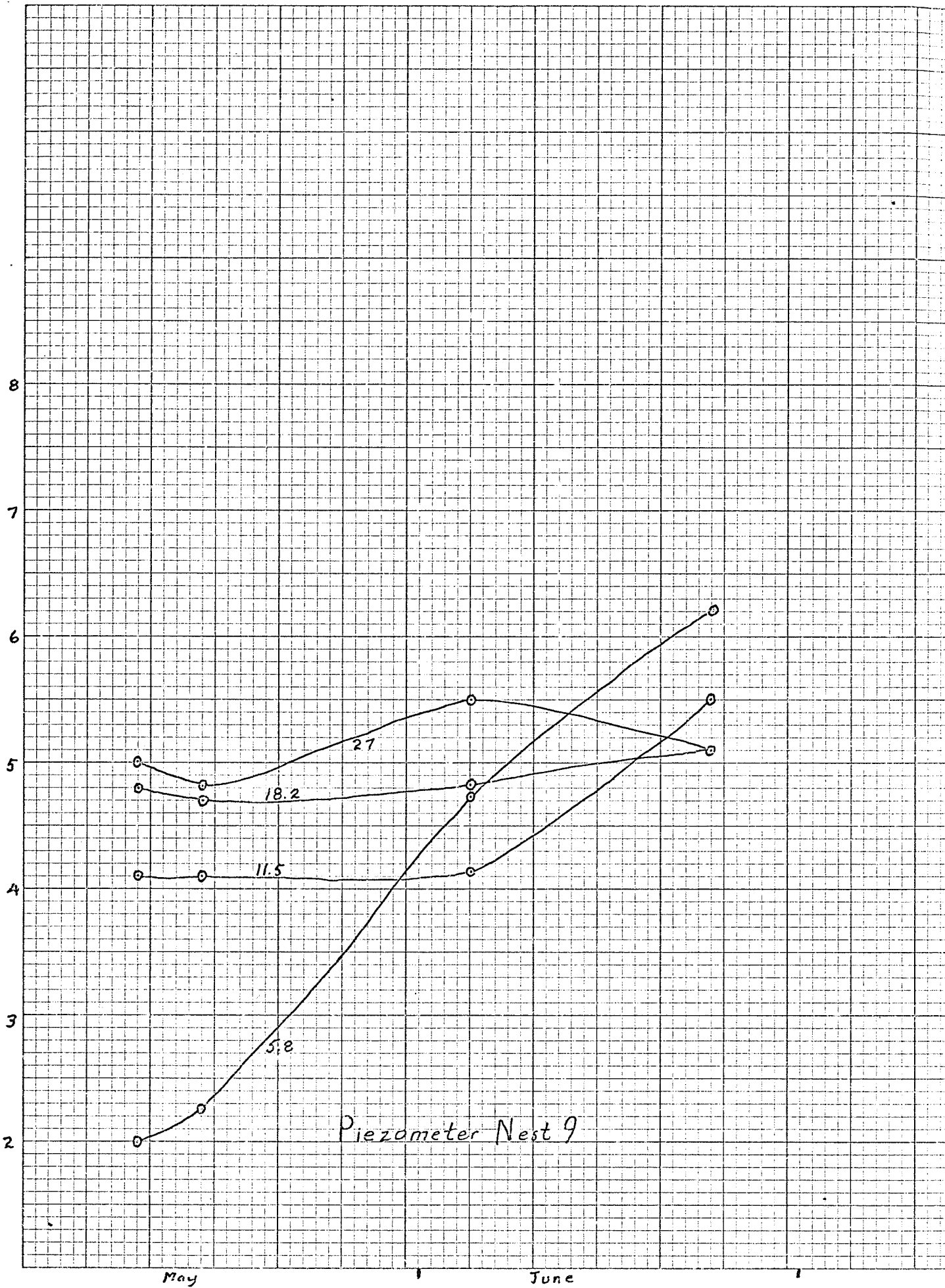
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MADE IN GERMANY
KEUFFEL & ESSER CO.



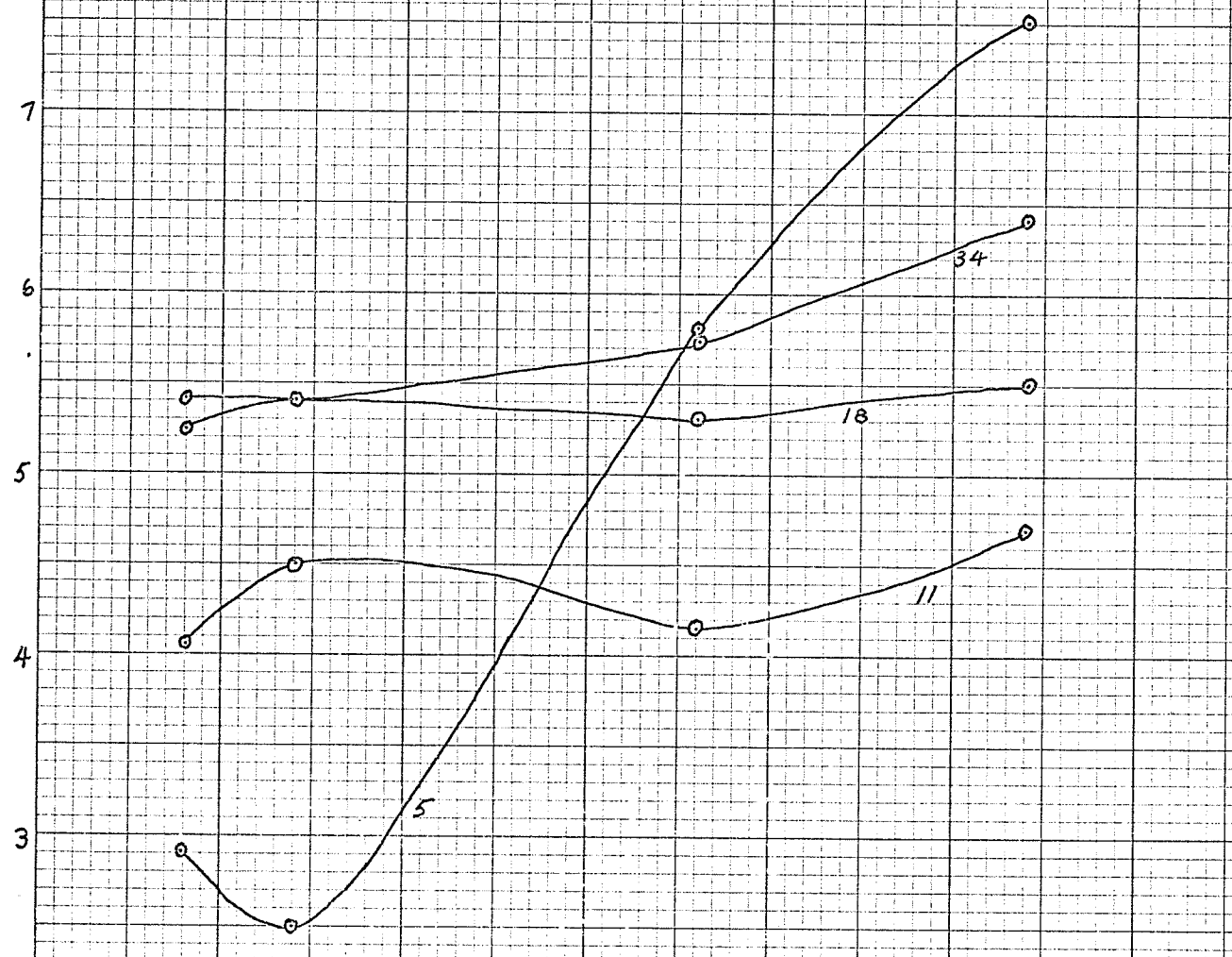
KEUFFEL & ESSER CO. °C

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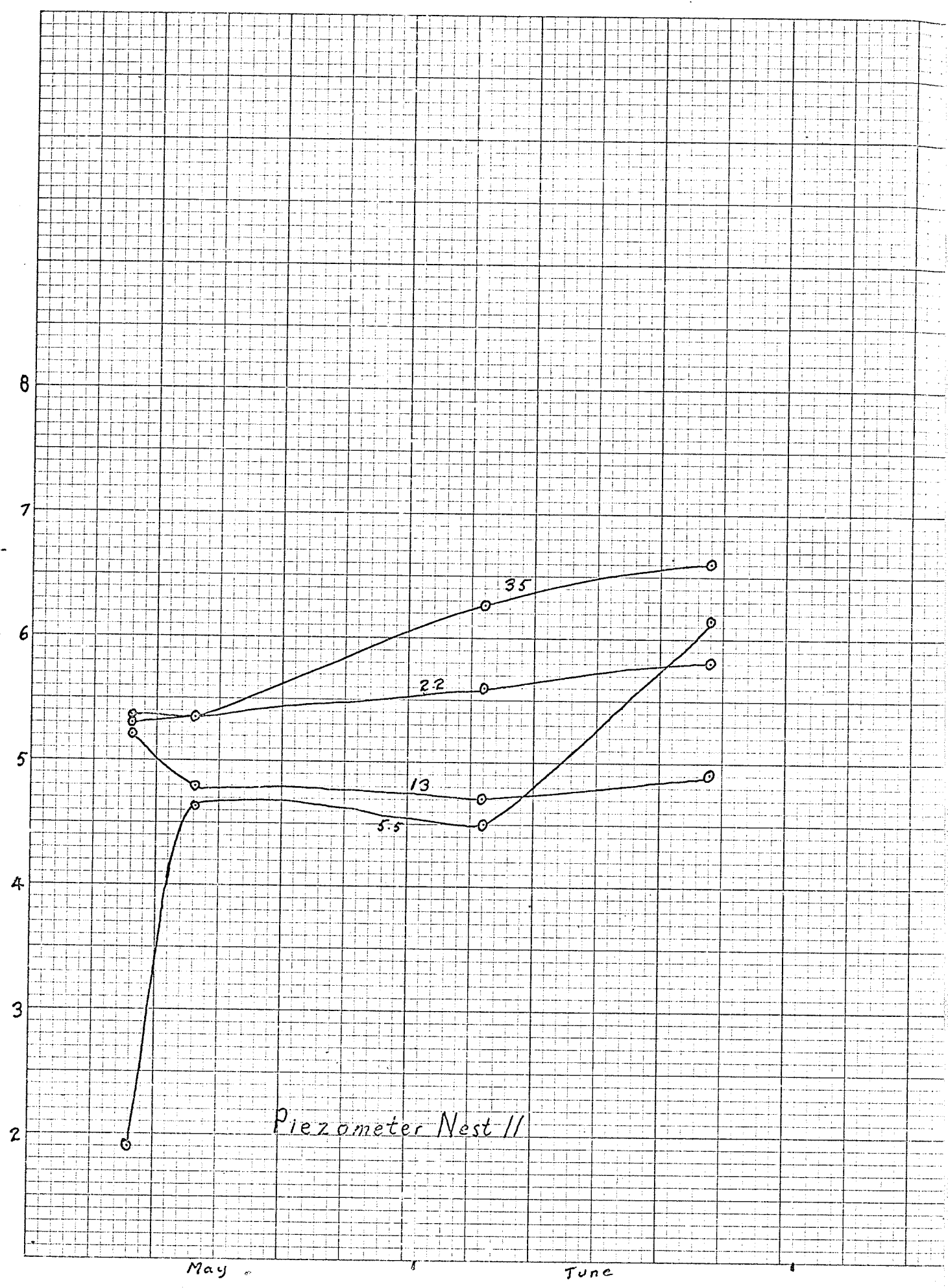
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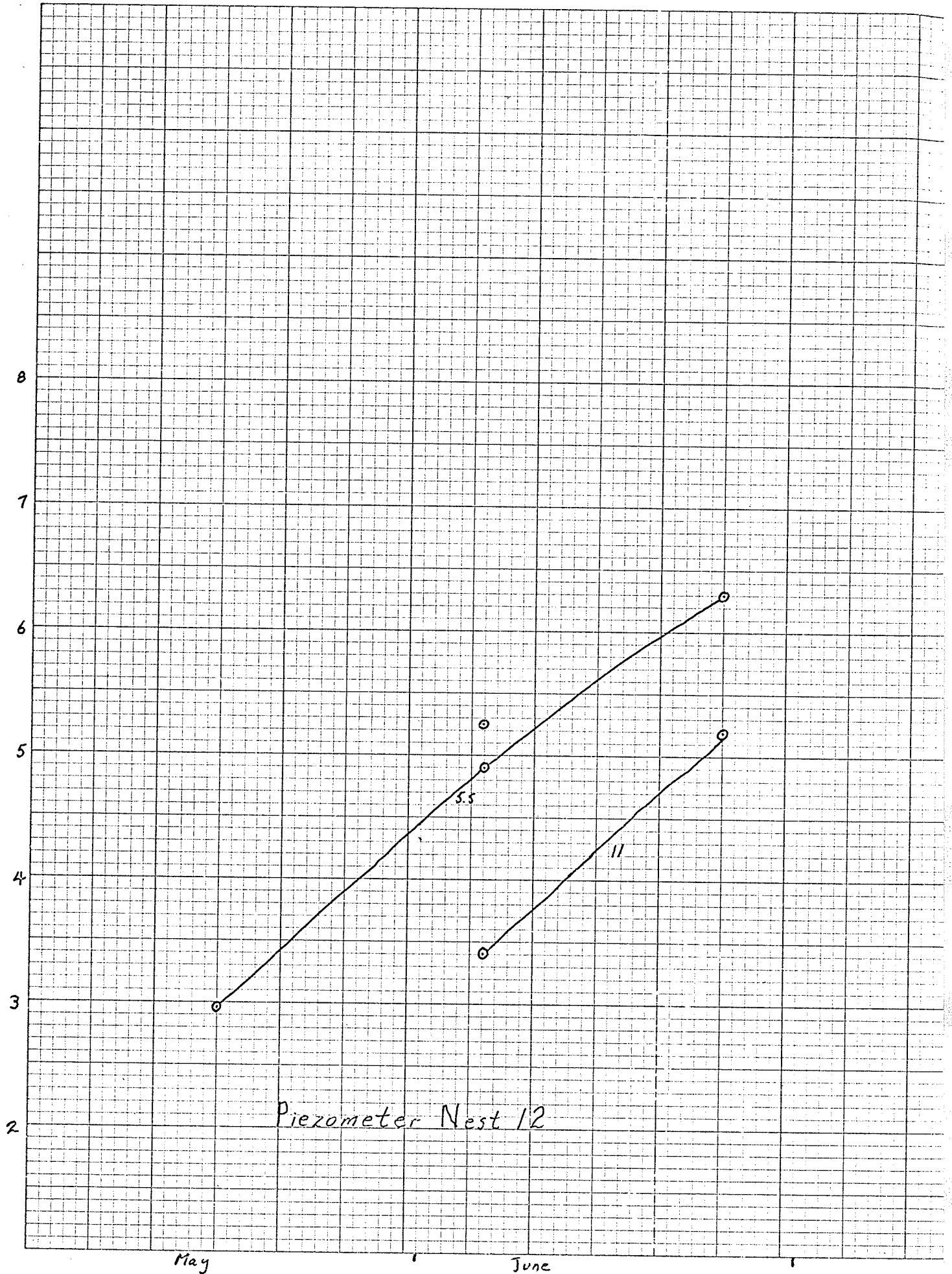
June

Piezometer Nest 10



KEUFFEL & ESSER CO.



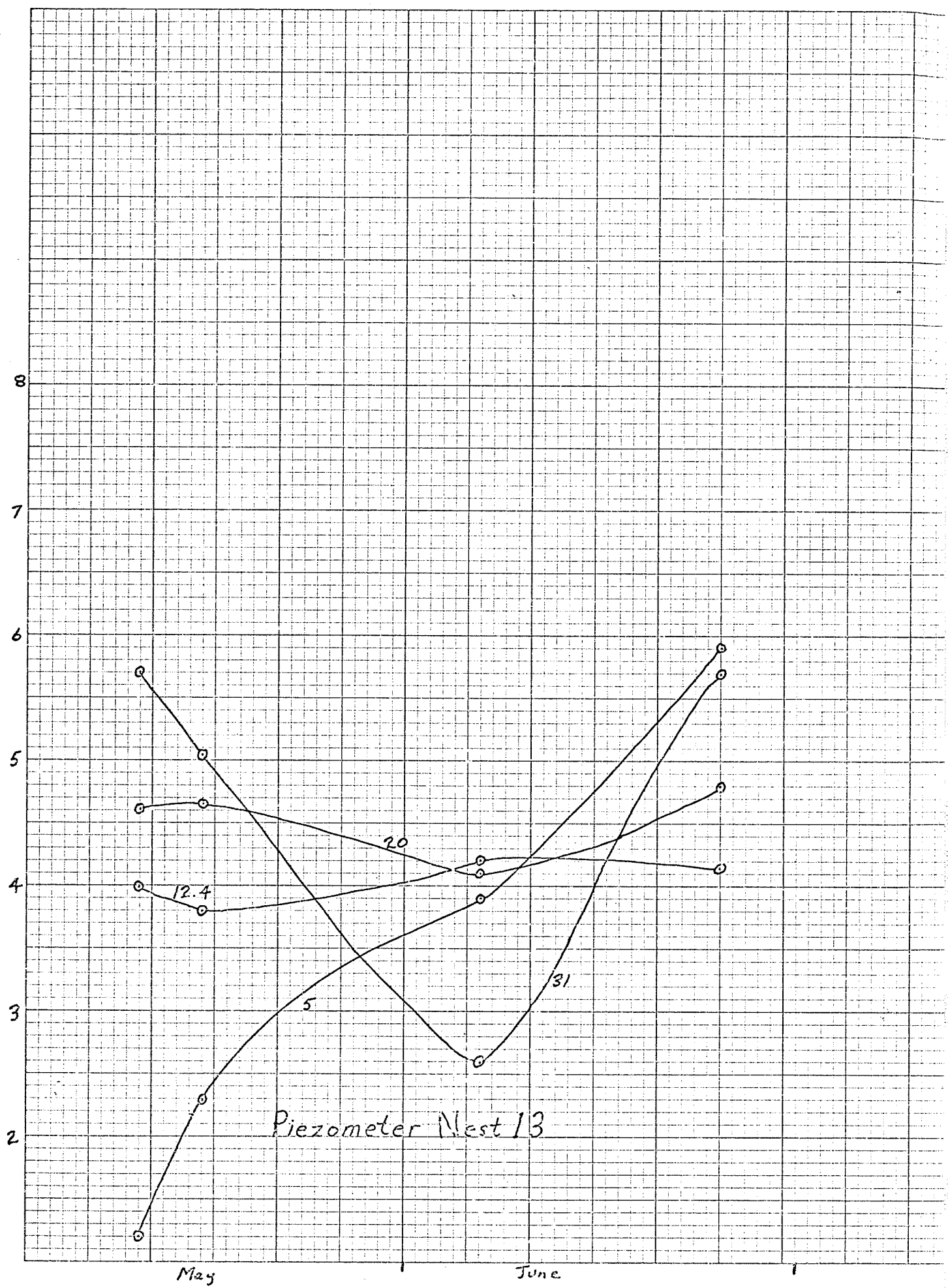


Piezometer Nest 12

May

June

KEUFFEL & ESSER CO.
°C



KEUFFEL & ESSER CO.

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May

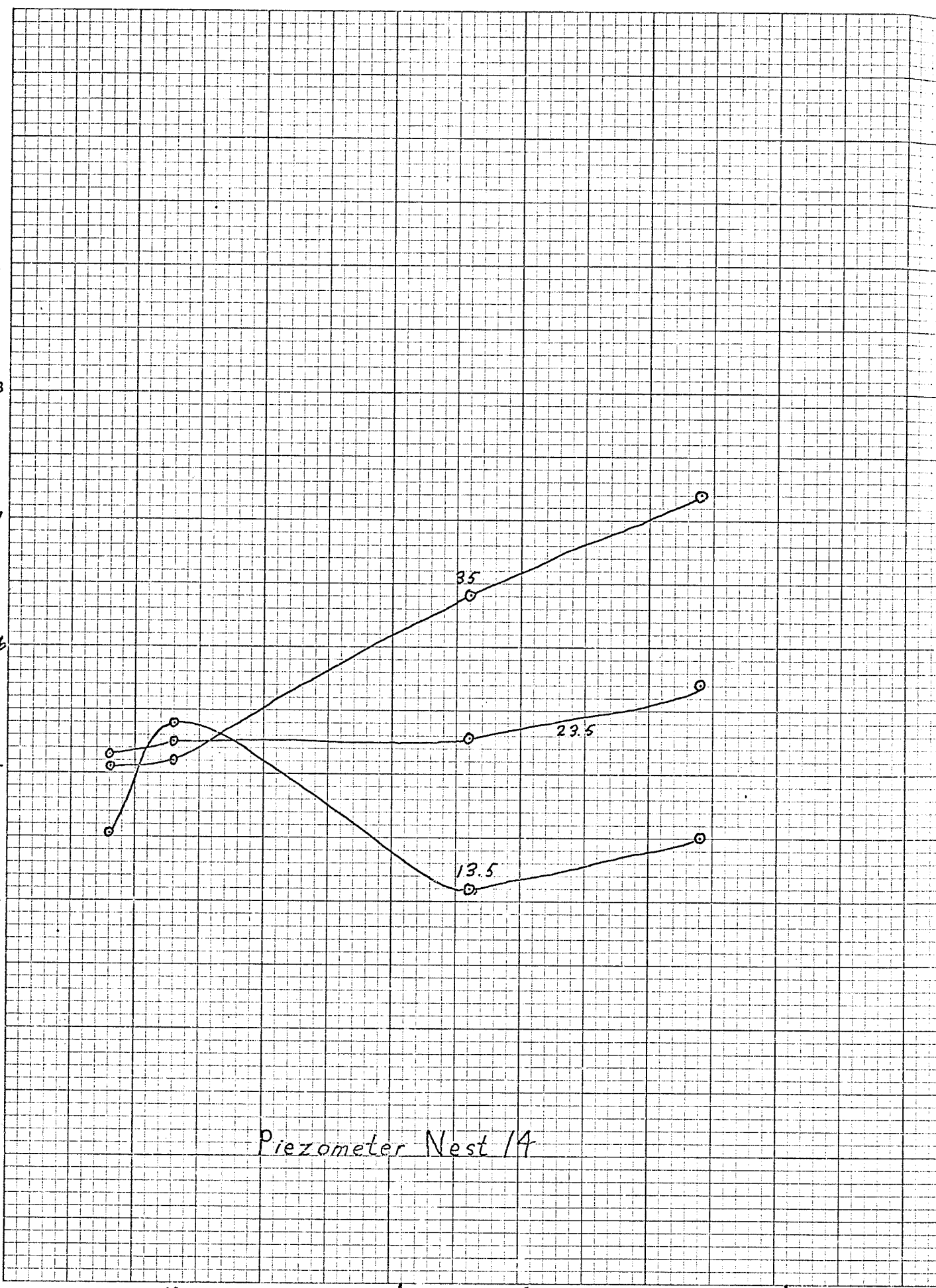
June

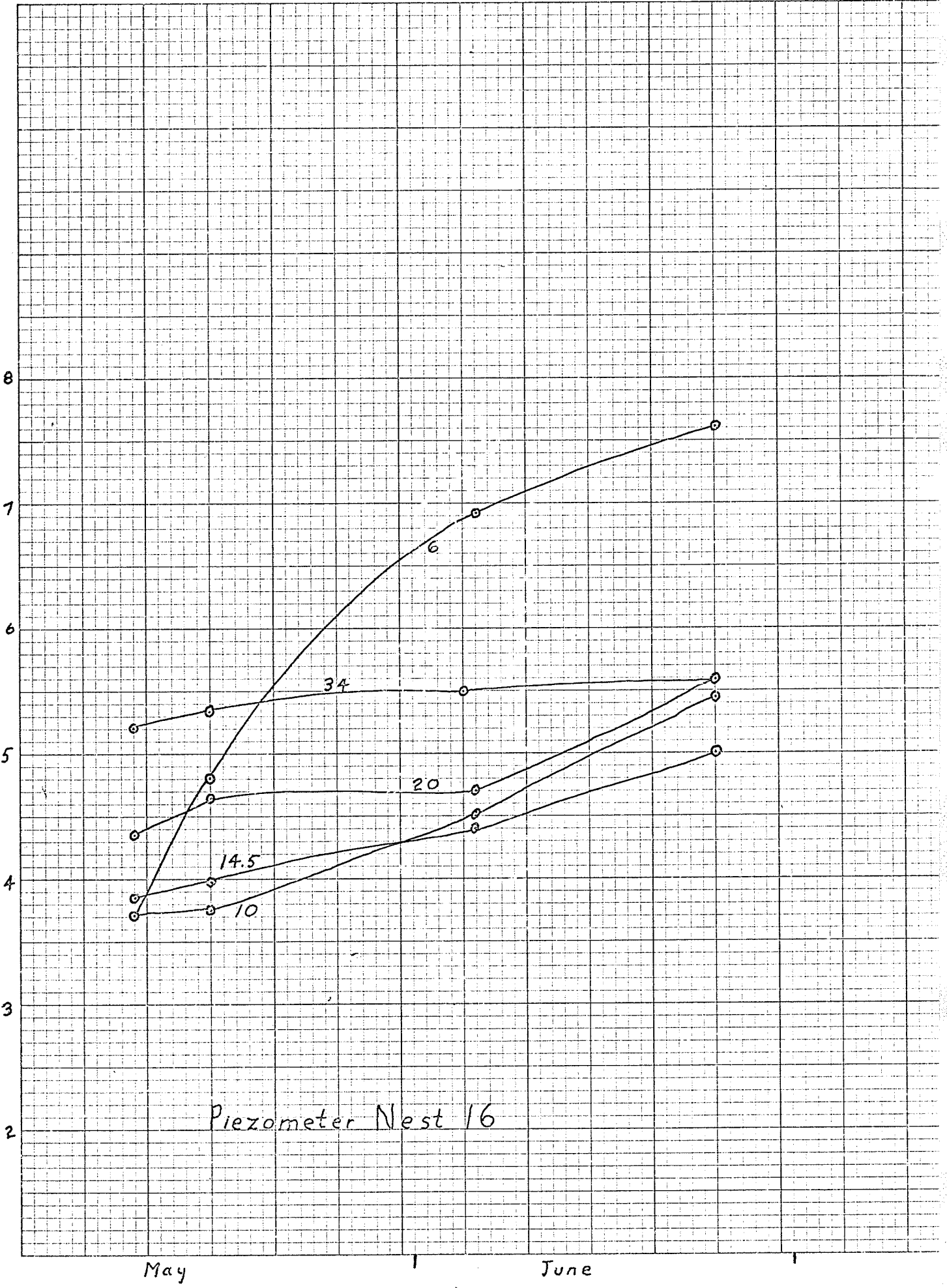
Piezometer Nest 1A

35

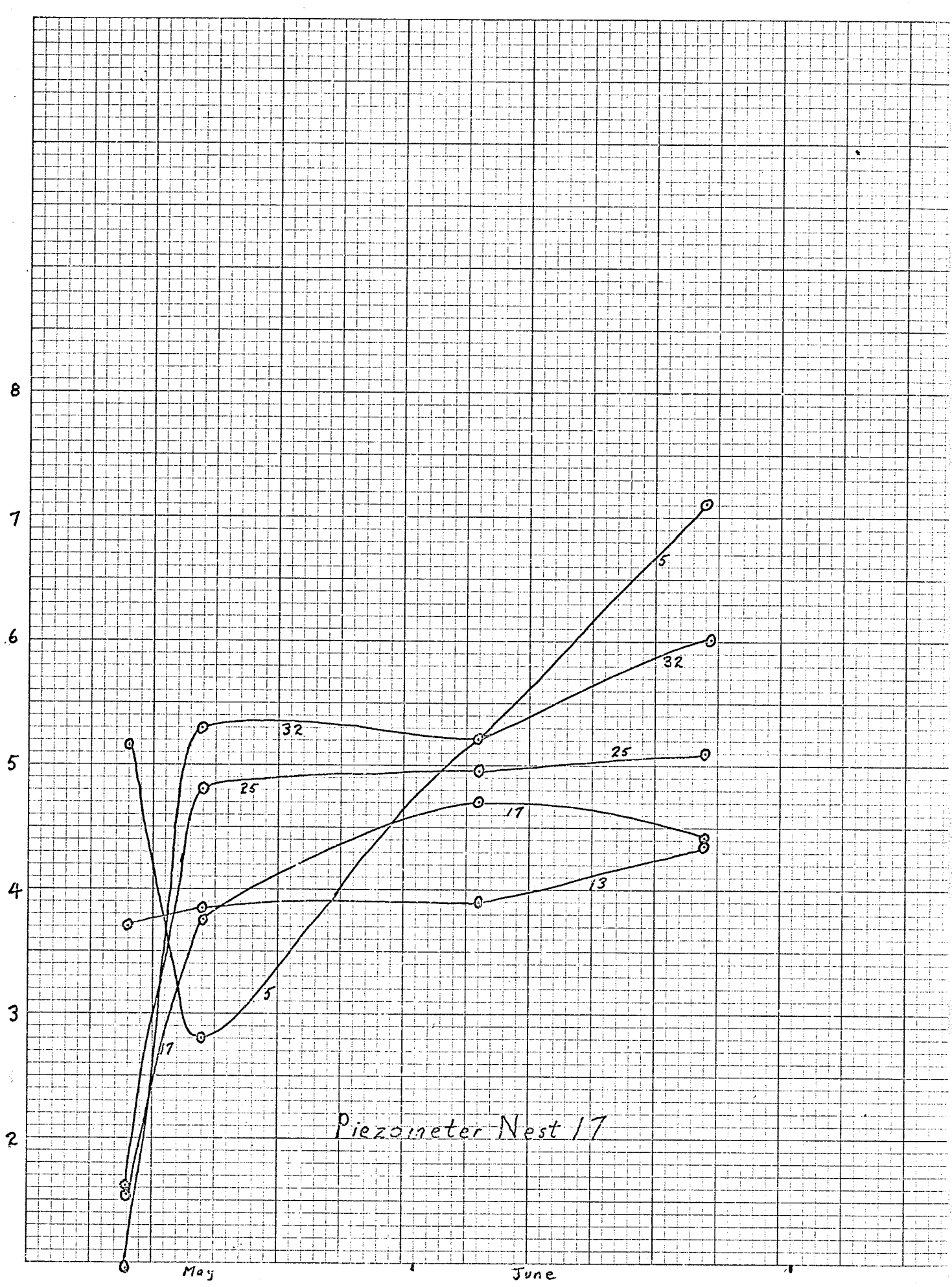
23.5

13.5





REUTHER & LESBEK CO.



Piezometer Nest 17

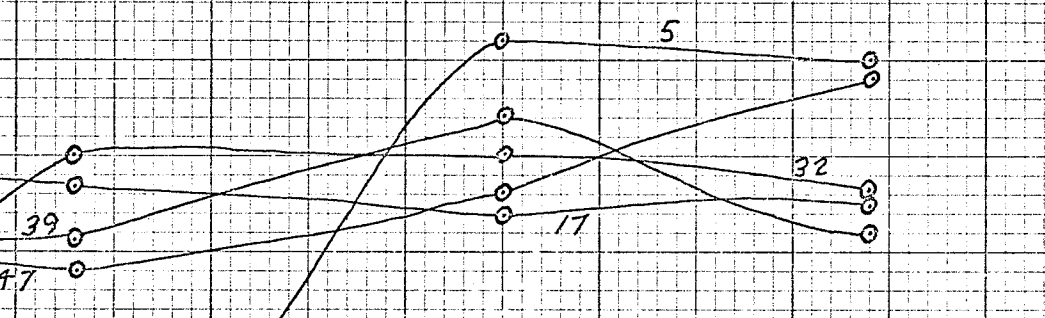
KEUFFEL & ESSER CO. °C

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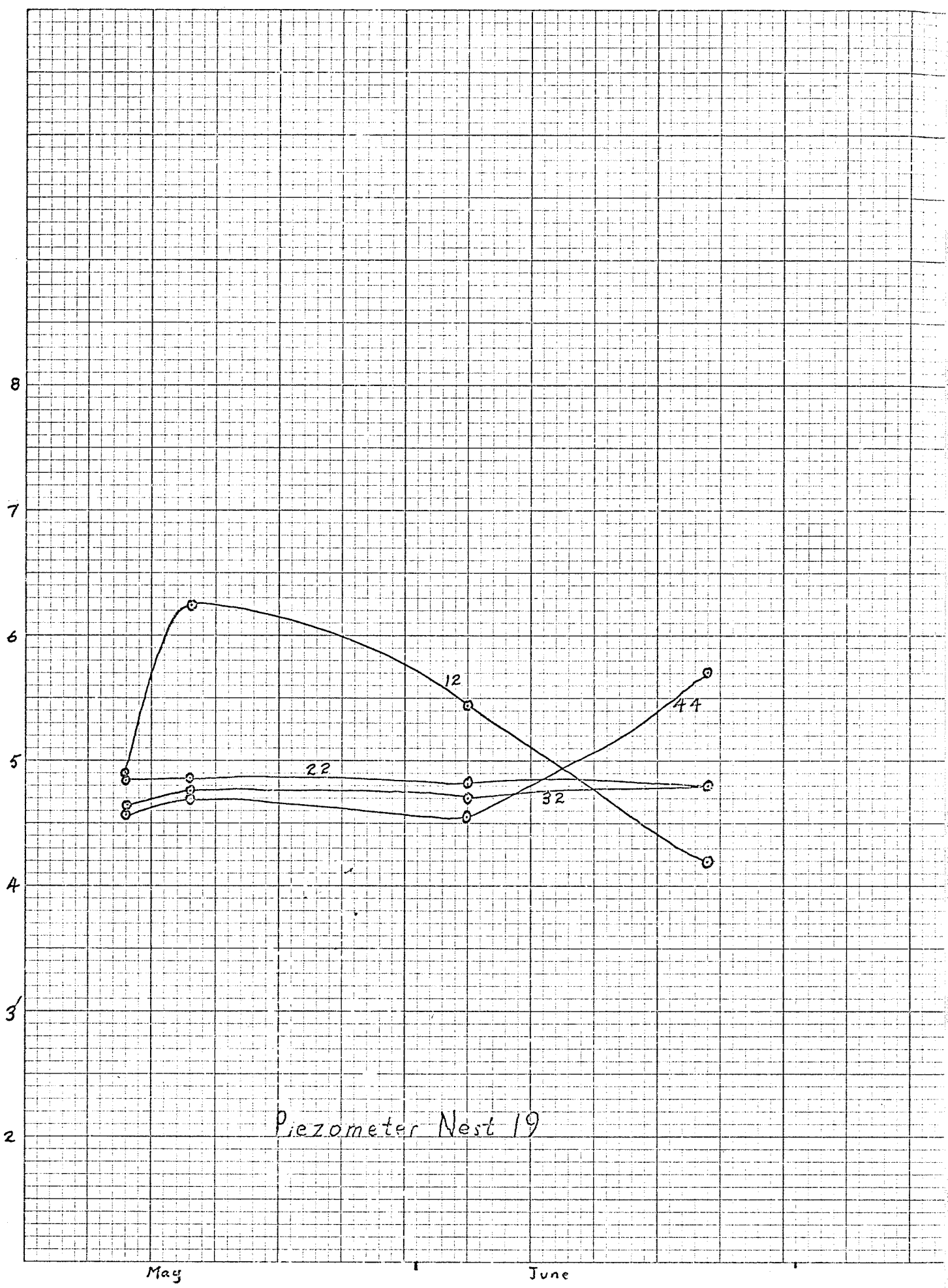
May

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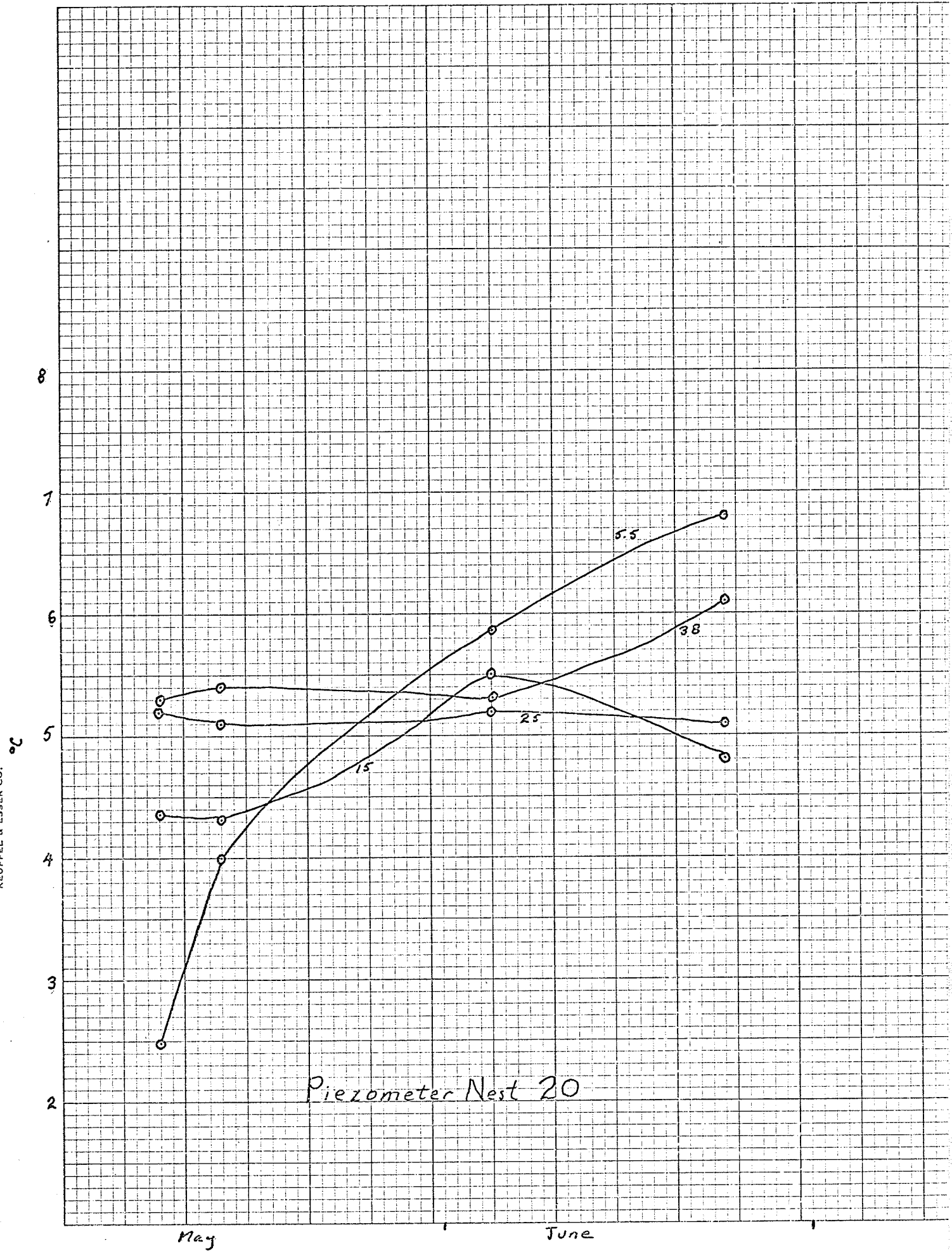
Piezometer Nest 18



KEUFFEL & ESSER CO.



Piezometer Nest 19



Piezometer Nest 20

May

June

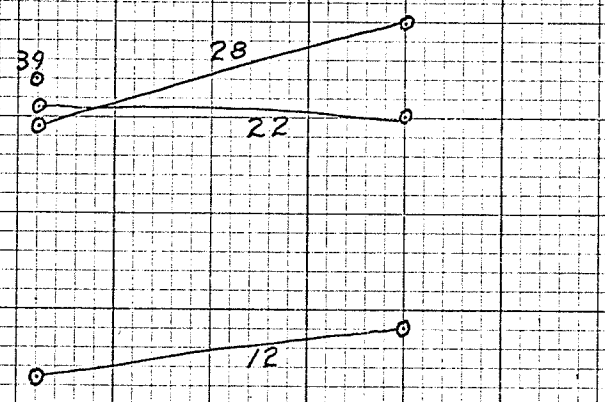
KEUFFEL & ESSER CO.

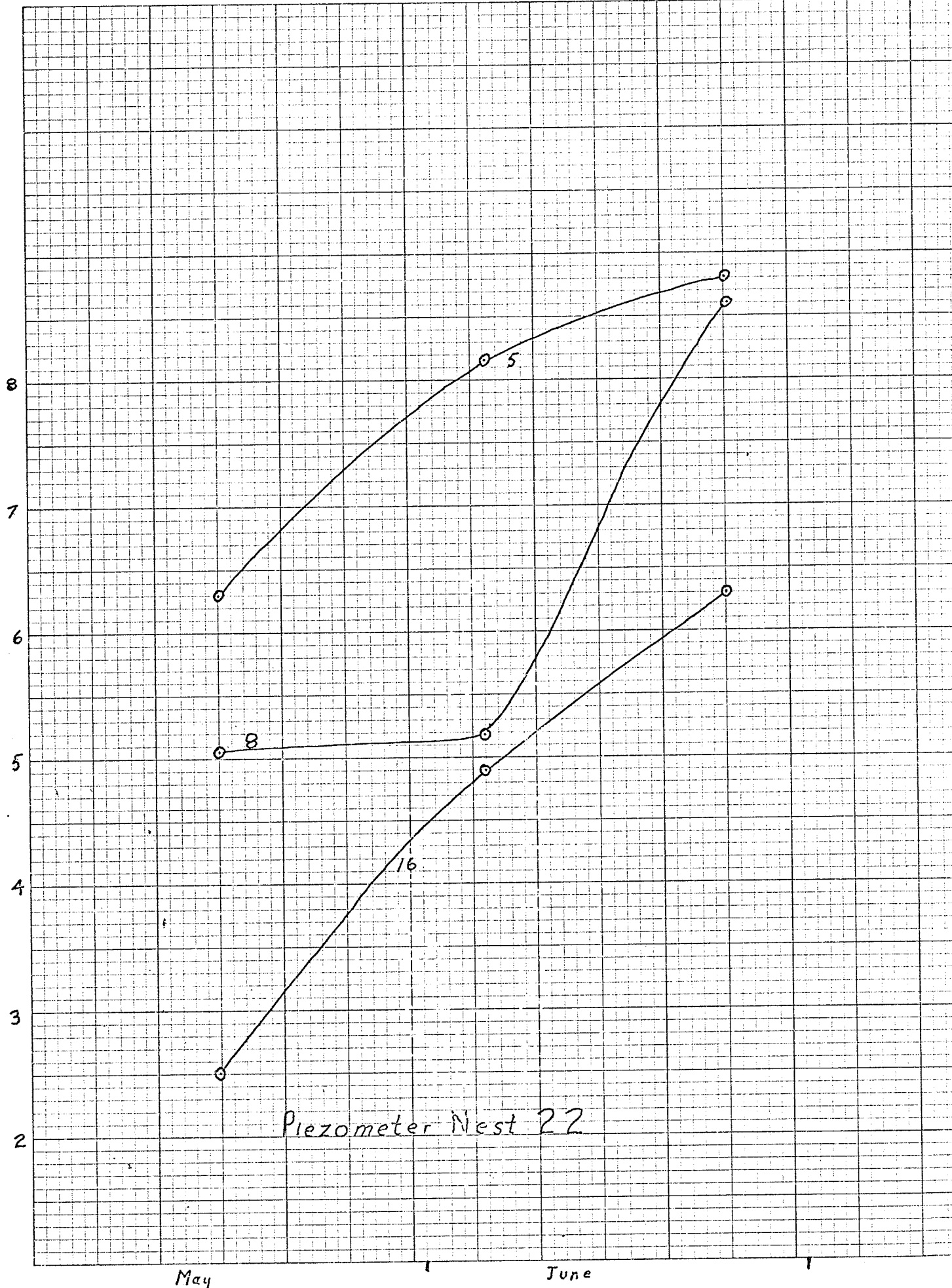
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May

June

Piezometer Nest 21





Piezometer Nest 22

APPENDIX F
TRACER VELOCITIES IN GROUNDWATER

Tritiated heavy water was injected into the flow regime under pressures of less than 10 psi. A schematic of the injection system is shown in figure 13A.

Samples were taken with a vacuum pump connected to flasks in which a 20 - 50 ml sample was collected. To ensure a fresh sample for every sampling period, a 100 ml aliquot was taken and discarded prior to final sampling. A down-hole sampler identical to that used for the geochemical samples (Fig. 11) was used for the initial few samples but the difficulty of preventing contamination resulted in a modification of sampling technique. Each sample point was given individual vacuum lines and a vacuum flask was used for sampling (Fig. 15A).

For analysis, an aliquot of sample was placed in 20 ml of Chicago Nuclear NE220 liquid scintillator in 25 ml scintillation counter (low sodium) glass vials and counted by a Chicago Nuclear 3-channel scintillometer for beta activity. Results were printed and stored on paper tape and counts repeated to ensure reliable values. Background radiation was not considered because of its relatively stable 18-25 cpm during the period of study. Decay of the tritium was not considered because of the relatively short period of study relative to the 12.26 year half-life of tritium.

Tabulated activity counts are given only for sample tubes considered active or for those indicating contamination.

INJECTION SITE #1

July 2, 1969 - injection of 20 gallons solution of 20.3 gms fluorescein at 10 psi to ensure adequate safety precautions for tritium injection. Immediate response of water level in all sample tubes noted when pressure applied. Injection rate @ 10 psi approximately 2 gpm.

July 9, 1969 - 1 curie tritium diluted in 20 gallons water and injected under less than 10 psi followed by 20 gallons water to flush all tritium into the formation. All sample tubes were corked during injection to prevent overflow due to pressure. Fluorescein noted in sample tube 15 immediately after injection. Samples taken immediately after injection showed IWP-1-15 to have count of 210 cpm. Samples taken periodically over the next 96 hours showed a rapid increase to 148,000 cpm/ml. A peak count in sample tube 15 of 826,445 cpm/ml was recorded on August 15, 1969. By November 5, the count had fallen to 675,675 cpm/ml.

The injection well samples indicated counts of 4.78×10^3 cpm/ml (top) and 3.15×10^5 cpm/ml (bottom) on July 21. The difference was a result of the injection of pure water following the tritium. A peak of 3.65×10^6 cpm/ml was reached on August 22, 1969 and by May 15, 1970 the count had fallen to 3.9×10^5 .

Sample tube 13 began to show above background beta activity 8 hours after injection. Values fluctuated but reached a peak of 4,470 on August 17, 1969. A value of 12,300 cpm/ml on July 19 is felt to be in error. By November 5, 1969 the value fell off to 460 cpm/ml.

No other sample tubes indicated the presence of tritium.

TRITIUM ANALYSES RECORD - IWP-1-INJ

<u>Date</u>	<u>Activity (cpm/ml)</u>	<u>Date</u>	<u>Activity (cpm/ml)</u>
21/7	4.780×10^3	6/ 8	--
21/7	3.150×10^5	6/ 8	1.462×10^6
22/7	4.149×10^5	7/ 8	2.084×10^6
23/7	6.493×10^5	8/ 8	1.429×10^6
24/7	7.751×10^5	10/ 8	1.916×10^6
25/7	9.708×10^5	11/ 8	2.080×10^6
28/7	--	13/ 8	1.770×10^6
28/7	1.405×10^6	14/ 8	2.220×10^6
29/7	1.456×10^6	15/ 8	1.740×10^6
30/7	7.619×10^5	17/ 8	1.870×10^6
31/7	9.709×10^5	18/ 8	1.585×10^6
1/8	9.346×10^5	22/ 8	5.55×10^5
3/8	1.0×10^6	29/ 8	3.65×10^6
4/8	--	7/ 9	1.136×10^6
4/8	1.550×10^6	17/ 9	7.368×10^5
5/8	--	8/10	8.620×10^5
5/8	1.417×10^6	5/11	5.848×10^5

TRITIUM ANALYSES RECORD - IWP-1-15

<u>Date</u>	<u>Activity (cpm/ml)</u>	<u>Date</u>	<u>Activity (cpm/ml)</u>
9/7	210	28/ 7	529,100
10/7	51,400	29/ 7	549,450
10/7	70,600	30/ 7	552,485
10/7	95,000	31/ 7	537,633
10/7	119,000	1/ 8	502,500
11/7	102,000	3/ 8	507,613
12/7	100,000	4/ 8	526,315
13/7	148,000	5/ 8	518,134
14/7	168,500	6/ 8	490,195
15/7	170,000	7/ 8	555,555
15/7	177,600	8/ 8	588,234
16/7	290,000	10/ 8	609,755
16/7	293,000	11/ 8	653,594
17/7	288,000	13/ 8	704,224
18/7	357,000	14/ 8	781,249
19/7	367,000	15/ 8	826,445
20/7	385,000	22/ 8	734,607
21/7	426,000	17/ 8	793,650
22/7	438,000	18/ 8	793,650
23/7	448,430	29/ 8	757,120
24/7	462,962	7/ 9	775,193
25/7	487,804	17/ 9	813,674
27/7	490,195	8/10	746,268

TRITIUM ANALYSES RECORD - IWP-1-13

<u>Date</u>	<u>Activity (cpm/ml)</u>	<u>Date</u>	<u>Activity (cpm/ml)</u>
9/7	7	29/ 7	910
10/7	36	30/ 7	775
10/7	91	31/ 7	940
10/7	134	1/ 8	850
10/7	303	3/ 8	850
11/7	453	4/ 8	1,325
12/7	535	5/ 8	950
13/7	540	6/ 8	1,565
14/7	508	7/ 8	2,060
15/7	435	8/ 8	1,755
15/7	235	10/ 8	2,095
16/7	375	11/ 8	3,171
16/7	625	13/ 8	2,899
17/7	625	14/ 8	2,436
18/7	2,225	15/ 8	2,650
19/7	12,300	22/ 8	3,200
20/7	4,700	17/ 8	4,467
21/7	2,750	18/ 8	2,330
22/7	1,675	29/ 8	1,782
23/7	875	7/ 9	1,494
24/7	1,175	17/ 9	1,289
25/7	1,025	8/10	816
27/7	1,125	5/11	460
28/7	655		

INJECTION SITES #2 & #3

The procedure used for injection #1 was followed for injections 2 and 3 with the exception that the fluorescein test was not used. In injections 2 and 3 only 10 millicuries of tritium was injected rather than the 1 curie used at 1.

Serious sample tube contamination occurred in injection site 3 in sample tube 20 and 24. It probably resulted from contamination during the initial few sampling periods before a routine sampling procedure was developed. The anomalous values are not felt to be true indicators of the subsurface movement of tritium because the values are very low relative to what would be expected from the 10 mCi injection.

A series of peaks and declines in beta activity in the active holes is felt significant, although of what is not certain.

TRITIUM ANALYSES RECORD - IWP-3-INJ

<u>Date</u>	<u>Activity (cpm/ml)</u>	<u>Date</u>	<u>Activity (cpm/ml)</u>
20/8	495,282	3/ 9	226,244
20/8	686	5/ 9	204,498
20/8	248	10/ 9	182,546
20/8	26,404	13/ 9	168,960
24/8	189,907	17/ 9	147,547
28/8	252,650	20/ 9	163,335
22/8	93,023	24/ 9	307,666
25/8	225,733	1/10	184,162
27/8	264,550	8/10	112,935
30/8	226,244	17/10	49,799
4/9	222,222	24/10	28,778
1/9	220,264	5/11	16,446
2/9	228,310	24/11	12,492

TRITIUM ANALYSES RECORD - IWP-3-6

<u>Date</u>	<u>Activity (cpm/ml)</u>	<u>Date</u>	<u>Activity (cpm/ml)</u>
20/8	20	13/ 9	1,948
20/8	13,298	17/ 9	1,846
24/8	13,872	20/ 9	1,267
22/8	25,203	24/ 9	1,444
26/8	14,464	1/10	1,046
27/8	9,292	8/10	1,350
30/8	8,069	17/10	6,277
4/9	4,842	24/10	6,925
5/9	3,887	5/11	7,096
2/9	6,589	24/11	6,809
10/9	2,689	6/ 5/70	4,525

TRITIUM ANALYSES RECORD - IWP-3-18

<u>Date</u>	<u>Activity (cpm/ml)</u>	<u>Date</u>	<u>Activity (cpm/ml)</u>
24/8	32	4/9	52
28/8	30	5/9	23
30/8	34	10/9	30

TRITIUM ANALYSES RECORD - IWP-3-20

<u>Date</u>	<u>Activity (cpm/ml)</u>	<u>Date</u>	<u>Activity (cpm/ml)</u>
20/8	29	2/ 9	73
20/8	45	10/ 9	74
24/8	74	1/10	67
22/8	73	8/10	59
26/8	93	17/10	100
27/8	72	24/10	82
30/8	94	5/11	77
4/9	135	24/11	94

TRITIUM ANALYSES RECORD - IWP-3-24

<u>Date</u>	<u>Activity (cpm/ml)</u>	<u>Date</u>	<u>Activity (cpm/ml)</u>
20/8	20	17/ 9	161
20/8	24	24/ 9	101
24/8	27	1/10	114
28/8	77	8/10	106
30/8	82	17/10	100
4/9	20	24/10	86
5/9	144	5/11	82
10/9	163	24/11	101

TRITIUM ANALYSES RECORD - IWP-2-INJ

<u>Date</u>	<u>Activity (cpm/ml)</u>	<u>Date</u>	<u>Activity (cpm/ml)</u>
21/8	57,735	10/ 9	116,157
21/8	349,950	13/ 9	106,951
21/8	6,000	17/ 9	83,756
24/8	5,007	20/ 9	85,917
26/8	9,454	24/ 9	76,915
28/8	106,593	1/10	59,245
30/8	138,979	8/10	53,248
1/9	137,363	17/10	41,119
3/9	132,626	24/10	37,825
4/9	132,978	5/11	32,873
5/9	130,208	24/11	33,213

TRITIUM ANALYSES RECORD - IWP-2-26-43

<u>Date</u>	<u>Activity (cpm/ml)</u>	<u>Date</u>	<u>Activity (cpm/ml)</u>
24/8	28,926	13/ 9	46,168
28/8	37,059	17/ 9	42,706
21/8	28	20/ 9	45,241
22/8	16,239	24/ 9	44,339
26/8	35,844	1/10	41,098
30/8	30,754	8/10	40,919
4/9	43,195	17/10	37,900
1/9	42,444	24/10	37,797
3/9	46,749	5/11	36,416
5/9	42,633	24/11	43,370
10/9	42,047		

TRITIUM ANALYSES RECORD - IWP-2-4

<u>Date</u>	<u>Activity (cpm/ml)</u>	<u>Date</u>	<u>Activity (cpm/ml)</u>
24/8	102,499	10/ 9	27,120
28/8	99,087	13/ 9	27,901
21/8	15,836	17/ 9	25,177
22/8	100,678	20/ 9	27,865
23/8	106,461	24/ 9	27,282
30/8	98,597	1/10	22,663
4/9	52,402	17/10	19,523
1/9	77,869	24/10	19,627
3/9	60,961	5/11	20,006
5/9	42,890	24/11	20,538

TRITIUM ANALYSES RECORD - IWP-2-1

<u>Date</u>	<u>Activity (cpm/ml)</u>	<u>Date</u>	<u>Activity (cpm/ml)</u>
24/8	853	13/ 9	963
28/8	2,389	17/ 9	695
21/8	221	20/ 9	744
23/8	679	24/ 9	650
30/8	1,794	1/10	545
4/9	1,322	8/10	649
5/9	1,301	17/10	511
1/9	1,488	24/10	521
3/9	1,373	5/11	550
10/9	1,070	24/11	625

APPENDIX G

LISTING OF AND COMMENTS ON
INPUT AND OUTPUT FOR THE
STEADY-STATE MODEL

The first program includes a complete set of data input and output.

The second program includes print checks for debugging new data decks.

```

THIS PROGRAM CALCULATES TOTAL HEAD AT ALL NODES IN A REGION
WHEN GIVEN HEADS AT SPECIFIED BOUNDARY NODES
REAL NL(352),KX(576),KY(576)
COMMON M,CJ(352,2),IE(576,3),L1,L2,L3, T,ST(3),KL,KX,KY,S(576)
DIMENSION LIST(353),A(353),ISR(576),HDG(20)
DIMENSION STORE(6000), NEIN(353),NI(353,8)
DIMENSION G(3,6),DISPL(6),STRESS(576)
EQUIVALENCE (NE,NN)

```

```

JPD=5 1
JWT=6 3

```

```

READ JOB TITLE
1 READ(JRD,510,END=9999)(HDG(I),I=1,20)
WRITE(JWT,512)(HDG(I),I=1,20)

```

```

512 FORMAT('1'//20A4)
510 FORMAT(20A4)
WRITE(JWT,600)

```

```

600 FORMAT('/' TWO DIMENSIONAL SEEPAGE PROBLEM'//' INPUT DATA'//)
READ NUMBER OF ELEMENTS, NUMBER NODES, NUMBER SPECIFIED NODAL
HEADS, THICKNESS
ALL INPUT PRINTED OUT FOR CHECKING PURPOSES
READ(JRD,500) NEL,NE,NBD,T
IF(T.EQ.0.) T=1.
WRITE(JWT,601) NEL,NN,NBD,T

```

```

601 FORMAT(I4,' ELEMENTS'I4,' NODES'I4,' SPECIFIED NODAL HEADS'//' E
ELEMENT THICKNESS ='F8.3//)
500 FORMAT(3I5,F10.3)
READ COORDINATES OF NODES
READ(JRD,501)((CJ(J,I),I=1,2),J=1,NN)
WRITE(JWT,602)

```

```

602 FORMAT('//' NODAL COORDINATES (FT)' //' NODE X COORD
1 Y COORD')
DO 99 J=1,NN
99 WRITE(JWT,694)J,CJ(J,1),CJ(J,2)
501 FORMAT(6F10.3) -
ZERO CONSTANT VECTOR
191 DO 39 J=1,NN
39 NL(J)=0.

```

```

GENERATE NODAL INCIDENCE TABLE
WRITE(JWT,686)

```

```

686 FORMAT('//' ELEMENT INCIDENCE TABLE'//' ELEMENT NODE 1 NOD
1E 2 NODE 3 KX KY SLOPE'//)
SS=0.
DO 17 M=1,NEL
READ(JRD,505)(IE(M,I),I=1,3),XK,YK,SL
505 FORMAT(3I5,3F10.3)
IF(XK.NE.0.)XX=XK
IF(YK.NE.0.)YY=YK
IF(SL.NE.0.) SS=SL
S(M)=SS
KX(M)=XX
KY(M)=YY
17 WRITE(JWT,687)M,(IE(M,I),I=1,3),XX,YY,S(M)
687 FORMAT(4I9,3F10.3)
DO 18 J=1,NN
NEIN(J)=0

```



```

DO 18 M=1,6
18 NI(J,M)=0
DO 19 M=1,NEL
DO 19 K=1,3
J=IE(M,K)
NEIN(J)=NEIN(J)+1
L=NEIN(J)
19 NI(J,L)=M
694 FORMAT(I5,6F12.4)
READ ANY PRESCRIBED NODAL HEADS
DO 376 J=1,NN
376 ISR(J)=0
WRITE(JWT,596)
596 FORMAT(///' PRESCRIBED NODAL HEADS'///' NODE PRESCRIBED HEAD'///)
DO 574 I=1,NBD
READ(JRD,694)J, NL(J)
ISR(J)=1
574 WRITE(JWT,694)J, NL(J)
GENERATION AND ELIMINATION OF NODAL EQUILIBRIUM EQUATIONS
GENERATE I TH ROW OF STIFFNESS MATRIX AND STORE IN A TEMPORARILY
172 LIST(I)=1
DO 100 I=1,NE
NON ZERO BAND OF ROW I IN STIFFNESS IS FROM KL TO KH. KL = LOWEST
NODE NO FOR NODES INCIDENT ON ELEMENTS INCIDENT ON NODE I, KH = HIGHEST
KL=I
KH=I
IM=NEIN(I)
DO 5 J=1,IM
M=NI(I,J)
DO 5 K=1,3
JF=IE(M,K)
IF(JF-NE)10,10,5
10 IF(JF-KH)14,14,12
12 KH=JF
GO TO 5
14 IF(JF-KL)16,5,5
16 KL=JF
5 CONTINUE

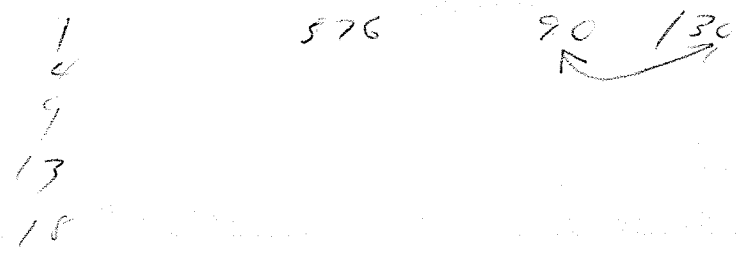
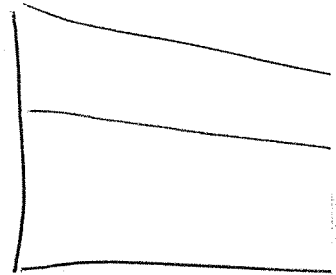
ZERO ALL A MATRICES IN NON - ZERO BAND
K=KH-KL+1
DO 29 J=1,K
29 A(J)=0.
INSERT STIFFNESS MATRICES INTO NON ZERO BAND
DO 200 J=1,IM
M=NI(I,J)
DO 20 L=1,3
IF(I-IE(M,L))20,22,20
22 KI=IE(M,L)
L1=L
L2=L+1-3*(L/3)
KJ=IE(M,L2)
L3=L+2-3*(L/2)
KK=IE(M,L3)
20 CONTINUE
CALL STIFF
NNI, NJ AND NK ARE NODES I, J AND K FOR ELEMENT M - (POSITION IN ROW I
RELATIVE TO KL = 1

```

```

- NNI=I-KL+1
INSERT STIFFNESS MATRIX I
A(NNI)=A(NNI)+ST(1)
TEST WHETHER NODE NJ IS A FIXED SUPPORT Water table
IF(KJ-NE)47,47,6
INSERT STIFFNESS MATRIX J
47 NJ=KJ-KL+1
A(NJ)=A(NJ)+ST(2)
TEST WHETHER NODE NK IS FIXED SUPPORT
6 IF(KK-NE)8,8,200
8 NK=KK-KL+1
INSERT STIFFNESS MATRIX K
A(NK)=A(NK)+ST(3)
200 CONTINUE
MODIFY EQUATION IF SPECIFIED HEAD AT NODE I
IF(ISR(I).EQ.0) GO TO 497
A(NNI)=10.**25 ←
NL(I)=NL(I)*10.**25
497 LINC=KH-I
FOR FIRST EQUATION, BYPASS ELIMINATION
IF(I-KL)60,60,50
PERFORM ELIMINATION FOR ROW I TO ZERO BELOW MAIN DIAGONAL
50 KU=I-1
DO 256 K=KL,KU
IK = PIVOTAL COLUMN RELATIVE TO KL = 1
IK=K+1-KL
IM=LIST(K+1)-LIST(K)
IJ=K+IM-I-LINC
IF NON ZERO BAND FOR PIVOTAL EQ ENDS TO RIGHT OF THAT FOR EQ I, EXTEND
FOR EQ I
IF(IJ.LE.0.) GO TO 82
KK=LINC+I-KL+2
LINC=LINC+IJ
LL=IJ+KK-1
DO 83 L=KK,LL
83 A(L)=0.
82 IF(IM.LE.0.) GO TO 256
DO 55 J=1,IM
IJ=IK+J
KJ= LIST(K)+J-1
IF(ABS(STORE(KJ)).LE.10.**(-10)) STORE(KJ)=0.
55 A(IJ)=A(IJ)-A(IK)*STORE(KJ)
256 NL(I)=NL(I)-A(IK)*NL(K)
NORMALIZE ROW I. JJ = PIVOTAL ELEMENT RELATIVE TO KL = 1
60 JJ=I+1-KL
IJ=JJ
LIST(I+1)=LINC+LIST(I) ← ←
IF(LINC)75,75,65 ←
65 DO 70 J=1,LINC
IJ=IJ+1 ←
IK=LIST(I)+J-1 ←
70 STORE(IK)=A(IJ)/A(IJ) ←
NORMALIZE HEAD AT NODE I
75 NL(I)=NL(I)/A(JJ) ←
100 CONTINUE
START BACK SUBSTITUTION

```



A(IJ)

```
NZ=NE-1
IF(N2) 210,210,220
220 DO 250 K=1,N2
    I=NE-K
    KU=LIST(I+1)-LIST(I)
    SUM=0.
    DO 240 J=1,KU
        IK=LIST(I)+J-1
        IJ=I+J
240 SUM=SUM+STORE(IK)*NL(IJ)
250 NL(I)=NL(I)-SUM
    WRITE NODAL HEADS
210 WRITE(JWT,560)(HDG(I),I=1,20)
560 FORMAT('1'//20A4,/' RESULTS'/' NODAL HEADS'/' NODE HEAD (FT
1)'//)
    DO 397 L=1,NN
397 WRITE(JWT,694)L,NL(L)
    CALL PLOTTER(NL)
999 CALL EXIT
    GO TO 1
    END
```

SUBROUTINE STIFF

THIS SUBROUTINE DEVELOPS STIFFNESS MATRICES II, IJ AND IK FOR
CONSTANT STRAIN TRIANGLE I-J-K

REAL ML(352), KX(576), KY(576)

COMMON M, CJ(352,2), IE(576,3), L1, L2, L3, T, ST(3), KL, KX, KY, S(576)

DIMENSION A(3), B(3)

SL=ABS(S(M)/57.2958)

COSA=COS(SL)

SINA=SIN(SL)

IF(S(M).LE.0.) SINA=-SINA

I=IE(M,1)

J=IE(M,2)

K=IE(M,3)

A(1)=(CJ(K,1)-CJ(J,1))*COSA+(CJ(K,2)-CJ(J,2))*SINA

A(2)=(CJ(I,1)-CJ(K,1))*COSA+(CJ(I,2)-CJ(K,2))*SINA

A(3)=(CJ(J,1)-CJ(I,1))*COSA+(CJ(J,2)-CJ(I,2))*SINA

B(1)=(CJ(J,2)-CJ(K,2))*COSA-(CJ(J,1)-CJ(K,1))*SINA

B(2)=(CJ(K,2)-CJ(I,2))*COSA-(CJ(K,1)-CJ(I,1))*SINA

B(3)=(CJ(I,2)-CJ(J,2))*COSA-(CJ(I,1)-CJ(J,1))*SINA

DET=CJ(J,1)*CJ(K,2)-CJ(K,1)*CJ(J,2)-CJ(I,1)*CJ(K,2)+CJ(K,1)*CJ(I,2)
1)+CJ(I,1)*CJ(J,2)-CJ(J,1)*CJ(I,2)

ST(1)=(KY(M)*A(L1)*A(L1)+KX(M)*B(L1)*B(L1))/2./DET

ST(2)=(KY(M)*A(L1)*A(L2)+KX(M)*B(L1)*B(L2))/2./DET

ST(3)=(KY(M)*A(L1)*A(L3)+KX(M)*B(L1)*B(L3))/2./DET

RETURN

END

```
SUBROUTINE PLOTTER(Z)
COMMON M,CJ(352,2),IE(576,3),L1,L2,L3, T,ST(3),KL,KX,KY,S(576)
DIMENSION Z(352),IBUF(1000)
CALL PLOTS(IBUF,1000)
CALL PLOT(0.0,-11.0,-3)
CALL PLOT(1.0,1.0,-3)
```

DRAW AXES

```
CALL AXIS(0.,0.,'JOB320',-6,40.,0.,0.,200.)
CALL AXIS(0.,0.,' ',1,6.,90.,800.,20.)
```

SCALING

```
DO 20 I=1,352
A=CJ(I,1)/200.
B=(CJ(I,2)-800)/20.
```

PLOT POINTS

```
CALL SYMBOL(A,B,0.07,4,0.,-1)
C=A-0.175
D=B-0.105
E=Z(I)-300
CALL NUMBER(C,D,0.07,E,0.,2)
20 CONTINUE
CALL PLOT(40.,0.,999)
RETURN
END
```

FINITE ELEMENT SEEPAGE PROGRAM BESWICK PINAWA

TWO DIMENSIONAL SEEPAGE PROBLEM

INPUT DATA

576 ELEMENTS 352 NODES 57 SPECIFIED NODAL HEADS

ELEMENT THICKNESS = 1.000

NODAL COORDINATES (FT)

NODE	X COORD	Y COORD
1	1.0000	839.0000
2	70.0000	849.0000
3	95.0000	839.0000
4	1.0000	830.0000
5	120.0000	830.0000
6	205.0000	848.0000
7	225.0000	839.0000
8	200.0000	860.0000
9	1.0000	822.5000
10	200.0000	822.0000
11	300.0000	830.0000
12	390.0000	821.0000
13	485.0000	830.0000
14	420.0000	839.0000
15	375.0000	848.0000
16	320.0000	859.0000
17	340.0000	865.0000
18	450.0000	868.0000
19	400.0000	865.0000
20	490.0000	859.0000
21	550.0000	821.0000
22	600.0000	839.0000
23	650.0000	830.5000
24	700.0000	820.0000
25	570.0000	848.0000
26	695.0000	859.0000
27	570.0000	865.0000
28	610.0000	869.0000
29	790.0000	865.0000
30	830.0000	869.0000
31	880.0000	859.0000
32	720.0000	848.0000
33	760.0000	839.0000
34	810.0000	830.5000
35	900.0000	817.5000
36	910.0000	839.0000
37	890.0000	848.0000
38	1000.0000	859.0000
39	940.0000	865.0000
40	1010.0000	869.0000
41	1120.0000	869.0000
42	1080.0000	865.0000
43	1030.0000	848.0000
44	1000.0000	828.0000
45	1080.0000	813.0000

47	1140.0000	839.0000
48	1250.0000	848.0000
49	1180.0000	859.0000
50	1240.0000	865.0000
51	1305.0000	869.5000
52	1510.0000	870.0000
53	1440.0000	865.0000
54	1390.0000	859.0000
55	1440.0000	848.0000
56	1330.0000	839.0000
57	1410.0000	828.0000
58	1280.0000	813.0000
59	1440.0000	815.0000
60	1620.0000	817.5000
61	1590.0000	829.0000
62	1510.0000	839.0000
63	1690.0000	839.0000
64	1610.0000	848.0000
65	1550.0000	859.0000
66	1620.0000	865.0000
67	1700.0000	870.0000
68	1860.0000	870.5000
69	1800.0000	865.0000
70	1710.0000	859.0000
71	1790.0000	848.0000
72	1820.0000	839.0000
73	1740.0000	829.5000
74	1790.0000	820.0000
75	1880.0000	829.5000
76	1970.0000	821.0000
77	1990.0000	839.0000
78	1920.0000	848.0000
79	1890.0000	859.0000
80	1950.0000	865.0000
81	2005.0000	870.5000
82	2150.0000	870.5000
83	2090.0000	865.0000
84	2020.0000	859.0000
85	2090.0000	848.0000
86	2140.0000	839.0000
87	2040.0000	830.0000
88	2160.0000	824.0000
89	2320.0000	824.5000
90	2225.0000	831.0000
91	2310.0000	839.0000
92	2250.0000	848.0000
93	2200.0000	859.0000
94	2250.0000	865.0000
95	2330.0000	871.0000
96	2515.0000	871.0000
97	2430.0000	865.0000
98	2380.0000	859.0000
99	2420.0000	848.0000
100	2470.0000	839.0000
101	2400.0000	832.5000
102	2490.0000	824.5000
103	2560.0000	834.0000
104	2650.0000	825.0000
105	2610.0000	839.0000
106	2580.0000	848.0000
107	2540.0000	859.0000
108	2610.0000	865.0000
109	2695.0000	871.0000
110	2800.0000	871.0000
111	2730.0000	865.0000

113	2710.0000	848.0000
114	2810.0000	859.0000
115	2880.0000	848.0000
116	2730.0000	835.0000
117	2810.0000	825.0000
118	2990.0000	827.0000
119	2900.0000	835.0000
120	3030.0000	848.0000
121	2990.0000	859.0000
122	2890.0000	865.0000
123	2970.0000	871.0000
124	3105.0000	871.0000
125	3030.0000	865.0000
126	3200.0000	865.0000
127	3280.0000	871.5000
128	3120.0000	859.0000
129	3200.0000	848.0000
130	3060.0000	835.5000
131	3115.0000	829.5000
132	3295.0000	831.0000
133	3210.0000	839.0000
134	3310.0000	848.0000
135	3290.0000	859.0000
136	3350.0000	865.0000
137	3410.0000	871.5000
138	3520.0000	871.5000
139	3450.0000	865.0000
140	3400.0000	859.0000
141	3500.0000	859.0000
142	3420.0000	848.0000
143	3370.0000	841.5000
144	3450.0000	834.5000
145	3615.0000	836.0000
146	3540.0000	844.0000
147	3600.0000	852.5000
148	3650.0000	859.0000
149	3560.0000	865.0000
150	3610.0000	871.5000
151	3750.0000	871.5000
152	3700.0000	865.0000
153	3900.0000	871.5000
154	3840.0000	865.0000
155	3800.0000	859.0000
156	3760.0000	852.5000
157	3700.0000	845.0000
158	3790.0000	835.5000
159	3980.0000	835.0000
160	3840.0000	844.0000
161	3910.0000	852.5000
162	3970.0000	859.0000
163	4010.0000	865.0000
164	4080.0000	871.5000
165	4020.0000	842.0000
166	4100.0000	852.5000
167	4170.0000	859.0000
168	4270.0000	871.5000
169	4210.0000	865.0000
170	4120.0000	834.5000
171	4210.0000	843.0000
172	4290.0000	852.5000
173	4330.0000	859.0000
174	4390.0000	865.0000
175	4440.0000	871.5000
176	4290.0000	834.5000
177	4380.0000	845.0000

179	4500.0000	859.0000
180	4550.0000	865.0000
181	4600.0000	872.0000
182	4770.0000	872.0000
183	4700.0000	865.0000
184	4640.0000	859.0000
185	4600.0000	852.5000
186	4550.0000	846.0000
187	4440.0000	834.5000
188	4580.0000	833.0000
189	4610.0000	840.0000
190	4670.0000	832.0000
191	4720.0000	840.0000
192	4690.0000	846.5000
193	4790.0000	847.5000
194	4790.0000	859.0000
195	4880.0000	859.0000
196	4820.0000	865.0000
197	4920.0000	865.0000
198	4890.0000	872.5000
199	4990.0000	872.5000
200	5150.0000	873.0000
201	5100.0000	865.0000
202	5030.0000	859.0000
203	4980.0000	850.0000
204	4890.0000	840.0000
205	4810.0000	831.0000
206	4980.0000	830.0000
207	5030.0000	840.0000
208	5120.0000	830.0000
209	5200.0000	840.0000
210	5290.0000	830.0000
211	5350.0000	840.0000
212	5280.0000	849.5000
213	5180.0000	849.5000
214	5150.0000	855.0000
215	5190.0000	859.0000
216	5240.0000	865.0000
217	5300.0000	874.0000
218	5480.0000	875.0000
219	5400.0000	865.0000
220	5350.0000	858.5000
221	5400.0000	849.5000
222	5470.0000	830.0000
223	5500.0000	840.0000
224	5540.0000	849.5000
225	5460.0000	860.0000
226	5600.0000	859.0000
227	5610.0000	864.0000
228	5490.0000	865.0000
229	5560.0000	875.5000
230	5680.0000	877.5000
231	5830.0000	878.5000
232	6000.0000	880.0000
233	5800.0000	867.5000
234	5760.0000	859.0000
235	5710.0000	849.5000
236	5680.0000	840.0000
237	5630.0000	830.5000
238	5800.0000	831.5000
239	5820.0000	840.0000
240	5870.0000	849.5000
241	5900.0000	859.0000
242	5950.0000	870.0000
243	6120.0000	881.5000

245	6090.0000	870.0000
246	6040.0000	859.0000
247	6010.0000	849.5000
248	5980.0000	840.0000
249	5950.0000	832.5000
250	6050.0000	834.0000
251	6050.0000	840.5000
252	6170.0000	834.5000
253	6100.0000	849.5000
254	6205.0000	849.5000
255	6130.0000	859.0000
256	6230.0000	859.0000
257	6290.0000	870.0000
258	6180.0000	870.0000
259	6190.0000	874.5000
260	6210.0000	883.0000
261	6340.0000	885.0000
262	6305.0000	876.0000
263	6460.0000	886.0000
264	6440.0000	880.0000
265	6410.0000	870.0000
266	6380.0000	859.0000
267	6340.0000	849.5000
268	6300.0000	836.0000
269	6480.0000	837.5000
270	6510.0000	849.5000
271	6540.0000	859.0000
272	6580.0000	870.0000
273	6600.0000	882.0000
274	6620.0000	888.0000
275	6780.0000	889.0000
276	6770.0000	885.0000
277	6760.0000	882.0000
278	6710.0000	870.0000
279	6690.0000	859.0000
280	6670.0000	849.5000
281	6620.0000	839.0000
282	6770.0000	840.0000
283	6790.0000	849.5000
284	6810.0000	859.0000
285	6840.0000	870.0000
286	6880.0000	882.0000
287	6900.0000	888.0000
288	6900.0000	890.5000
289	7000.0000	892.5000
290	6990.0000	882.0000
291	6960.0000	870.0000
292	6920.0000	859.0000
293	6905.0000	849.5000
294	6890.0000	841.0000
295	7005.0000	842.0000
296	7110.0000	849.5000
297	7090.0000	859.0000
298	7050.0000	870.0000
299	7020.0000	882.0000
300	7120.0000	895.0000
301	7280.0000	900.0000
302	7260.0000	895.0000
303	7220.0000	882.0000
304	7200.0000	870.0000
305	7180.0000	859.0000
306	7150.0000	849.5000
307	7140.0000	842.5000
308	7280.0000	842.5000
309	7280.0000	849.5000

311	7320.0000	870.0000
312	7360.0000	882.0000
313	7390.0000	895.0000
314	7410.0000	903.0000
315	7530.0000	903.0000
316	7510.0000	895.0000
317	7490.0000	882.0000
318	7450.0000	870.0000
319	7420.0000	859.0000
320	7400.0000	849.5000
321	7390.0000	842.5000
222	7510.0000	842.5000
323	7520.0000	849.5000
324	7550.0000	859.0000
325	7580.0000	870.0000
326	7600.0000	882.0000
327	7620.0000	895.0000
328	7650.0000	902.0000
329	7770.0000	901.0000
330	7740.0000	895.0000
331	7710.0000	882.0000
332	7690.0000	870.0000
333	7660.0000	859.0000
334	7630.0000	849.5000
335	7610.0000	843.0000
336	7710.0000	843.0000
337	7740.0000	849.5000
338	7780.0000	859.0000
339	7800.0000	870.0000
340	7830.0000	882.0000
341	7880.0000	895.0000
342	7890.0000	900.5000
343	7925.0000	900.5000
344	7925.0000	895.0000
345	7925.0000	882.0000
346	7925.0000	870.0000
347	7890.0000	859.0000
348	7925.0000	860.0000
349	7925.0000	850.0000
350	7925.0000	843.0000
351	7830.0000	843.0000
352	7870.0000	849.5000

ELEMENT INCIDENCE TABLE

ELEMENT	NODE 1	NODE 2	NODE 3	KX	KY	SLOPE
1	51	53	52	1.000	1.000	0.0
2	50	53	51	1.000	1.000	0.0
3	50	54	53	1.000	1.000	0.0
4	49	54	50	1.000	1.000	0.0
5	48	54	49	1.000	1.000	0.0
6	43	48	49	1.000	1.000	0.0
7	43	47	48	1.000	1.000	0.0
8	36	47	43	1.000	1.000	0.0
9	36	44	47	1.000	1.000	0.0
10	34	44	36	1.000	1.000	0.0
11	41	50	51	1.000	1.000	0.0
12	41	42	50	1.000	1.000	0.0
13	40	42	41	1.000	1.000	0.0
14	39	42	40	1.000	1.000	0.0
15	42	49	50	1.000	1.000	0.0
16	38	49	42	1.000	1.000	0.0

18	37	43	38	1.000	1.000	0.0
19	36	43	37	1.000	1.000	0.0
20	33	36	37	1.000	1.000	0.0
21	33	34	36	1.000	1.000	0.0
22	23	34	33	1.000	1.000	0.0
23	38	42	39	1.000	1.000	0.0
24	31	38	42	1.000	1.000	0.0
25	31	37	38	1.000	1.000	0.0
26	31	32	37	1.000	1.000	0.0
27	18	19	27	1.000	1.000	0.0
28	18	27	28	1.000	1.000	0.0
29	5	11	7	1.000	1.000	0.0
30	7	11	14	1.000	1.000	0.0
31	7	14	15	1.000	1.000	0.0
32	14	25	15	1.000	1.000	0.0
33	15	25	20	1.000	1.000	0.0
34	20	25	26	1.000	1.000	0.0
35	20	26	27	1.000	1.000	0.0
36	26	29	27	1.000	1.000	0.0
37	27	29	28	1.000	1.000	0.0
38	28	29	30	1.000	1.000	0.0
39	11	13	14	1.000	1.000	0.0
40	13	22	14	1.000	1.000	0.0
41	14	22	25	1.000	1.000	0.0
42	22	32	25	1.000	1.000	0.0
43	25	32	26	1.000	1.000	0.0
44	26	32	31	1.000	1.000	0.0
45	26	31	29	1.000	1.000	0.0
46	29	31	39	1.000	1.000	0.0
47	29	39	30	1.000	1.000	0.0
48	30	39	40	1.000	1.000	0.0
49	13	23	22	1.000	1.000	0.0
50	22	23	33	1.000	1.000	0.0
51	22	33	32	1.000	1.000	0.0
52	32	33	37	1.000	1.000	0.0
53	19	20	27	1.000	1.000	0.0
54	16	20	19	1.000	1.000	0.0
55	15	20	16	1.000	1.000	0.0
56	6	15	16	1.000	1.000	0.0
57	6	7	15	1.000	1.000	0.0
58	3	7	6	1.000	1.000	0.0
59	3	5	7	1.000	1.000	0.0
60	3	4	5	1.000	1.000	0.0
61	17	19	18	1.000	1.000	0.0
62	16	19	17	1.000	1.000	0.0
63	8	16	17	1.000	1.000	0.0
64	6	16	8	1.000	1.000	0.0
65	2	6	8	1.000	1.000	0.0
66	2	3	6	1.000	1.000	0.0
67	1	3	2	1.000	1.000	0.0
68	1	4	3	1.000	1.000	0.0
69	287	289	288	1.000	1.000	0.0
70	275	287	288	1.000	1.000	0.0
71	275	276	287	1.000	1.000	0.0
72	274	276	275	1.000	1.000	0.0
73	273	276	274	1.000	1.000	0.0
74	263	273	274	1.000	1.000	0.0
75	263	264	273	1.000	1.000	0.0
76	261	264	263	1.000	1.000	0.0
77	261	262	264	1.000	1.000	0.0
78	259	262	260	1.000	1.000	0.0
79	243	259	260	1.000	1.000	0.0
80	260	262	261	1.000	1.000	0.0
81	232	244	243	1.000	1.000	0.0
82	232	242	244	1.000	1.000	0.0

84	231	233	242	1.000	1.000	0.0
85	230	233	231	1.000	1.000	0.0
86	227	233	230	1.000	1.000	0.0
87	227	230	229	1.000	1.000	0.0
88	227	229	228	1.000	1.000	0.0
89	225	227	228	1.000	1.000	0.0
90	218	228	229	1.000	1.000	0.0
91	218	219	228	1.000	1.000	0.0
92	219	225	228	1.000	1.000	0.0
93	219	220	225	1.000	1.000	0.0
94	217	219	218	1.000	1.000	0.0
95	216	219	217	1.000	1.000	0.0
96	216	220	219	1.000	1.000	0.0
97	214	220	216	1.000	1.000	0.0
98	200	216	217	1.000	1.000	0.0
99	200	201	216	1.000	1.000	0.0
100	199	201	200	1.000	1.000	0.0
101	197	201	199	1.000	1.000	0.0
102	197	199	198	1.000	1.000	0.0
103	196	197	198	1.000	1.000	0.0
104	182	196	198	1.000	1.000	0.0
105	182	183	196	1.000	1.000	0.0
106	181	183	182	1.000	1.000	0.0
107	180	183	181	1.000	1.000	0.0
108	175	180	181	1.000	1.000	0.0
109	174	180	175	1.000	1.000	0.0
110	201	215	216	1.000	1.000	0.0
111	201	202	215	1.000	1.000	0.0
112	202	214	215	1.000	1.000	0.0
113	202	203	214	1.000	1.000	0.0
114	193	203	195	1.000	1.000	0.0
115	195	203	202	1.000	1.000	0.0
116	195	202	197	1.000	1.000	0.0
117	197	202	201	1.000	1.000	0.0
118	195	197	196	1.000	1.000	0.0
119	194	195	196	1.000	1.000	0.0
120	193	195	194	1.000	1.000	0.0
121	192	193	194	1.000	1.000	0.0
122	184	186	192	1.000	1.000	0.0
123	184	192	194	1.000	1.000	0.0
124	183	184	194	1.000	1.000	0.0
125	183	194	196	1.000	1.000	0.0
126	177	186	178	1.000	1.000	0.0
127	178	186	185	1.000	1.000	0.0
128	178	185	179	1.000	1.000	0.0
129	179	185	184	1.000	1.000	0.0
130	179	184	180	1.000	1.000	0.0
131	180	184	183	1.000	1.000	0.0
132	171	177	172	1.000	1.000	0.0
133	172	177	178	1.000	1.000	0.0
134	172	178	173	1.000	1.000	0.0
135	173	178	179	1.000	1.000	0.0
136	173	179	174	1.000	1.000	0.0
137	174	179	180	1.000	1.000	0.0
138	168	174	175	1.000	1.000	0.0
139	168	169	174	1.000	1.000	0.0
140	164	169	168	1.000	1.000	0.0
141	163	169	164	1.000	1.000	0.0
142	153	163	164	1.000	1.000	0.0
143	153	154	163	1.000	1.000	0.0
144	169	173	174	1.000	1.000	0.0
145	167	173	169	1.000	1.000	0.0
146	167	172	173	1.000	1.000	0.0
147	166	172	167	1.000	1.000	0.0
148	166	171	172	1.000	1.000	0.0

150	163	167	169	1.000	1.000	0.0
151	162	167	163	1.000	1.000	0.0
152	162	166	167	1.000	1.000	0.0
153	161	166	162	1.000	1.000	0.0
154	161	165	166	1.000	1.000	0.0
155	160	165	161	1.000	1.000	0.0
156	154	162	163	1.000	1.000	0.0
157	154	155	162	1.000	1.000	0.0
158	155	161	162	1.000	1.000	0.0
159	155	156	161	1.000	1.000	0.0
160	156	160	161	1.000	1.000	0.0
161	156	157	160	1.000	1.000	0.0
162	146	157	147	1.000	1.000	0.0
163	147	157	156	1.000	1.000	0.0
164	147	156	148	1.000	1.000	0.0
165	148	156	155	1.000	1.000	0.0
166	148	155	152	1.000	1.000	0.0
167	152	155	154	1.000	1.000	0.0
168	151	152	154	1.000	1.000	0.0
169	151	154	153	1.000	1.000	0.0
170	150	152	151	1.000	1.000	0.0
171	149	152	150	1.000	1.000	0.0
172	148	152	149	1.000	1.000	0.0
173	141	148	149	1.000	1.000	0.0
174	141	147	148	1.000	1.000	0.0
175	142	146	147	1.000	1.000	0.0
176	141	142	147	1.000	1.000	0.0
177	142	143	146	1.000	1.000	0.0
178	133	143	134	1.000	1.000	0.0
179	134	143	142	1.000	1.000	0.0
180	134	142	140	1.000	1.000	0.0
181	140	142	141	1.000	1.000	0.0
182	139	140	141	1.000	1.000	0.0
183	139	141	149	1.000	1.000	0.0
184	138	139	149	1.000	1.000	0.0
185	138	149	150	1.000	1.000	0.0
186	137	139	138	1.000	1.000	0.0
187	136	139	137	1.000	1.000	0.0
188	135	140	136	1.000	1.000	0.0
189	136	140	139	1.000	1.000	0.0
190	134	140	135	1.000	1.000	0.0
191	129	134	135	1.000	1.000	0.0
192	129	133	134	1.000	1.000	0.0
193	129	130	133	1.000	1.000	0.0
194	119	130	120	1.000	1.000	0.0
195	120	130	129	1.000	1.000	0.0
196	120	129	128	1.000	1.000	0.0
197	128	129	135	1.000	1.000	0.0
198	126	128	135	1.000	1.000	0.0
199	126	135	136	1.000	1.000	0.0
200	126	136	127	1.000	1.000	0.0
201	127	136	137	1.000	1.000	0.0
202	124	126	127	1.000	1.000	0.0
203	124	125	126	1.000	1.000	0.0
204	125	123	126	1.000	1.000	0.0
205	121	128	125	1.000	1.000	0.0
206	120	123	121	1.000	1.000	0.0
207	115	120	121	1.000	1.000	0.0
208	115	119	120	1.000	1.000	0.0
209	115	116	119	1.000	1.000	0.0
210	103	116	113	1.000	1.000	0.0
211	113	116	115	1.000	1.000	0.0
212	113	115	114	1.000	1.000	0.0
213	114	115	121	1.000	1.000	0.0
214	114	121	122	1.000	1.000	0.0
215	114	121	122	1.000	1.000	0.0

216	122	125	123	1.000	1.000	0.0
217	123	125	124	1.000	1.000	0.0
218	110	122	123	1.000	1.000	0.0
219	110	111	122	1.000	1.000	0.0
220	109	111	110	1.000	1.000	0.0
221	108	111	109	1.000	1.000	0.0
222	111	114	122	1.000	1.000	0.0
223	111	112	114	1.000	1.000	0.0
224	108	112	111	1.000	1.000	0.0
225	112	113	114	1.000	1.000	0.0
226	106	113	112	1.000	1.000	0.0
227	105	113	106	1.000	1.000	0.0
228	107	112	108	1.000	1.000	0.0
229	106	112	107	1.000	1.000	0.0
230	99	106	107	1.000	1.000	0.0
231	99	100	106	1.000	1.000	0.0
232	100	105	106	1.000	1.000	0.0
233	100	103	105	1.000	1.000	0.0
234	100	101	103	1.000	1.000	0.0
235	90	101	91	1.000	1.000	0.0
236	91	101	100	1.000	1.000	0.0
237	91	100	99	1.000	1.000	0.0
238	96	108	109	1.000	1.000	0.0
239	96	97	108	1.000	1.000	0.0
240	97	107	108	1.000	1.000	0.0
241	97	98	107	1.000	1.000	0.0
242	98	99	107	1.000	1.000	0.0
243	92	99	98	1.000	1.000	0.0
244	91	99	92	1.000	1.000	0.0
245	86	91	92	1.000	1.000	0.0
246	86	90	91	1.000	1.000	0.0
247	86	87	90	1.000	1.000	0.0
248	95	97	96	1.000	1.000	0.0
249	94	97	95	1.000	1.000	0.0
250	82	94	95	1.000	1.000	0.0
251	82	83	94	1.000	1.000	0.0
252	83	93	94	1.000	1.000	0.0
253	93	98	94	1.000	1.000	0.0
254	94	98	97	1.000	1.000	0.0
255	92	98	93	1.000	1.000	0.0
256	85	92	93	1.000	1.000	0.0
257	85	86	92	1.000	1.000	0.0
258	77	86	85	1.000	1.000	0.0
259	77	87	86	1.000	1.000	0.0
260	75	87	77	1.000	1.000	0.0
261	83	84	93	1.000	1.000	0.0
262	84	85	93	1.000	1.000	0.0
263	78	85	84	1.000	1.000	0.0
264	77	85	78	1.000	1.000	0.0
265	72	77	78	1.000	1.000	0.0
266	72	75	77	1.000	1.000	0.0
267	72	73	75	1.000	1.000	0.0
268	81	83	82	1.000	1.000	0.0
269	80	83	81	1.000	1.000	0.0
270	68	80	81	1.000	1.000	0.0
271	68	69	80	1.000	1.000	0.0
272	80	84	83	1.000	1.000	0.0
273	79	84	80	1.000	1.000	0.0
274	78	84	79	1.000	1.000	0.0
275	71	78	79	1.000	1.000	0.0
276	71	72	78	1.000	1.000	0.0
277	63	72	71	1.000	1.000	0.0
278	63	73	72	1.000	1.000	0.0
279	61	73	63	1.000	1.000	0.0
280	69	79	80	1.000	1.000	0.0

282	70	71	79	1.000	1.000	0.0
283	64	71	70	1.000	1.000	0.0
284	63	71	64	1.000	1.000	0.0
285	62	63	64	1.000	1.000	0.0
286	61	63	62	1.000	1.000	0.0
287	57	61	62	1.000	1.000	0.0
288	67	69	68	1.000	1.000	0.0
289	66	69	67	1.000	1.000	0.0
290	66	70	69	1.000	1.000	0.0
291	65	70	66	1.000	1.000	0.0
292	64	70	65	1.000	1.000	0.0
293	55	64	65	1.000	1.000	0.0
294	55	62	64	1.000	1.000	0.0
295	55	56	62	1.000	1.000	0.0
296	56	57	62	1.000	1.000	0.0
297	46	57	56	1.000	1.000	0.0
298	52	66	67	1.000	1.000	0.0
299	52	53	66	1.000	1.000	0.0
300	53	65	66	1.000	1.000	0.0
301	53	54	65	1.000	1.000	0.0
302	54	55	65	1.000	1.000	0.0
303	48	55	54	1.000	1.000	0.0
304	48	56	55	1.000	1.000	0.0
305	47	56	48	1.000	1.000	0.0
306	46	56	47	1.000	1.000	0.0
307	44	46	47	1.000	1.000	0.0
308	4	9	5	1.000	1.000	0.0
309	5	9	10	1.000	1.000	0.0
310	5	10	11	1.000	1.000	0.0
311	10	12	11	1.000	1.000	0.0
312	12	21	13	1.000	1.000	0.0
313	11	12	13	1.000	1.000	0.0
314	13	21	23	1.000	1.000	0.0
315	21	24	23	1.000	1.000	0.0
316	23	24	34	1.000	1.000	0.0
317	24	35	34	1.000	1.000	0.0
318	34	35	44	1.000	1.000	0.0
319	35	45	44	1.000	1.000	0.0
320	44	45	46	1.000	1.000	0.0
321	45	58	46	1.000	1.000	0.0
322	46	58	57	1.000	1.000	0.0
323	57	58	59	1.000	1.000	0.0
324	57	59	61	1.000	1.000	0.0
325	59	60	61	1.000	1.000	0.0
326	60	73	61	1.000	1.000	0.0
327	60	74	73	1.000	1.000	0.0
328	73	74	75	1.000	1.000	0.0
329	74	76	75	1.000	1.000	0.0
330	75	76	87	1.000	1.000	0.0
331	76	88	87	1.000	1.000	0.0
332	87	88	90	1.000	1.000	0.0
333	88	89	90	1.000	1.000	0.0
334	89	101	90	1.000	1.000	0.0
335	89	102	101	1.000	1.000	0.0
336	101	102	103	1.000	1.000	0.0
337	102	104	103	1.000	1.000	0.0
338	103	104	116	1.000	1.000	0.0
339	104	117	116	1.000	1.000	0.0
340	116	117	119	1.000	1.000	0.0
341	117	118	119	1.000	1.000	0.0
342	118	130	119	1.000	1.000	0.0
343	118	131	130	1.000	1.000	0.0
344	130	131	133	1.000	1.000	0.0
345	131	132	133	1.000	1.000	0.0
346	132	143	133	1.000	1.000	0.0

348	143	144	146	1.000	1.000	0.0
349	144	145	146	1.000	1.000	0.0
350	145	157	146	1.000	1.000	0.0
351	145	158	157	1.000	1.000	0.0
352	157	158	160	1.000	1.000	0.0
353	158	159	160	1.000	1.000	0.0
354	159	165	160	1.000	1.000	0.0
355	159	170	165	1.000	1.000	0.0
356	165	170	171	1.000	1.000	0.0
357	170	176	171	1.000	1.000	0.0
358	171	176	177	1.000	1.000	0.0
359	176	177	187	1.000	1.000	0.0
360	177	187	186	1.000	1.000	0.0
361	186	187	189	1.000	1.000	0.0
362	187	188	189	1.000	1.000	0.0
363	188	190	189	1.000	1.000	0.0
364	189	190	191	1.000	1.000	0.0
365	186	189	192	1.000	1.000	0.0
366	189	191	192	1.000	1.000	0.0
367	190	205	191	1.000	1.000	0.0
368	191	205	204	1.000	1.000	0.0
369	191	204	193	1.000	1.000	0.0
370	204	205	206	1.000	1.000	0.0
371	204	206	207	1.000	1.000	0.0
372	193	204	203	1.000	1.000	0.0
373	203	204	207	1.000	1.000	0.0
374	203	207	214	1.000	1.000	0.0
375	206	208	207	1.000	1.000	0.0
376	207	208	209	1.000	1.000	0.0
377	207	209	214	1.000	1.000	0.0
378	208	210	209	1.000	1.000	0.0
379	209	210	211	1.000	1.000	0.0
380	210	222	211	1.000	1.000	0.0
381	211	222	223	1.000	1.000	0.0
382	222	237	223	1.000	1.000	0.0
383	223	237	236	1.000	1.000	0.0
384	236	237	238	1.000	1.000	0.0
385	236	238	239	1.000	1.000	0.0
386	238	249	239	1.000	1.000	0.0
387	239	249	248	1.000	1.000	0.0
388	248	249	250	1.000	1.000	0.0
389	248	250	251	1.000	1.000	0.0
390	209	212	213	1.000	1.000	0.0
391	212	220	213	1.000	1.000	0.0
392	213	220	214	1.000	1.000	0.0
393	209	211	212	1.000	1.000	0.0
394	211	221	212	1.000	1.000	0.0
395	211	223	221	1.000	1.000	0.0
396	221	223	224	1.000	1.000	0.0
397	223	236	224	1.000	1.000	0.0
398	224	236	235	1.000	1.000	0.0
399	235	236	239	1.000	1.000	0.0
400	235	239	240	1.000	1.000	0.0
401	239	248	240	1.000	1.000	0.0
402	240	248	247	1.000	1.000	0.0
403	247	248	251	1.000	1.000	0.0
404	247	251	253	1.000	1.000	0.0
405	250	252	253	1.000	1.000	0.0
406	252	254	253	1.000	1.000	0.0
407	252	268	254	1.000	1.000	0.0
408	254	268	267	1.000	1.000	0.0
409	267	268	269	1.000	1.000	0.0
410	267	269	270	1.000	1.000	0.0
411	269	281	270	1.000	1.000	0.0
412	270	281	280	1.000	1.000	0.0

414	280	282	283	1.000	1.000	0.0
415	282	294	283	1.000	1.000	0.0
416	283	294	293	1.000	1.000	0.0
417	293	294	295	1.000	1.000	0.0
418	293	295	296	1.000	1.000	0.0
419	295	307	296	1.000	1.000	0.0
420	296	307	306	1.000	1.000	0.0
421	306	307	308	1.000	1.000	0.0
422	306	308	309	1.000	1.000	0.0
423	309	321	320	1.000	1.000	0.0
424	308	321	309	1.000	1.000	0.0
425	320	321	322	1.000	1.000	0.0
426	320	322	323	1.000	1.000	0.0
427	322	335	323	1.000	1.000	0.0
428	323	335	334	1.000	1.000	0.0
429	334	335	336	1.000	1.000	0.0
430	334	336	337	1.000	1.000	0.0
431	336	351	337	1.000	1.000	0.0
432	337	351	352	1.000	1.000	0.0
433	350	352	351	1.000	1.000	0.0
434	349	352	350	1.000	1.000	0.0
435	347	352	349	1.000	1.000	0.0
436	347	349	348	1.000	1.000	0.0
437	338	352	347	1.000	1.000	0.0
438	337	352	338	1.000	1.000	0.0
439	323	337	338	1.000	1.000	0.0
440	333	334	337	1.000	1.000	0.0
441	324	334	333	1.000	1.000	0.0
442	323	334	324	1.000	1.000	0.0
443	319	323	324	1.000	1.000	0.0
444	319	320	323	1.000	1.000	0.0
445	310	320	319	1.000	1.000	0.0
446	309	320	310	1.000	1.000	0.0
447	305	309	310	1.000	1.000	0.0
448	305	306	309	1.000	1.000	0.0
449	297	306	305	1.000	1.000	0.0
450	296	306	297	1.000	1.000	0.0
451	292	296	297	1.000	1.000	0.0
452	292	293	296	1.000	1.000	0.0
453	284	293	292	1.000	1.000	0.0
454	283	293	284	1.000	1.000	0.0
455	279	283	284	1.000	1.000	0.0
456	279	280	283	1.000	1.000	0.0
457	271	280	279	1.000	1.000	0.0
458	270	280	271	1.000	1.000	0.0
459	266	270	271	1.000	1.000	0.0
460	266	267	270	1.000	1.000	0.0
461	256	267	266	1.000	1.000	0.0
462	254	267	256	1.000	1.000	0.0
463	254	256	255	1.000	1.000	0.0
464	253	254	255	1.000	1.000	0.0
465	246	253	255	1.000	1.000	0.0
466	246	247	253	1.000	1.000	0.0
467	241	247	246	1.000	1.000	0.0
468	212	221	220	1.000	1.000	0.0
469	220	221	225	1.000	1.000	0.0
470	221	224	225	1.000	1.000	0.0
471	224	227	225	1.000	1.000	0.0
472	224	235	226	1.000	1.000	0.0
473	226	234	227	1.000	1.000	0.0
474	226	235	234	1.000	1.000	0.0
475	227	234	233	1.000	1.000	0.0
476	234	235	240	1.000	1.000	0.0
477	234	240	241	1.000	1.000	0.0
478	240	247	241	1.000	1.000	0.0
479	235	235	235	1.000	1.000	0.0

480	233	241	242	1.000	1.000	0.0
481	241	246	242	1.000	1.000	0.0
482	242	246	245	1.000	1.000	0.0
483	242	245	244	1.000	1.000	0.0
484	245	246	255	1.000	1.000	0.0
485	245	255	258	1.000	1.000	0.0
486	244	245	258	1.000	1.000	0.0
487	244	258	259	1.000	1.000	0.0
488	255	256	258	1.000	1.000	0.0
489	256	257	258	1.000	1.000	0.0
490	257	259	258	1.000	1.000	0.0
491	256	266	257	1.000	1.000	0.0
492	257	266	265	1.000	1.000	0.0
493	257	265	262	1.000	1.000	0.0
494	257	262	259	1.000	1.000	0.0
495	262	265	264	1.000	1.000	0.0
496	264	265	272	1.000	1.000	0.0
497	265	271	272	1.000	1.000	0.0
498	265	266	271	1.000	1.000	0.0
499	264	272	273	1.000	1.000	0.0
500	271	279	272	1.000	1.000	0.0
501	272	279	278	1.000	1.000	0.0
502	272	278	273	1.000	1.000	0.0
503	273	278	277	1.000	1.000	0.0
504	273	277	276	1.000	1.000	0.0
505	278	279	284	1.000	1.000	0.0
506	277	278	285	1.000	1.000	0.0
507	276	277	286	1.000	1.000	0.0
508	276	286	287	1.000	1.000	0.0
509	284	292	285	1.000	1.000	0.0
510	285	291	286	1.000	1.000	0.0
511	285	292	291	1.000	1.000	0.0
512	286	291	290	1.000	1.000	0.0
513	286	290	287	1.000	1.000	0.0
514	287	290	289	1.000	1.000	0.0
515	289	299	300	1.000	1.000	0.0
516	289	290	299	1.000	1.000	0.0
517	290	298	299	1.000	1.000	0.0
518	290	291	298	1.000	1.000	0.0
519	291	297	298	1.000	1.000	0.0
520	291	292	297	1.000	1.000	0.0
521	297	305	298	1.000	1.000	0.0
522	298	305	304	1.000	1.000	0.0
523	293	304	299	1.000	1.000	0.0
524	299	304	303	1.000	1.000	0.0
525	299	303	300	1.000	1.000	0.0
526	300	303	302	1.000	1.000	0.0
527	300	302	301	1.000	1.000	0.0
528	304	305	310	1.000	1.000	0.0
529	304	310	311	1.000	1.000	0.0
530	303	304	311	1.000	1.000	0.0
531	303	311	312	1.000	1.000	0.0
532	302	303	312	1.000	1.000	0.0
533	302	312	313	1.000	1.000	0.0
534	301	302	313	1.000	1.000	0.0
535	301	313	314	1.000	1.000	0.0
536	313	316	314	1.000	1.000	0.0
537	314	316	315	1.000	1.000	0.0
538	313	317	316	1.000	1.000	0.0
539	312	317	313	1.000	1.000	0.0
540	312	318	317	1.000	1.000	0.0
541	311	318	312	1.000	1.000	0.0
542	311	319	318	1.000	1.000	0.0
543	310	319	311	1.000	1.000	0.0
544	318	319	324	1.000	1.000	0.0

546	317	318	325	1.000	1.000	0.0
547	317	325	326	1.000	1.000	0.0
548	316	317	326	1.000	1.000	0.0
549	316	326	327	1.000	1.000	0.0
550	315	316	327	1.000	1.000	0.0
551	315	327	328	1.000	1.000	0.0
552	327	330	328	1.000	1.000	0.0
553	328	330	329	1.000	1.000	0.0
554	327	331	330	1.000	1.000	0.0
555	326	331	327	1.000	1.000	0.0
556	326	332	331	1.000	1.000	0.0
557	325	332	326	1.000	1.000	0.0
558	325	333	332	1.000	1.000	0.0
559	324	333	325	1.000	1.000	0.0
560	332	333	338	1.000	1.000	0.0
561	332	338	339	1.000	1.000	0.0
562	338	347	339	1.000	1.000	0.0
563	339	347	346	1.000	1.000	0.0
564	346	347	348	1.000	1.000	0.0
565	331	332	339	1.000	1.000	0.0
566	331	339	340	1.000	1.000	0.0
567	330	331	340	1.000	1.000	0.0
568	330	340	341	1.000	1.000	0.0
569	329	330	341	1.000	1.000	0.0
570	329	341	342	1.000	1.000	0.0
571	341	344	342	1.000	1.000	0.0
572	342	344	343	1.000	1.000	0.0
573	341	345	344	1.000	1.000	0.0
574	340	345	341	1.000	1.000	0.0
575	340	346	345	1.000	1.000	0.0
576	339	346	340	1.000	1.000	0.0

PRESCRIBED NODAL HEADS

NODE PRESCRIBED HEAD

1	839.0000
2	849.0000
8	860.0000
17	865.0000
18	868.0000
28	869.0000
30	869.0000
40	869.0000
41	869.0000
51	869.5000
52	870.0000
67	870.0000
68	870.5000
81	870.5000
82	870.5000
95	871.0000
96	871.0000
109	871.0000
110	871.0000
123	871.0000
124	871.0000
127	871.5000
137	871.5000
138	871.5000
150	871.5000
151	871.5000

164	871.5000
168	871.5000
175	871.5000
181	872.0000
182	872.0000
198	872.5000
199	872.5000
200	873.0000
217	874.0000
218	875.0000
229	875.5000
230	877.5000
231	878.5000
232	880.0000
243	881.5000
260	883.0000
261	885.0000
263	886.0000
274	888.0000
275	889.0000
288	890.5000
289	892.5000
300	895.0000
301	900.0000
314	900.0000
315	900.0000
328	900.0000
329	900.0000
342	900.0000
343	900.0000

FINITE ELEMENT SEEPAGE PROGRAM BESWICK PINAWA

RESULTS

NODAL HEADS

NODE HEAD (FT)

1	838.9998
2	848.9995
3	851.0352
4	840.5310
5	852.9375
6	859.7358
7	860.3801
8	859.9998
9	841.3079
10	858.8625
11	863.7202
12	866.5659
13	868.3914
14	867.1941
15	866.0562
16	864.4456
17	864.9998
18	867.9995
19	866.8091
20	868.3201
21	869.0432
22	869.3037
23	869.4353
24	869.4233
25	869.0679
26	869.2236
27	868.9602
28	868.9995
29	869.0750
30	868.9995
31	869.0200
32	869.2710
33	869.2639
34	869.1909
35	869.0786
36	869.0361
37	869.0376
38	868.9929
39	868.9902
40	868.9995
41	868.9995
42	868.9946
43	869.0417
44	869.0420
45	869.0916
46	869.2551
47	869.1563
48	869.3413
49	869.1831
50	869.3196
51	869.4998
52	869.9998
53	869.8462
54	869.7170
55	869.7510

56	869.5308
57	869.7058
58	869.4126
59	869.7656
60	870.0750
61	870.0295
62	869.9063
63	870.1614
64	870.0413
65	869.9463
66	869.9988
67	869.9998
68	870.4995
69	870.3040
70	870.1572
71	870.2920
72	870.3367
73	870.2356
74	870.2993
75	870.3994
76	870.4878
77	870.5090
78	870.4517
79	870.4421
80	870.5117
81	870.4995
82	870.4995
83	870.5005
84	870.4968
85	870.5833
86	870.6548
87	870.5583
88	870.6914
89	870.8794
90	870.7646
91	870.8672
92	870.7847
93	870.6951
94	870.7852
95	870.9995
96	870.9995
97	871.0210
98	870.9705
99	870.9944
100	871.0188
101	870.9653
102	871.0281
103	871.0493
104	871.0432
105	871.0532
106	871.0454
107	871.0303
108	871.0117
109	870.9995
110	870.9995
111	871.0063
112	871.0171
113	871.0261
114	871.0063
115	870.9924
116	871.0256
117	871.0046
118	871.0098
119	870.9944
120	871.0132

122	870.9983
123	870.9995
124	870.9995
125	870.9756
126	871.2681
127	871.4995
128	871.1309
129	871.2754
130	871.0532
131	871.1331
132	871.4329
133	871.2974
134	871.4504
135	871.4331
136	871.5278
137	871.4995
138	871.4995
139	871.5103
140	871.5320
141	871.5176
142	871.5332
143	871.5168
144	871.5398
145	871.5278
146	871.5364
147	871.5249
148	871.5161
149	871.5107
150	871.4995
151	871.4995
152	871.5071
153	871.4995
154	871.5010
155	871.5046
156	871.5105
157	871.5188
158	871.5112
159	871.5071
160	871.5090
161	871.5039
162	871.5027
163	871.5010
164	871.4995
165	871.5054
166	871.5046
167	871.4958
168	871.4995
169	871.4929
170	871.4998
171	871.5034
172	871.4670
173	871.4604
174	871.4861
175	871.4995
176	871.5295
177	871.4751
178	871.5640
179	871.7144
180	871.8364
181	871.9998
182	871.9998
183	872.0195
184	871.9587
185	871.9077
186	871.8337
187	871.8510

188	871.8755
189	871.9155
190	871.9856
191	872.0564
192	872.0217
193	872.1685
194	872.1519
195	872.3447
196	872.1943
197	872.4441
198	872.4998
199	872.4998
200	872.9995
201	872.8330
202	872.6626
203	872.5347
204	872.3564
205	872.2092
206	872.5427
207	872.6833
208	873.0212
209	873.3674
210	873.8208
211	874.1924
212	873.8235
213	873.2786
214	873.1182
215	873.2734
216	873.5918
217	873.9998
218	874.9998
219	874.5342
220	874.2236
221	874.5020
222	875.0352
223	875.2996
224	875.6709
225	874.8840
226	876.3049
227	876.4055
228	875.1121
229	875.4995
230	877.4998
231	878.4995
232	879.9995
233	878.3367
234	877.9363
235	877.4390
236	877.1284
237	876.6292
238	878.2590
239	878.4426
240	878.8953
241	879.1448
242	879.5435
243	881.4998
244	881.2493
245	881.1284
246	880.5276
247	880.2063
248	879.9233
249	879.6379
250	880.6445
251	880.6619
252	882.4316

254 882.9695
255 881.7590
256 883.3550
257 884.2439
258 882.5422
259 882.6936
260 882.9995
261 884.9995
262 884.4861
263 885.9998
264 885.8406
265 885.5637
266 885.2644
267 884.7783
268 884.2695
269 886.4675
270 886.7947
271 887.1038
272 887.5303
273 887.7773
274 887.9995
275 888.9998
276 888.9250
277 888.8469
278 888.5002
279 888.3616
280 888.2002
281 887.7898
282 888.9141
283 889.0950
284 889.2751
285 889.6211
286 890.2378
287 890.5105
288 890.4995
289 892.4995
290 892.2542
291 891.6790
292 890.8838
293 890.5889
294 890.3293
295 892.8464
296 895.2527
297 894.7258
298 893.6775
299 892.8164
300 894.9995
301 899.9995
302 899.4331
303 898.1677
304 897.6335
305 897.0952
306 896.3186
307 896.0415
308 899.1243
309 899.1262
310 899.3940
311 899.6335
312 900.0295
313 900.0815
314 899.9995
315 899.9995
316 900.0222
317 900.0776
318 900.1160

320	900.0002
321	899.9568
322	900.0916
323	900.0952
324	900.0906
325	900.0640
326	900.0359
327	900.0142
328	899.9995
329	899.9995
330	900.0042
331	900.0176
332	900.0315
333	900.0486
334	900.0645
335	900.0708
336	900.0457
337	900.0422
338	900.0352
339	900.0293
340	900.0168
341	900.0071
342	899.9995
343	899.9995
344	900.0000
345	900.0056
346	900.0110
347	900.0232
348	900.0151
349	900.0181
350	900.0205
351	900.0374
352	900.0332

THIS PROGRAM CALCULATES TOTAL HEAD AT ALL NODES IN A REGION
WHEN GIVEN HEADS AT SPECIFIED BOUNDARY NODES

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REAL NL(360),KX(576),KY(576)
INTEGER ELEM(576)
COMMON M,CJ(360,2),IE(576,3),L1,L2,L3, T,ST(3),KL,KX,KY,S(576)
COMMON JRD,JWT
DIMENSION LIST(360),A(360),ISR(576),HDG(20)
DIMENSION STORE(6000),NODE(360), NEIN(360),NI(360,8)
DIMENSION G(3,6),DISPL(6),STRESS(576)
EQUIVALENCE (NE,NN)
JRD=1
JWT=3
IPAGE=1
LINE=0

```

```

510 READ JOB TITLE
    1 READ(JRD,510)IDENT, (HDG(I),I=1,19)
    IF(IDENT.EQ.0) GO TO 9999
564 FORMAT('1',100X,'PAGE ',I4)
    WRITE(JWT,564) IPAGE
    IPAGE=IPAGE+1
512 FORMAT('0', 'NO.',15,1X,19A4)
    WRITE(JWT,512) IDENT,(HDG(I),I=1,19)
    WRITE(JWT,600)
600 FORMAT(/' TWO DIMENSIONAL SEEPAGE PROBLEM'/' INPUT DATA'/'/)
    READ NUMBER OF ELEMENTS, NUMBER NODES,NUMBER SPECIFIED NODAL
    HEADS, THICKNESS
    ALL INPUT PRINTED OUT FOR CHECKING PURPOSES
    READ(JRD,500) NEL,NE,NBD,T
500 FORMAT(3I5,F10.3)
    IF(T.EQ.0.) T=1.
    WRITE(JWT,601) NEL,NN,NBD,T
601 FORMAT(I4,' ELEMENTS'I4,' NODES'I4,' SPECIFIED NODAL HEADS'/'/' EL
    EMENT THICKNESS ='F8.3'/'/)
    READ COORDINATES OF NODES
501 FORMAT(3(I5,2F10.3))
    READ(JRD,501)(NODE(J),(CJ(J,I),I=1,2),J=1,NN)
    LINE=5
    WRITE(JWT,564) IPAGE
    IPAGE=IPAGE+1
    WRITE(JWT,602)
6020FORMAT(1H0,'NODAL COORDINATES (FT) '/'/' NODE X COORD Y COO
    1RD'/'/)
    DO 99 J=1,NN
    WRITE(JWT,694)NODE(J),CJ(J,1),CJ(J,2)
    LINE=LINE+1
    IF(LINE.LT.60)GO TO 99
    WRITE(JWT,564) IPAGE
    IPAGE=IPAGE+1
    LINE=5
99 CONTINUE
    LINE=4
    ZERO CONSTANT VECTOR

```

191 DO 39 J=1,360
39 NL(J)=0.

GENERATE NODAL INCIDENCE TABLE

WRITE(JWT,564) IPAGE
IPAGE=IPAGE+1
WRITE(JWT,686)

6860 FORMAT(1H0, ' ELEMENT INCIDENCE TABLE '///' ELEMENT NODE 1 NODE
1 2 NODE 3 KX KY SLOPE'//)

SS=0.

DO 17 M=1,NEL

505 FORMAT(I5,3I5,3F10.3)
READ(JRD,505)ELEM(M),(IE(M,I),I=1,3),XK,YK,SL
IF(XK.NE.C.)XX=XK
IF(YK.NE.C.)YY=YK
IF(SL.NE.C.)SS=SL

S(M)=SS

KX(M)=XX

KY(M)=YY

WRITE(JWT,687)ELEM(M),(IE(M,I),I=1,3),XX,YY,S(M)

687 FORMAT(4I9,3F10.3)

LINE=LINE+1

IF(LINE.LT.60) GO TO 2002

WRITE(JWT,564) IPAGE

IPAGE=IPAGE+1

LINE=4

~~2002 IF(IE(M,1).EQ.IE(M,2)) GO TO 2000~~

~~IF(IE(M,1).EQ.IE(M,3)) GO TO 2000~~

~~IF(IE(M,2).EQ.IE(M,3)) GO TO 2000~~

GO TO 17

~~2001 FORMAT(10, 'CHECK ELEMENT(1,13,1) INCIDENCES')~~

~~2000 WRITE(JWT,2001) ELEM(M)~~

~~STOP 2000~~

*elements checked
duplication of values. (debug)*

17 CONTINUE

LINE = 5

DO 18 J=1,360

A(J)=0.0

NEIN(J)=0

DO 18 M=1,8

18 NI(J,M)=0

DO 19 N=1,NEL

DO 19 K=1,3

J=IE(M,K)

NEIN(J)=NEIN(J)+1

L=NEIN(J)

NI(J,L)=M

19 CONTINUE

694 FORMAT(I5,6F12.4)

READ ANY PRESCRIBED NODAL HEADS

DO 376 J=1,576

376 ISR(J)=0

WRITE(JWT,564) IPAGE

IPAGE = IPAGE + 1

```

596 FORMAT('0','PRESCRIBED NODAL HEADS'///' NODE    PRES.HEAD'///)
WRITE(JWT,596)
DO 574 I=1,NBD
READ(JRD,694)J, NL(J)
ISR(J)=1
WRITE(JWT,694)J, NL(J)
LINE = LINE + 1
IF(LINE.LT.60)GO TO 574
WRITE (JWT,564)IPAGE
WRITE (JWT,596)
IPAGE = IPAGE + 1
574 CONTINUE
LINE=11
GENERATION AND ELIMINATION OF NODAL EQUILIBRIUM EQUATIONS
GENERATE I TH ROW OF STIFFNESS MATRIX AND STORE IN A TEMPORARILY
172 LIST(1)=1
LINE=0
DO 100 I=1,NE
NON ZERO BAND OF ROW I IN STIFFNESS IS FROM KL TO KH. KL = LOWEST
  NODE NO FOR NODES INCIDENT ON ELEMENTS INCIDENT ON NODE I, KH = HIGHEST
KL=I
KH=I
IM=NEIN(I)-1
DO 5 J=1,IM
M=NI(I,J)
DO 5 K=1,3
JF=IE(M,K)
IF(JF-NE)10,10,5
10 IF(JF-KH)14,14,12
12 KH=JF
GO TO 5
14 IF(JF-KL)16,5,5
16 KL=JF
5 CONTINUE

ZERO ALL A MATRICES IN NON - ZERO BAND
K=KH-KL+1
DO 29 J=1,K
29 A(J)=0.
INSERT STIFFNESS MATRICES INTO NON ZERO BAND
DO 200 J=1,IM
M=NI(I,J)
DO 20 L=1,3
IF(I-IE(M,L))20,22,20
22 KI=IE(M,L)
L1=L
L2=L+1-3*(L/3)
KJ=IE(M,L2)
L3=L+2-3*(L/2)
KK=IE(M,L3)
20 CONTINUE
CALL STIFF
NNI, NJ AND NK ARE NODES I, J AND K FOR ELEMENT M - (POSITION IN ROW I
RELATIVE TO KL = 1

```

NNI=I-KL+1

INSERT STIFFNESS MATRIX I

A(NNI)=A(NNI)+ST(1)

TEST WHETHER NODE NJ IS A FIXED SUPPORT

IF(KJ-NE)47,47,6

INSERT STIFFNESS MATRIX J

47 NJ=KJ-KL+1

A(NJ)=A(NJ)+ST(2)

TEST WHETHER NODE NK IS FIXED SUPPORT

6 IF(KK-NE)8,8,200

8 NK=KK-KL+1

INSERT STIFFNESS MATRIX K

A(NK)=A(NK)+ST(3)

200 CONTINUE

MODIFY EQUATION IF SPECIFIED HEAD AT NODE I

IF(ISR(I).EQ.0) GO TO 497

A(NNI)=10.**25

NL(I)=NL(I)*10.**25

497 LINC=KH-I

FOR FIRST EQUATION, BYPASS ELIMINATION

IF(I-KL)60,60,50

PERFORM ELIMINATION FOR ROW I TO ZERO BELOW MAIN DIAGONAL

50 KU=I-1

DO 256 K=KL,KU

IK = PIVOTAL COLUMN RELATIVE TO KL = 1

IK=K+1-KL

IM=LIST(K+1)-LIST(K)

IJ=K+IM-I-LINC

IF NON ZERO BAND FOR PIVOTAL EQ ENDS TO RIGHT OF THAT FOR EQ I, EXTEND FOR EQ I

IF(IJ.LE.0.) GO TO 82

KK=LINC+I-KL+2

LINC=LINC+IJ

LL=IJ+KK-1

DO 83 L=KK,LL

83 A(L)=0.

82 IF(IM.LE.0.) GO TO 256

~~3000~~ ~~FORMAT(10,'MAIN LOOP I=',I4,' IM=',I4,' *****', LINC=',I4/)~~

WRITE(JWT,3000) I,IM,LINC

Print check for main loop (debug)

DO 55 J=1,IM

IJ=IK+J

KJ= LIST(K)+J-1

IF(ABS(STORE(KJ)).LE.10.**(-10)) STORE(KJ)=0.

A(IJ)=A(IJ)-A(IK)*STORE(KJ)

~~IF(ABS(A(IJ)).EQ.0.) GO TO 55~~

~~IF(ABS(A(IJ)).GT.10.**(-50)) GO TO 55~~

~~3001~~ ~~FORMAT(10,'IM LOOP J=',I4,' IJ=',I4,' KJ=',I4,' A(IJ)=',E12.5~~

~~1,' A(IK)=',E12.5,' STORE(KJ)=',E12.5)~~

Print check for location of underflow errors (debug)

~~IF(LINE.EQ.1000) GO TO 55~~

~~WRITE(JWT,3001) J,IJ,KJ,A(IJ),A(IK),STORE(KJ)~~

~~LINE=LINE+1~~

55 CONTINUE

256 NL(I)=NL(I)-A(IK)*NL(K)

NORMALIZE ROW I. JJ = PIVOTAL ELEMENT RELATIVE TO KL = 1

```
60 JJ=I+1-KL
    IJ=JJ
```

```
    LIST(I+1)=LINC+LIST(I)
    IF(LINC)75,75,65
```

```
65 DO 70 J=1,LINC
    IJ=IJ+1
    IK=LIST(I)+J-1
```

```
70 STORE(IK)=A(IJ)/A(JJ)
    NORMALIZE HEAD AT NODE I
```

```
75 NL(I)=NL(I)/A(JJ)
```

```
100 CONTINUE
    START BACK SUBSTITUTION
    N2=NE-1
```

```
    IF(N2)210,210,220
```

```
220 DO 250 K=1,N2
    I=NE-K
```

```
    KU=LIST(I+1)-LIST(I)
    SUM=0.
```

```
    DO 240 J=1,KU
```

```
    IK=LIST(I)+J-1
    IJ=I+J
```

```
240 SUM=SUM+STORE(IK)*NL(IJ)
```

```
250 NL(I)=NL(I)-SUM
```

```
    WRITE NODAL HEADS
```

```
210 WRITE(JWT,564) IPAGE
```

```
    IPAGE=IPAGE+1
```

```
    WRITE(JWT,560)(HDG(I),I=1,20)
```

```
560 FORMAT('0'1X2CA4, '// ' RESULTS' // ' NODAL HEADS' // ' NODE HEAD (F
IT)' //)
```

```
    DO 397 L=1,NN
```

```
562 FORMAT(I6,F12.4)
```

```
    WRITE(JWT,562)L,NL(L)
```

```
    LINE =LINE + 1
```

```
    IF(LINE.LT.60) GO TO 397
```

```
    WRITE(JWT,564) IPAGE
```

```
    IPAGE=IPAGE+1
```

```
561 FORMAT('0', ' RESULTS CONT. ' // ' NODE HEAD (FT)' //)
```

```
    WRITE(JWT,561)
```

```
    LINE = 7
```

```
397 CONTINUE
```

```
563 FORMAT('//1X, 'END OF RUN NO. ',I4)
```

```
    WRITE(JWT,563) IDENT
```

```
    LINE=0
```

```
    IPAGE=1
```

```
    GO TO 1
```

```
9999 STOP 1000
    END
```

CALL PLOTTER (NL) - subroutine PLOTTER may be introduced at this point

SUBROUTINE STIFF

THIS SUBROUTINE DEVELOPS STIFFNESS MATRICES II,IJ AND IK FOR
CONSTANT STRAIN TRIANGLE I-J-K

REAL NLI(360),KX(576),KY(576)

COMMON M,CJ(360,2),IE(576,3),L1,L2,L3, T,ST(3),KL,KX,KY,S(576)

COMMON JRD,JWT

DIMENSION A(3),B(3)

~~15~~ ~~FORMAT(10,I,1,CHECK NODAL INCIDENCES \$\$\$ I=1, I4, J=2, I4, K=3, I4)~~

SL=ABS(S(M)/57.2958)

COSA=COS(SL)

SINA=SIN(SL)

IF(S(M).LE.0.) SINA=-SINA

I=IE(M,1)

J=IE(M,2)

K=IE(M,3)

A(1)=(CJ(K,1)-CJ(J,1))*COSA+(CJ(K,2)-CJ(J,2))*SINA

A(2)=(CJ(I,1)-CJ(K,1))*COSA+(CJ(I,2)-CJ(K,2))*SINA

A(3)=(CJ(J,1)-CJ(I,1))*COSA+(CJ(J,2)-CJ(I,2))*SINA

B(1)=(CJ(J,2)-CJ(K,2))*COSA-(CJ(J,1)-CJ(K,1))*SINA

B(2)=(CJ(K,2)-CJ(I,2))*COSA-(CJ(K,1)-CJ(I,1))*SINA

B(3)=(CJ(I,2)-CJ(J,2))*COSA-(CJ(I,1)-CJ(J,1))*SINA

DET=CJ(J,1)*CJ(K,2)-CJ(K,1)*CJ(J,2)-CJ(I,1)*CJ(K,2)+CJ(K,1)*CJ(I,2)
1)+CJ(I,1)*CJ(J,2)-CJ(J,1)*CJ(I,2)

~~IF(DET.NE.0.) GO TO 16~~

~~WRITE(JWT,15)I,J,K~~

~~STOP 3000~~

~~16~~ ST(1)=(KY(M)*A(L1)*A(L1)+KX(M)*B(L1)*B(L1))/2./DET

ST(2)=(KY(M)*A(L1)*A(L2)+KX(M)*B(L1)*B(L2))/2./DET

ST(3)=(KY(M)*A(L1)*A(L3)+KX(M)*B(L1)*B(L3))/2./DET

RETURN

END

print check for nodal values (debug)